

STORMWATER MANAGEMENT REPORT FOR ENVIRONMENTAL ASSESSMENT (EA) AND PRELIMINARY DESIGN FOR DRAINAGE IMPROVEMENTS OF HIGHWAY 50 FROM MAYFIELD ROAD TO HEALEY ROAD

Draft

November 15, 2021

Prepared for:









STORMWATER
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FOR ENVIRONMENTAL
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Region of Peel



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RVA 194615

November 15, 2021

Regional Road 50 / Mayfield Road / Healey Road Storm Drainage Design Brief

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1.0 Introduction

1.1 Project Description

R.V. Anderson Associates Limited was retained by the Region of Peel (the Region) to undertake a Schedule "B" Municipal Class Environmental Assessment (EA) and preliminary design to assess low impact development (LID) and drainage infrastructure improvements required for the roadside drainage ditches and culvert crossings on Regional Road 50 (RR50) from Mayfield Road to Healey Road. The Region has identified several driveway culverts that are failing along RR50 from Mayfield Road to Healey Road (approximately 2.5 km) within the Town of Caledon.

As part of the Class EA study, preliminary recommendations to improve pedestrian and cyclist amenities (i.e. multi-use path (MUP) along the west side of Regional Road 50 from Mayfield Road to Healey Road and a new sidewalk between #12599 Regional Road (Highway) 50 and George Bolton Parkway have also been included to support the Region's 'Sustainable Transportation Strategy' (STS).

RR50 and Mayfield Road are located at the northeast corner of the City of Brampton border. RR50 / Albion - Vaughan Road is the regional border division between the Region to the west and the Regional Municipality of York (York Region) to the east. It is also the municipal border division between the City of Brampton to the west and the City of Vaughan to the east. Mayfield Road is the municipal border division between the City of Brampton and the Town of Caledon.

This Stormwater Management (SWM) report was undertaken in support of the development of the alternatives and recommendations as identified through the Class EA study and outlines the proposed storm drainage system consisting of storm sewers, infiltration facilities, ditch inlets, oil grit separators, ditches, and culvert replacements for RR50 from Mayfield Road to Healey Road.

1.2 Project Background

The subject study area is shown below in Figure 1.1 and is within the Humber River Watershed (Main Humber River primary subwatershed). It is also located within the Rainbow Creek subwatershed of the Humber River watershed. This watershed is managed by the Toronto and Region Conservation Authority (TRCA). The Rainbow Creek subwatershed is drained by two watercourses –







Rainbow Creek and Robinson Creek. A Rainbow Creek tributary parallels the study area and Robinson Creek crosses RR50 at George Bolton Parkway.

In late 1996, the driveway culverts identified by the Region were transferred from the Ministry of Transportation (MTO) in "as is" condition. The Region is in the process of improving the existing SWM / drainage along RR50 to cater to existing and future growth around the study area. Condition assessments were completed by the Region for seventeen subject culverts in 2017. Furthermore, the EA preferred alternative includes building a multi - use path (MUP) along the west side of RR50 and sidewalk on the east side of the corridor north of George Bolton, to satisfy requirements for Active Transportation (AT).

The existing subcatchments on RR50 were delineated based on the information provided by TRCA, localized survey information, and from previous studies as provided by the Region and the Town of Caledon. Current land uses in the study catchments are primarily industrial paved areas.

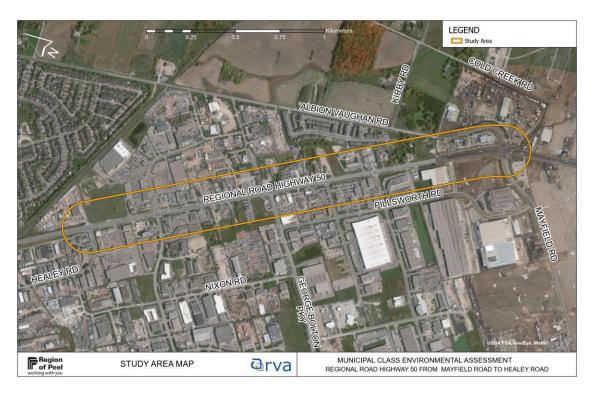


Figure 1.1: Study Area

Potential permits and approvals necessary to complete the undertaking include TRCA, Ministry of Natural Resources (MNR), Ministry of Tourism, Culture, and Sport (MTCS), surrounding regional and municipal approvals, and utility approvals or relocations.







The following reports and models were utilized as the basis for the SWM design:

- Ontario Ministry of the Environment (MECP) Stormwater Management Planning and Design Manual, 2003
- LID Implementation Process for Regional Road Right Of Ways, 2014
- Development Standards, Policies & Guidelines, Town of Caledon, 2009
- Fluvial Geomorphology and Hydraulic Assessments, Highway 50 Active Transportation and Drainage Improvements, Environmental Assessment, Matrix Solutions, 2021 (Draft)
- InfoWorks Model for Regional Road 50, Region of Peel, 2020
- Public Works Stormwater Design Criteria and Procedural Manual, Region of Peel, 2019
- Stormwater Management Implementation Report, Bolton Gateway Developments Inc., Town of Caledon, Region of Peel, C.F. Crozier & Associates Inc., 2015
- Natural Heritage Report Existing Conditions, Schedule B Class Environmental Assessment (EA) and Preliminary Design for Drainage Improvements of Regional Road 50 From Mayfield Road to Healey, Town of Caledon, Region of Peel, LGL Limited, 2020

1.3 Purpose

The purpose of this report is to provide a SWM strategy that considers best management practices, supports climate change requirements and meets existing and future regulatory requirements in support of the Class EA. The SWM strategy will support the EA study requirements by:

- identifying the recommended drainage infrastructure improvements required along Regional Road 50,
- identifying and addressing impacts related to the EA recommended MUP along the west side of RR50 and proposed sidewalk on the east side of RR50, north of George Bolton Parkway,
- incorporating concerns from stakeholders and regulatory agencies.

Under existing conditions RR50 is a fully urbanized five lane road. Sidewalk exists intermittently along the west side of the road. Between Healey Road and







Hopcroft Road there are sidewalks on both sides. In the west boulevard existing utilities including hydro poles, underground gas, bell, and watermains are present. An existing sanitary sewer runs along the west side of RR50 until George Bolton Parkway, where it crosses and runs along the east boulevard. Surrounding land use is commercial and industrial with a high percentage of impervious cover. Road drainage is primarily conveyed by storm sewers along the east side of the road, while ditches on either side of the road convey predominantly external drainage. Existing sewers discharge directly to Robinson Creek at George Bolton Parkway and Simona Drive. No quantity or quality controls are provided for the existing road drainage.

Robinson Creek crosses RR50 at George Bolton Parkway and flows east behind several industrial lots, before flowing west back towards RR50, and flowing alongside the road in existing ditches for approximately 200 m. The existing culverts along this section of RR50 show signs of deterioration and present a potential flood risk for surrounding properties. These culverts will need to be replaced due to safety concerns and to improve drainage flows.

As part of the EA study recommendations, a MUP is proposed along the west side of RR50, which will widen and replace the existing sidewalk where it exists. The proposed MUP is not expected to increase peak flows, as the overall increase in impervious area is negligible compared to the existing roadway, surrounding commercial lots and industrial lots. Due to the proposed alignment of the MUP some existing ditches along the west side of RR50 will be replaced by storm sewers with ditch inlet catchbasins.

The EA study has also recommended a sidewalk on the east side of RR50. The sidewalk ties in with the existing sidewalk at the front of property #12599 and runs south to the intersection of George Bolton Parkway and RR50. This recommendation was undertaken to address public concerns and pedestrian safety considerations.







2.0 Existing Site Conditions Characterization

2.1 Tributary Areas, Outlets, and Drainage Patterns

Catchment boundaries for the entire project area were put together through information received from the Region, Town of Caledon, and existing hydraulic assessment models. For the RR50 right-of-way, these areas were confirmed through localized road survey.

2.2 Condition of Receiving Watercourses

Existing watercourses receiving drainage from RR50, Healey Road, Mayfield Road and external areas within the project limits are located within the Humber River watershed, as well as the Rainbow Creek and Robinson Creek subwatersheds. These existing watercourses flow primarily across scattered woody riparian cover.

Rainbow Creek and Robinson Creek are intermittent watercourses regarded as warmwater habitat by the TRCA. However, the TRCA has indicated that all watercourses within the study area currently have a Redside Dace timing window (July 1st - September 15th).

2.3 Watercourse and Drainage Crossings

The existing storm drainage system of RR50 consists of roadside ditches and culverts as shown on drawings enclosed in Appendix B. As previously mentioned, the Region determined that a total of seventeen culverts require improvements. RVA inspected all culverts within the corridor and identified an additional twenty - three deficient culverts. RVA suggests that in total, thirty culverts may require improvements. The existing information for the roadway culverts is summarized in Table 2.1.







Table 2.1: Existing Roadway Culverts List

Crossing Number	Culvert ID	Material and Shape	Existing Diameter / Dimensions	Length (m)	U / S Invert (m)	U / S Edge of Travelled Lane	
RR - WD - 3.1	1	Circular CSP	750 mm	65.5	244.853	247.288	244.720
RR - WD - 6.1	2	Circular CSP	750 mm	92.6	244.310	245.882	244.000
RR - WD - 8.1	3	Circular CSP	900 mm	47.2	243.032	245.500	242.887
RR - WD - 10.1	4	Circular CSP	750 mm	51.8	242.399	243.730	241.796
RR - WD - 12.1	5	Circular CSP	900 mm	111.9	241.668	243.114	240.816
RR - WD - 14.1	6	Circular CSP	900 mm	10.9	240.733	242.240	240.500
RR - WD - 16.1	7	Circular CSP	900 mm	34.9	240.500	242.049	240.350
RR - WD - 18.1	8	Circular CSP	600 mm	22.1	240.150	241.594	240.024
RR - WD - 20.1	9	Circular CSP	900 mm	19.8	239.801	241.213	239.610
RR - WD - 22.1	10	Circular CSP	900 mm	43.1	239.305	241.000	238.672
RR - WD - 24.1	11	Circular CSP	750 mm	26.1	237.763	240.000	237.685
RR - WD - 26.1	12	Circular CSP	900 mm	17.2	236.790	239.000	236.340
2223.44 - 2.1	13	PVC	600 mm	15.1	236.763	238.500	236.668
2223.44 - 3.1	-	Concrete	900 mm	36.6	236.008	238.599	235.825
RR - WD - 28.1	14	Circular CSP	600 mm	23.2	236.823	237.600	236.730
RR - WD - 30.1	15	Circular CSP	500 mm	17.9	236.566	237.600	236.505
RR - WD - 46.1	16	Circular CSP	450 mm	25.6	228.659	231.000	228.328







2223.08 - 2.1	17	Concrete Box	4500 mm x 1700 mm	19.8	226.290	230.850	226.130
2223.11 - 2.1	18	Concrete Box	4500 mm x 1450 mm	19.0	226.700	229.750	226.520
RR - ED - 41.1	19	Circular CSP	525 mm	38.1	229.054	230.641	228.708
RR - ED - 38.1	20	Circular CSP	525 mm	23.8	229.745	230.898	229.376
2223.19 - 2.1	21	Metal Arch	2270 mm x 1600 mm	17.2	229.300	233.050	229.260
2223.22 - 2.1	22	Metal Arch	2060 mm x 1450 mm	24.1	230.150	233.400	230.060
2223.25 - 2.1	24	Metal Arch	2260 mm x 1660 mm	23.5	230.408	234.000	230.200
2223.29 - 2.1	25	Metal Arch	2900 mm x 1900 mm	16.7	231.178	234.850	231.142
RR - ED - 33.1	26	Circular CSP	600 mm	30.1	233.560	235.000	233.200
RR - ED - 31.1	27	Circular CSP	600 mm	9.1	234.112	236.000	233.966
RR - ED - 29.1	28	Circular CSP	525 mm	18.8	234.744	237.000	234.381
RR - ED - 26A.1	29	Circular CSP	450 mm	35.2	235.650	238.000	235.190
2223.44 - 4.1	30	Concrete Box	1800 mm x 1200 mm	34.2	235.825	238.500	235.654







2.4 Soil and Groundwater Conditions

Below the existing road surface, the general subsurface soils within the project area consist of fill material (sand and gravel, sand, gravelly sand, silty sand, sandy silt, silt, clayey silt, and silty clay). The fill layer extends to depths ranging from 0.6 m to 3.3 m below the ground surface.

Groundwater levels in the monitoring wells were measured by Thurber Engineering Ltd. between May 26, 2020 and August 24, 2020. The range of water level elevations in the monitoring wells were from 223.65 m to 243.67 m. The groundwater levels indicated that shallow groundwater flows follow local topography from northwest to southeast towards the tributary of Humber River. Hydraulic conductivity values were obtained through a series of slug tests carried out at some of the boreholes within the study area. The estimated hydraulic conductivity values range from 1.10 x 10 - 8 m / s to 9.80 x 10 - 10 m / s.

Guelph Permeameter testing was also carried out at nine locations along the grass boulevards on the west side of RR50. An infiltration rate was estimated by measuring the change in water level in the Guelph Permeameter reservoir once a steady state was reached. The infiltration rate was estimated to be in the range from 43 mm / hr to 101 mm / hr.

The subsurface information is described in detail within the hydrogeological report titled 'Preliminary Design for Drainage Improvements Regional Road Highway 50 from Mayfield Road to Healey Road' prepared by Thurber Engineering Ltd. dated October 2020.

2.5 Significant Natural Features

A Natural Heritage Report on existing conditions has been prepared by LGL Limited dated December 2020. The report indicated that the Land Information Ontario (LIO) and the Humber River Fisheries Management Plan has identified Robinson Creek as 'Small Riverine Warmwater' fish habitat. Brook Stickleback (*Culea inconstans*) and Creek Chuck (*Semotilus atromaculatus*) have been identified south of the study area. These two fish are a tolerant warmwater species.

The study area consists of naturalized vegetation communities of Dry - Moist Old Field Meadow (CUM1 - 1) and Mineral Shallow Marsh (MAS2 - 1 and MAS). These communities consist of high proportion of non - native and tolerant plant species. These plant species are well adopted to survive in areas that are







regularly disturbed and subjected to high light conditions. During LGL's botanical investigation no plant species that are regulated under the Ontario Endangered Species Act (ESA) or the Canada Species at Risk Act were encountered.

The report also indicated that the wildlife species identified within the study area are tolerant of anthropogenic features and disturbance. During LGL's investigation, ten birds were recorded that are protected under the Migratory Birds Convention Act (MBCA) and a single bird species is protected under the Fish and Wildlife Conservation Act (FWCA). In total there were eighteen wildlife species recorded within the study area. However, none of the wildlife species are regulated under the Ontario Endangered Species Act, 2007 (ESA) or the federal Species at Risk Act (SARA). Furthermore, LGL reviewed the National Heritage Information Center database (MNRF 2020) for rare species records but the study area contains no element occurrences for wildlife species at risk.

LGL limited is currently working on the second phase of the Natural Heritage Report which will indicate if the proposed condition impacts the natural heritage within the study area.







3.0 Stormwater Objectives

3.1 Water Quantity and Flood Control

The objective of the SWM report for drainage infrastructure improvements on RR50 from Mayfield Road to Healey Road is to assess the EA recommended solutions that will address the following:

- Ensure no increased risk of flooding to downstream properties and / or infrastructure.
- Design any proposed sewer to convey 10 year return period storm runoff.
- Where applicable, promote infiltration within the road right of way.

It is important to note that best management practices (BMP) were utilized to match the precondition of RR50 in the postcondition. Since no road widening is proposed along RR50 from Mayfield Road to Healey Road, there is a negligible increase in the percent of impervious surface present in the postcondition as compared to the precondition. As such, no water quantity or flood controls are proposed within the study area. However, wherever there is a risk of flooding due to undersized culverts this report proposes an upsize to existing infrastructure as part of the EA process.

3.2 Water Quality, Erosion, and Sediment Control

The objective for water quality and erosion and sediment control for this project is to provide best efforts to treat stormwater runoff from RR50. A combination of different Low Impact Development (LID) techniques to provide a basic level treatment (50 % Total Suspended Solids (TSS) removal) are proposed. To improve water quality in the postcondition Oil Grit Separators (OGS), bioretention facilities, infiltration trenches and catchbasin shields were included in the study recommendations. It is to note that no lane widening has been identified in the EA recommended alternative. As a result there is a negligible increase in the amount of impervious surface. Since the increase in imperviousness is negligible in the postcondition, the TSS loading is approximately the same as precondition. However, the improvements recommended through the EA are expected to improve stormwater quality in the postcondition, compared to precondition.

3.3 Water Balance

Water balance was not considered to be an objective for this project. Since no road widening is proposed along RR50 from Mayfield Road to Healey Road,







there is a negligible increase in the percent of impervious surface in the postcondition as compared to the precondition.







4.0 Model Build Process – Hydraulic Modelling

An InfoWorks model was created to model the creek, ditches, culverts, and sewers to assess the existing drainage conditions of RR50. The creek, ditch and culvert information were obtained from the TRCA's HEC - RAS model updated by Matrix Solutions Inc. The HEC - RAS model also included design flows for the creek. In addition, an InfoWorks model was also obtained from the Region. This InfoWorks model consisted of partial road subcatchment data as well as design storm and sewer information.

The InfoWorks model that was created by RVA combined the information obtained from the updated HEC - RAS and the existing InfoWorks models. Two scenarios were created in the RVA InfoWorks model for assessment: (1) the existing / precondition, and (2) the EA recommended condition. The following sections describe in detail how the RVA InfoWorks model was developed.

4.1 Design Storm

The design storm used in this project was obtained through the Region's InfoWorks Model. The design storm was developed using the Chicago Distribution storm for a duration of four hours using a five -minute time step. All sewers and roadside culverts were designed to convey the 10 - year design storm event. A 100 - year storm event was also modelled and run to understand the effect of flooding, if any, within the RR50 right - of - way.

4.2 Existing Condition Scenario

The existing condition was built in InfoWorks using two main sources: HEC - RAS and InfoWorks models, from the TRCA and the Region, respectively. The profile data of the creek was imported into InfoWorks by using the cross - section data from HEC - RAS. The inverts of the channels were assigned based on the cross - section inverts of the HEC - RAS model. In general, the creek has several culverts that allow conveyance. The information for these culverts was also imported as per the HEC - RAS model.

The Region's InfoWorks model did not include open profile roadside diches. Instead, the existing roadside ditches were modelled in a simplified manner as 1000 mm diameter circular pipes running along RR50. To be able to accurately capture the drainage capacity RVA used topographic survey data to model all the roadside ditches with their respective open profiles.







To the north of Healey Road there is a railway track that crosses RR50. The ditches along the railway track west of RR50 and the external drainage area draining to those ditches, were added to the Region's InfoWorks model since they contribute runoff and flows into the study area. Flows from these additional drainage areas contribute to the west side road ditch and enter the Robinson Creek culvert at George Bolton Parkway.

RVA performed an onsite investigation to determine the approximate external drainage areas that drain to these ditches and included them in the InfoWorks model. Since limited information about the outside drainage areas was available, runoff surface area parameters, such as imperviousness and runoff factors, were estimated. Flows from these areas were limited to a maximum runoff value into the west side ditch based on available rail crossing culvert capacity (750 mm diameter culvert, pfc = $1.08 \text{ m}^3/\text{ s}$, restricted 1 - year flows to approximately 500 l / s peak flow). This estimate was made based on the assumption that the rail line should not flood for a 10 - year design storm. Figure 4.1 below shows the extent of the described external drainage area.



Figure 4.1: External Drainage Area That Contributes into Study Area

A large culvert crossing exists at the intersection of George Bolton Road and RR50. Figure 4.2 below shows the drainage / culvert alignment. The flows from Robinson Creek enter a 600 mm diameter PVC storm sewer and merge after 15 m with a 900 mm diameter CSP pipe, that conveys additional flow from the west side of RR50 ditch drainage towards an 1800 mm x 1200 mm rectangular box culvert that crosses beneath RR50. The box culvert outlets into an online pond on the east side of RR50 and continues its flow path further as Robinson Creek.







A site investigation was also carried out to understand the online pond's drainage condition. The site investigation revealed that the inlet of the wet pond has a significant amount of silt and debris accumulated which causes a tail water condition and permanent submergence within the 1800 mm x 1200 mm rectangular box culvert. The outlet of the pond was also observed to be approximately 0.5 m higher than the inlet of the wet pond which leads to the silt and debris built up and standing water in the culvert.

RVA has added into the hydraulic model a storage node on the southwest corner (west of Esso Gas station) of George Bolton Parkway and RR50. This floodplain storage was added based on the observations made on the site to represent available floodplain storage flood volume that would be activated during the 100-year storm scenario. This storage would fill up and slowly raise the water elevation before reaching a level where RR50 would be overtopped. Figure 4.2 below shows the described 1800 mm x 1200 mm rectangular box culvert alignment. The road overtopping is predicted in the existing scenario model due to the applied steady peak flows from Robinson Creek. Such an overtopping scenario has not been reported from historic observations and this would depend greatly on the duration of an experienced extreme storm event and the upstream associated catchment wetting and runoff parameters.





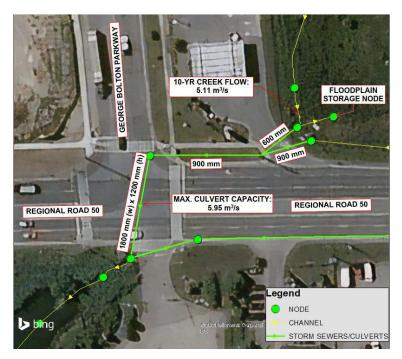


Figure 4.2: Existing Drainage Configuration at George Bolton Parkway
Intersection

4.3 EA Recommendations / Proposed Condition Scenario

The preferred solution to address active transportation (AT) enhancements as identified through the EA study includes the implementation of a MUP along the west side of the corridor and a sidewalk along the east side of RR50 north of George Bolton Parkway. In support of the recommended AT amenities, the preferred drainage infrastructure improvements include infiltration trenches and bioretention facilities on the west side of RR50 and culvert upsizing where hydraulic restrictions exist along the east side the corridor.

Bioretention facilities include bioswales and bioswale boxes. All three of these LID techniques were placed in locations, based on their implementation feasibility and hydrogeologic soil conditions. These features were modelled with infiltration rates that matched the test results taken from the locations considered. Bioswales are proposed in strategic locations where a bioswale box was not feasible to be implemented due to spatial constraints.

Bioswales are proposed on the west side ditch section RR - WD - 27.1 to RR - WD - 28.1 and RR - WD - 29.1 to RR - WD - 30.1. Within the bioswales, check dams are recommended to create a cascading system for runoff to flow along. This effect will decrease the runoff velocity and utilize the storage volume of the bioswale to promote settling. The cascading system of check dams will ensure







adequate infiltration within the proposed bioswales. A bioswale box is proposed south of the Simona Drive and RR50 intersection. This bioswale box is modelled with a depth of 1 m and a 201.60 m² surface area. An OGS is also proposed at the west side of RR50, north of and the Mayfield Road intersection.

4.3.1 Recommended Sidewalk on East Side of RR50 (North of RR50 and George Bolton Parkway)

As part EA process, an approximately 400 m long sidewalk on the east side of RR50 is recommended. The sidewalk starts at the front of the property #12559 RR50 and ends at the intersection of RR50 and George Bolton Parkway. There are currently roadside ditches in place of the proposed sidewalk alignment. These ditches convey external drainage area flow into the Robinson Creek. Due to the sidewalk construction, the roadside ditches are proposed to be filled and a storm sewer system is proposed to convey the 10-year storm flow without surcharge and the 100-year storm flow without flooding.

North of RR50 and George Bolton Parkway intersection there are multiple catch basins that capture 10-year road drainage flow from RR50 and convey these into the existing 825 mm diameter storm sewer system. The existing storm sewer system conveys the road drainage flows into the Robinson Creek, outletting at RR50 and George Bolton Parkway intersection into the online pond. It is important to note that the existing 825 mm diameter storm sewer system does currently not convey any external drainage area other than the road drainage.

As such, a parallel storm sewer system is recomended, ranging from 450 mm to 825 mm diameter to connect to the existing 825 mm diameter storm sewer system. From the connection point of the new storm sewer with the existing 825 mm diameter storm sewer, approximately 181 m of 825 mm diameter pipes need to be upgraded to 1050 mm to 1200 mm diameter to be able to convey the added flows without surcharge during a 10-year storm event into the pond. This pipe upgrade ensures that only one outfall into the pond will remain. All road catchbasins currently connected to the existing storm maintenance holes will be kept as per current drainage arrangement.

The extent of the external drainage area and the proposed and the existing storm sewer system are shown in Figure 4.3.







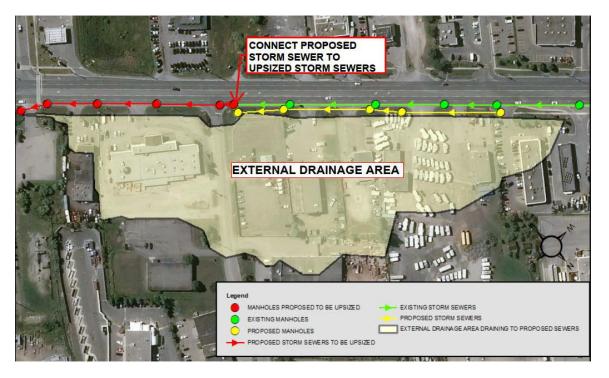


Figure 4.3: External Drainage Area & Proposed Storm Sewer System (North of RR50 & George Bolton intersection)

4.3.2 Culvert Crossing at RR50 and George Bolton Parkway

Due to anticipated high peak flows in Robinson Creek, the existing 600 mm / 900 mm diameter inlet pipes are insufficient to convey the proposed flows and require upsizing. The existing box culvert crossing (1800 mm x 1200 mm) has an available full pipe capacity of approximately 5.95 m³/s and is in itself capable to convey the 10-year design storm flow without flooding or overtopping the road. However due to the upstream pipe capacity limitations, the current model scenario shows road overtopping.

Based on the available HEC - RAS flows, the road crossing is proposed to be enhanced with a consistent 1800 mm x 1200 mm box culvert crossing along the existing pipe alignment. Figure 4.3 shows the proposed culverts at the George Bolton Parkway and RR50 intersection.









Figure 4.4: Creek Bypass Enhancements at George Bolton and RR50

RVA carried out a site investigation to understand the hydraulic situation and condition of the online pond into which the existing 1800 mm x 1200 mm box culvert outlets. It is proposed to remove the debris and silt built up at the inlet of the online pond. To avoid future built up at the inlet again, it is also proposed to lower the outlet of the online pond by approximately 0.5 m. Figure 4.5 shows the online wet pond and proposed maintenance required to mitigate tailwater condition at the existing 1800 mm x 1200 mm box culvert.

In order the validate the provided flows, RVA has analyzed flow monitoring and rain gauge data at the inlet of the online pond. The results and discussion of this analysis is provided in Section 8.0. It is also important to note that the Region is currently undertaking the preliminary condition assessments of these culverts at this intersection.







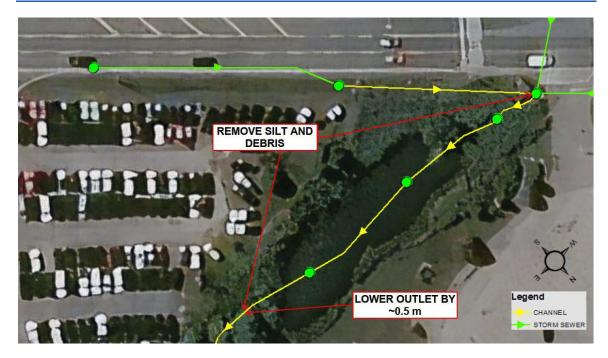


Figure 4.5: Proposed Maintenance at online wet pond

4.3.3 Flooding Complaints at # 12207 RR50

During the study's public consultation period, a flooding complaint was received pertaining to property # 12207 RR50. The problem was described as backyard property flooding due to high water levels in Robinson Creek. Robinson Creek turns away from the roadside and flows around the backyard of this property before realigning back to the east side of RR50. In relation to the flooding report, the property owner mentioned a former roadside ditch to the front of his property had provided flood relief and a creek bypass, but this was replaced by the Region with a 525 mm diameter sewer to fill the ditch.

In addition, directly opposite this flooding issue, a recent residential development was created with a Storm Water Management (SWM) pond with a controlled discharge point directly into the creek.

RVA undertook site investigation works to better understand the flooding mechanism and reviewed the SWM report and drawings of the pond to understand the discharge flow rate from the pond to the creek. The SWM pond is proposed to discharge at $0.18~\text{m}^3/\text{s}$ and $0.34~\text{m}^3/\text{s}$ for 10- year and 100- year, respectively. Compared to the HEC - RAS flows in the creek of $5.57~\text{m}^3/\text{s}$ and $8.62~\text{m}^3/\text{s}$ for 10- year and 100- year, respectively, the proposed SWM pond discharge rates are negligible. Excerpts from the SWM report have been attached in Appendix A. As such, it can be understood that the flooding at #







12207 RR50 is not due to the discharge rate from the SWM pond. Figure 4.6 illustrates Robinson Creek, the flood property, and the creek bypass channel.

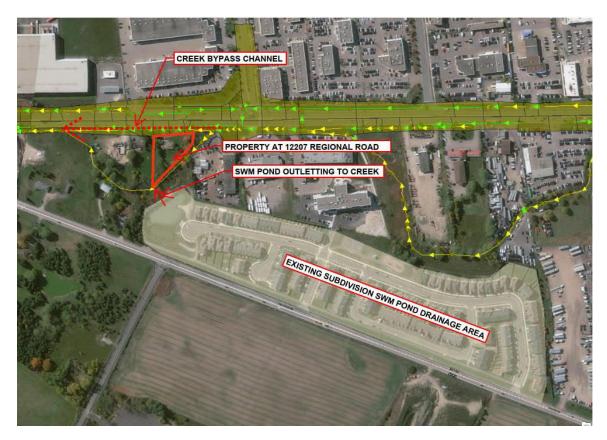


Figure 4.6: Robinson Creek, Flood Property and Creek Bypass Channel

RVA has designed increased capacity for flooding relief of this property. The bypass level between the creek and the relief pipe was adjusted as well as the relief pipe being upsized from 525 mm diameter to 900 mm diameter.







4.3.4 Proposed Culvert Upgrades

The following culverts are proposed to be upgraded:

Table 4.1: Proposed Culvert Sizes and Shapes

Crossing Number	Existing Material and Shape	Existing Diameter / Dimensions	Proposed Material and Shape	Proposed Diameter / Dimensions
2223.44 - 2.1	Circular PVC	600 mm	Concrete Box	1800 mm x 1200 mm
2223.44 - 3.1	Circular CSP	900 mm	Concrete Box	1800 mm x 1200 mm
2223.25 - 2.1	Metal Arch	2260 mm x 1660 mm	Concrete Box	3000 mm x 1500 mm
2223.22 - 2.1	Metal Arch	2060 mm x 1450 mm	Concrete Box	3000 mm x 1500 mm
2223.19 - 2.1	Metal Arch	2270 mm x 1600 mm	Concrete Box	3000 mm x 1500 mm
RR - ED - 38.1	CSP	525 mm	Concrete	900 mm
RR - ED - 39.1	CSP	525 mm	Concrete	900 mm
RR - ED - 40.1	CSP	525 mm	Concrete	900 mm
RR - ED - 41.1	CSP	525 mm	Concrete	900 mm







5.0 Model Results

5.1 Existing Condition Scenario Results

The existing condition scenario was tested for a 10 - year storm and a 100 - year storm event to analyze the effects of a design storm and a major storm on the culverts along RR50.

The results indicate that under a 10 - year storm the culverts under George Bolton Parkway and RR50 flood. The existing culverts on this intersection are 600 mm diameter PVC pipe, which is connected to a 900 mm diameter CSP culvert. The 900 mm diameter pipe connects to an 1800 mm x 1200 mm concrete box culvert that crosses RR50 and outlets into the online pond. Robinson Creek flows through this pipe/culvert arrangement. Based on the created InfoWorks model, for10-year storm and 100-year storm events under existing conditions, there is road ponding of 230 mm and 311 mm respectively predicted for the RR50 and George Bolton Parkway intersection. Figure 5.1 represents the culverts under the George Bolton Parkway and RR50 intersection. Based on TRCA's HEC - RAS model, the 10 - year flow at the intersection is 5.10 m³/s and the InfoWorks model predicts hydraulic restrictions due to the small culverts (600 mm and 900mm) under the intersection.



Figure 5.1: George Bolton Parkway and RR50 Intersection Culverts

For a location further south near Parr Boulevard, a flooding complaint was received during the public consultation meeting. The complaint is related to







property # 12207 RR50. Hydraulic details were added and analyzed in the existing model.

The hydraulic situation around this property can be explained as follows. Robinson Creek departs from the RR50 road alignment and runs around the property. Between the property and RR50, a former roadside ditch, that acted as flow relief to bypass high flows in Robinson Creek was filled and replaced with a 525 mm diameter sewer of lesser conveyance capacity. Compared with the former drainage situation, the water level in the creek rises due to decreased relief capacity from the replaced ditch sections with 525 mm diameter storm sewers. The water level in the adjacent creek section ranges from 230.25 m to 229 m for the 10-year storm flows and 230.5 m to 229 m for the 100-year storm flows. Since the property elevation is within 230 m to 231 m, RVA can confirm that a flood risk currently exists at property # 12207 RR50. This flood risk might have been increased due to the filling of the ditch and replacement with a 525 mm diameter sewer which provides less flow capacity and flow relief than the previously existing ditch sections. In addition, the existing 525 mm storm sewers surcharge during a 10 - year storm event.

For a section of Robinson Creek that is located upstream of the above described flood complaint property, the following hydraulic capacity restrictions exist. Under a 100 - year storm, there are three culverts that run along RR50 that show hydraulic restrictions to convey the 100 - year storm flows. The three culverts are located near the Parr Boulevard and RR50 intersection and their crossing ID numbers are 2223.22 - 2.1, 2223.25 - 2.1 and 2223.19 - 2.1. These culverts are made of corrugated metal arch culverts with sizes 2060 mm x 1450 mm, 2260 mm x 1660 mm and 2270 mm x 1600 mm. Figure 5.2 highlights the three culverts that show hydraulic restrictions in the existing condition.







Figure 5.2: Culverts on RR50 and Parr Boulevard Showing Hydraulic Restrictions Under 100 - year flows

The InfoWorks model output results are presented in Appendix B.

5.2 EA Recommended / Proposed Condition Scenario Results

The EA recommended condition scenario was set up to analyze the proposed drainage infrastructure improvements within the RR50 corridor. LID design elements were also proposed and were integrated into the model to promote sustainable drainage features.

The proposed condition model performs well under the established 10 - year design storm scenario and does not show any culvert capacity restrictions along the study area section for Robinson Creek and Rainbow Creek. The roadside drainage features that consist of ditches, pipes, swales, and infiltration trenches / chambers are all performing as expected and within their capacity limits. Under the 10 - year design storm, all the proposed culverts under the George Bolton Parkway and RR50 intersection convey the 10 - year flows without ponding or overtopping the intersection. Under the 100-year design storm the InfoWorks model predicts a ponding depth of 123 mm at the intersection. This represents a ponding depth reduction of approximately 100 mm from the existing condition. Since the predicted flow depth over the road depends on the width of the spill path and is difficult to properly quantify with a one-dimensional model, the result is an approximate value that is based on best available topographic data. Since







no historic overtopping at the road was observed for the existing conditions and the proposed pipe upgrades show a significant improvement in drainage conditions, the likelihood of actual road overtopping to occur is very limited. Further considerations are made with regards to peak flows in Robinson Creek under section 8 of this report. Several meetings were held with the TRCA on the peak flow prediction of Robinson Creek and the current presented peak flows represent already a reduction, compared with original supplied hydrologic data. Further flow measurements during observed rainfall events in 2021 were undertaken to relate measured flows and try to upscale and compare them to the current predicted peak flows, since it is observed that the predicted peak flows might be excessive and overpredict current conditions.

The 525 mm CSP sewers at the front of property # 12207 RR50 are upsized to 900 mm concrete sewers in the proposed condition. The model predicts that since the sewer capacity has been increased, the flows in the creek decreases in the post condition compared to the precondition. The proposed 900 mm concrete sewers also do not surcharge during a 100 - year storm event. The table below summarizes the pre and post flows of the creek at the back of # 12207 RR50.

Table 5.1: Creek Flow Comparison (at back of # 12207 RR50)

Cross - Section Number	Existing (10 - Year) (m³/s)	Proposed (10 - Year) (m³/s)	Existing (100 - year) (m³/s)	Proposed (100 - year) (m³/s)
2223.16 - 1.1	5.57	4.72	8.63	7.68
2223.15 - 1.1	5.57	4.72	8.63	7.68
2223.14 - 1.1	5.57	4.72	8.63	7.68

The three culverts near Parr Boulevard and RR50 also show no hydraulic restriction or flooding during a 100 - year storm event. It is important to note that these proposed concrete box culverts of size 3000 mm x 1500 mm operate at 60-90% capacity during the 100 - year storm event.

The InfoWorks model output results for the proposed condition scenario are presented in Appendix B.







6.0 SWM and LID Features Plan and Design

6.1 Evaluation and Selection of Candidate Features

In support of the evaluation process that was undertaken as part of the Class EA study, the Region's *LID Implementation Process for Regional Road Right - of – Ways* was utilized in developing the most applicable Low Impact Development (LID) technique(s) for this project. Based on a thorough review of available information and evaluation against all relevant criteria during the EA process, it was determined that a combination of underground storage elements combined with infiltration elements and Oil and Grit Separator (OGS) units were feasible options for this site.

Table 6.1 below indicates the opportunities and constraints for each of the applicable LID techniques relevant to Regional Roads within the Region.

Table 6.1: LID Practice Analysis

Project Type	LID Practice	Constraint / Opportunity
		Constraint
Regional Road Works (Urban)	Stormwater Management (SWM) Pond	 Space is not available within the road right - of - way. The adjacent properties within the project area are either farmlands or developed sites. To meet the Region's SWM initiative, multiple SWM ponds will be required at each existing culvert. Buying a large amount of property to build SWM ponds is not an economical solution.
	Bioretention Facilities – Bioswales and Bioswale Box	 Opportunity Can be designed with overflow capacity and can provide surface conveyance for flows. Water retention can be designed above the capacity for the filtration element to account for emergency overflows. Higher costs for soil remediation and maintenance.







Project Type	LID Practice	Constraint / Opportunity		
Regional Road Works	Bioswales and Bioswale Box			
(Urban)		Constraint		
		 Permeable surfaces can be provided along the MUP. 		
		 Runoff water from boulevard is relatively clean and from a small area. 		
	Permeable	 If provided along the MUP, periodic cleaning is required to maintain drainage properties. 		
	Pavement	 No costs associated with property requirements. 		
		 Higher maintenance costs to maintain permeability. 		
		 Will not meet the drainage requirements due to capacity restrictions. 		
		Opportunity		
	Oil / Grit	 OGS units can be used due to their smaller footprint and treatment design flexibility for treatment area size. 		
	Separator Units (OGS)	 OGS units need to be designed as part of a multi component approach to achieve water quality treatment targets. 		
		Constraint		
	Superpipe Storage	 Shallow outlet points will not allow water to drain completely. Larger pipes will require the proposed road profile to be raised. Does not provide the quality controls that the Region and other agencies are looking for. 		







Project Type	LID Practice	Constraint / Opportunity			
Regional Road Works (Urban)	Infiltration Trenches	 Opportunity Underground storage / infiltration arches such as those manufactured by Terrafix, Stormtech or Cultec can be used to detain and infiltrate stormwater. Can be used underneath pavement. Clean out manholes provide the opportunity to clean out sediment without removal or any pavement. 			
	Enhanced Roadside Ditches	 Constraint Not compatible with adjacent land use. Enhanced swale would require significant adjacent property. Not practical within the RR50 corridor due to constant water flows and large storm flows. Significant property required to widen ditches for enhanced swales. 			
	Catchbasin Capture Devices / (Catchbasin Shields)	 Opportunity Does not impact existing flow capture and conveyance. Provides some treatment benefits by removing larger TSS particles at catchbasin locations. Compatible with adjacent land use. No cost associated with property requirement. Standard maintenance costs anticipated. 			







As indicated above, the selected feasible LID techniques that are recommended for incorporation in the SWM Design for RR50 from Healey Road to Mayfield Rd are:

- Oil / Grit Separator Units (OGS)
- Bioretention Facilities
- Infiltration Trenches
- Catchbasin Capture Devices / Catchbasin Shields







7.0 Drainage Plan and Design

7.1 Minor System Design

The minor drainage system consists of storm sewers, roadside ditches, and driveway culverts. The systems were designed to convey the 10 - year storm event for the RR50 subcatchments and external contributing drainage areas.

7.2 Major System Design

The major drainage system consists of the minor drainage system and the overland flow routes which convey excess runoff above the minor drainage system's capacity. The major drainage system was designed to convey the peak runoff flow from the 100 - year storm events.

7.3 Monitoring and Maintenance

To allow the stormwater maintenance facilities to function properly, the following monitoring and maintenance program is recommended by the MECP and TRCA guidelines.

The storm drainage systems should be maintained at regular intervals by inspecting and cleaning the sumps of catchbasins and maintenance holes as well as the OGS structures. In addition, infiltration trenches should be flushed out and any collected sediment should be removed via vacuum truck. The following sections describe the operations and maintenance requirement for each type of LID systems proposed within the study area.

7.3.1 Oil / Grit Separator (OGS)

With regards to monitoring and maintenance Imbrium System's OGS manual was reviewed. However, in the detailed design stage any OGS that is approved and equivalent to Imbrium System's OGS can be used. The manual by Imbrium System suggests that the inspection of the EFO® filter units should be carried out over the first year on a regular basis to inspect and assess sediment accumulation. Inspection in subsequent years should be based on the inspection schedule established based on the results on the first year. It is also important to note that the inspections should be performed immediately after oil, fuel or other chemical spills that take place within the area and drain to the OGS.

Task of inspection includes the removal of manhole covers, inspection of sediment buildup using a sediment probe with bulb valve or sludge judge, and an







oil dipstick for oil inspection. The remaining tasks are primarily visual. Inspection and assessment of the unit performance can be logged using the sample maintenance logs provided in the EF Owner's manual. A copy of the owner's manual is attached in Appendix C.

7.3.2 Catchbasin Capture Devices / Catchbasin Shields

A Catchbasin Shield Operation manual was reviewed as part of this report. Inspecting a Catchbasin Shield should be done by opening the grate and then attaching a lifting rope to the top of the centered leg of the Catchbasin Shield insert. A Sludge Judge should be used to measure the sediment depth in four to six locations of the sump. The unit is recommended to be cleaned if the sediment depth is 300 mm – 600 mm. A copy of the owner's manual is attached in Appendix C.

7.3.3 Bioretention Facilities: Bioswales and Bioswale Box

Based on the 'Low Impact Development Stormwater Management Planning and Design Guide: Version 1.0,' developed by the Credit Valley Conservation (CVC) and TRCA, the proposed bioswales and bioswale box need to be maintained to ensure that the infiltration and water quality benefits are preserved. At the bioretention facility locations, routine roadside ditch maintenance practices to be avoided are scraping and regrading. In addition, vehicles should not be parked or driven on the bioswales and bioswale boxes. If routine mowing takes place, then it should be carried out using the lightest possible mowing equipment to prevent soil compaction.

After every major storm event (> 25 mm) and quarterly for the first two years the vegetation density needs to be inspected to ensure at least 80 % coverage exists, to observe if vegetation has been damaged due to foot or vehicular traffic, as well as, for channelization and accumulation of debris, waste, or sediment. After two years inspections are required twice annually.

At least twice annually during the first two years the proposed bioswales and bioswale box need to be regularly watered while vegetation is becoming established. The bioswales need to be mowed to ensure the height is between 75 mm to 150 mm.

Annually the proposed bioswales and bioswale box need to be inspected for dead vegetation, invasive growth, dethatching, thatching, and aerating. Dead vegetation needs to be replaced. Also, any erosion must be repaired. If the







sediment within the bioretention facilities exceeds 25 mm depth, then it must be removed when dry.

7.3.4 Infiltration Trenches (Stormtech)

Maintenance hole inspections should be carried out to observe if trash, debris, or pipe blockages have occurred. More thorough inspection should be conducted if vacuuming and removal of sediment or nondraining water are required. During the first two years of operation, inspections should be made after every significant storm event (> 25 mm) to ensure proper functioning. On an average about four inspections are required every year for the first two years.

After the first two years, the infiltration trenches should be inspected on a regular basis, typically twice per year, and maintained as required. The maintenance frequency should be based on site specific characteristics and driven by the amount of runoff and pollutant loading encountered by the system. Typically, maintenance intervals for the proposed infiltration trenches would be 5 years.







8.0 Flow Monitoring and Rain Gauge Data Analysis

Meetings were held with TRCA and Peel Region to discuss the perceived overprediction of the peak flows in Robinson Creek that are based on the TRCA's HEC-Ras model. The reason for this observation was, that flows at the road intersection of George Bolton Parkway and RR50 were predicted to overtop the road even in 5-year and 10-year storm events and the creek peak flows did exceed the available culvert conveyance capacity. No such observations were made in the past and no flood complaints exists for this road section or the creek.

TRCA agreed to reduce the duration of the design storm to lessen the peak design flows for the creek. In addition, it was agreed between Peel Region and RVA to conduct further flow monitoring to gain a better understanding of the creek flow response to rainfall events and based on the results, to hopefully be able to extrapolate flow data to give an indication for the TRCA predicted extreme storm peak flows and either confirm these or find out if these flows are still conservative.

For this purpose, Advanced Monitoring Group (AMG) Environmental installed a flow monitor and rain gauge at the RR50 and George Bolton Parkway intersection. The flow monitor and the rain gauge were installed at the outlet of the existing 1800 mm x 1200 mm rectangular box culvert on April 20, 2021. RVA has obtained and analyzed flow monitoring and rain gauge data between April 20, 2021 and July 31, 2021. The monitored data was measured under a 5-minute timestep.

The monitored data analysis revealed that there were four (4) significant rainfall events that happened during the monitored period. For the purpose of this analysis, any flow recorded below 200 L/s was considered to be insignificant. The four (4) significant rainfall events occurred twice in July and once in September and October. The peak flow ranged from 200 L/s to 652 L/s. The most significant peak flow was observed in the month of July. The below graph, Figure 8.1, shows the recorded flow data and rainfall data between the period July 6, 2021 20:50 to July 8, 2021 16:00 (2 rain events).

During the aforementioned period in July, two independent rainfall events occurred. The first rainfall event produced a volume of 10.16 mm with a peak intensity of 30.48 mm/hr. As there was no previous rainfall event, the flow







monitor recorded approximately 150 L/s from the drainage area. However, the second rainfall of 21.34 mm of volume and a peak intensity of 45.72 mm/hr produced approximately 652 L/s of peak flow. The high peak flow of 652 L/s is caused due to the already wet drainage area due to previous rainfall approximately 5 hours before. This indicates, that catchment wetting and storm duration have a significant impact on the peak flow outcome.

In October 2021, there was also a significant rainfall that was captured by the rain gauge. Figure 8.2 illustrates the measured rainfall and the flow response from the drainage area. The graph shows that there was a 23.37 mm rainfall that occurred with an intensity of 36.58 mm/hr. The duration of the rain event was approximately 4 hours. The peak flow that was recorded during this event was approximately 460 L/s.







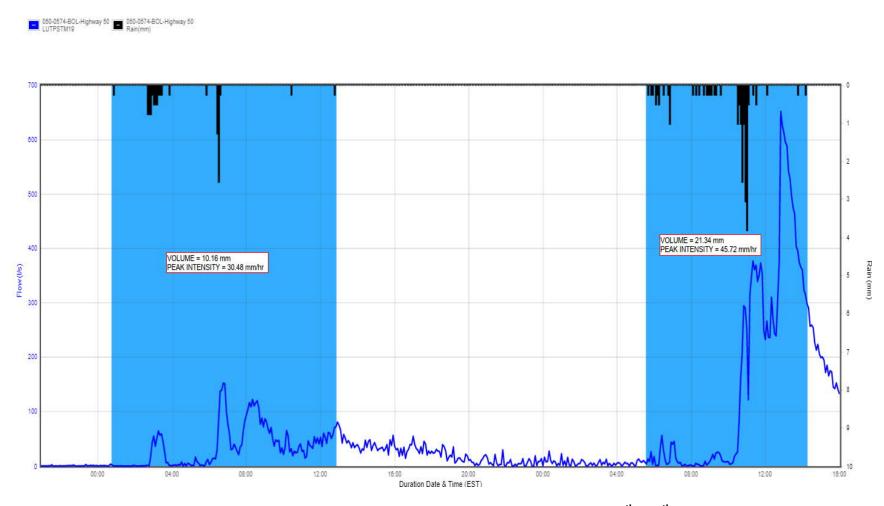


Figure 8.1: Measured Rainfall and Flow Data on July 6th – 8th







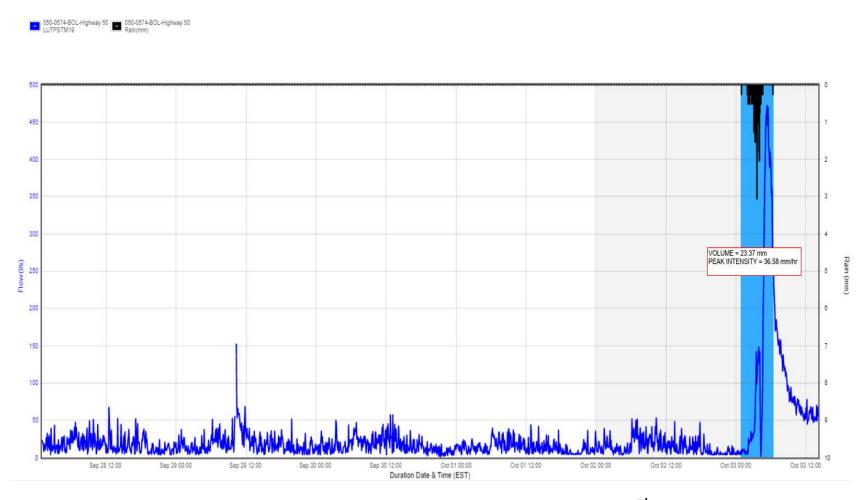


Figure 8.2: Measured Rainfall and Flow Data on October 3rd







The TRCA has revised the HEC-RAS flows, since RVA's previous SWM brief for this project, by changing the design storms from 6-Hour AES to 12-Hour AES. Email correspondence with TRCA is attached in Appendix A. The 12-Hour AES storm was analyzed to calculate the total volume for a 2-year storm. The calculation shows that under a 2-year 12-Hour AES storm, the total depth of rainfall would be approximately 42 mm. To compare the TRCA's provided 2-year flow of 3.11 m³/s and the measured flow, RVA compared the closest rainfall event that occurred for approximately 12 hours with approximately volume of 42 mm. Between September 22nd, 2021 10:25 AM and September 22, 2021 22:45 PM, a total volume of 41.66 mm rainfall was recorded with a peak intensity of 42.67 mm/hr. The graph **below**, **Figure 8.3**, **shows the** accumulated rainfall (total depth) and observed flow under an approximately 12-hour storm duration.







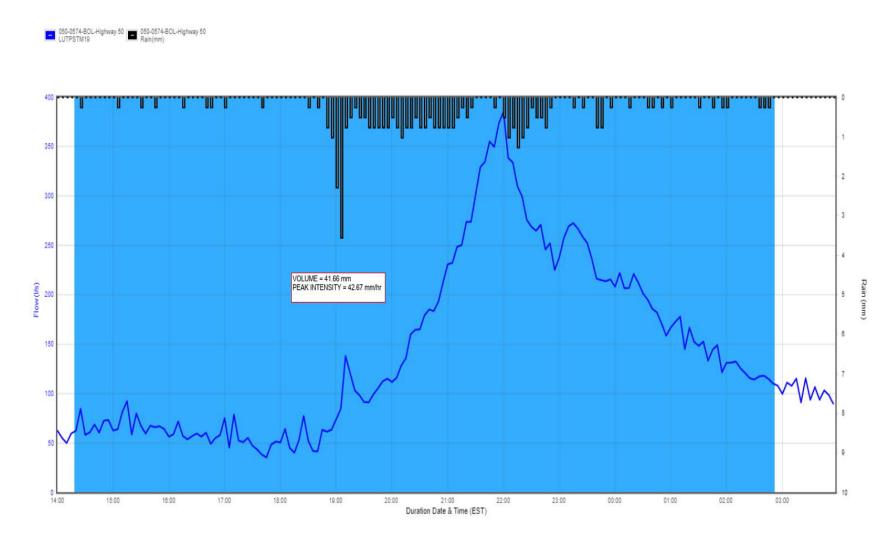


Figure 8.3: 12-Hour Rainfall Data and Measured Flow Data During September 22nd, 2021







The graph above shows that an approximately 42 mm depth of rainfall over a 12-hour period results into an approximately ~375 L/s of recorded flow. Since, the accumulated rainfall is approximately same as the 2-year 12-hour AES storm total rainfall depth, it can be inferred that the flows provided by TRCA are overestimated. This could be due to the overestimation of the drainage areas and controlled site plan areas which may not have been included in the hydrologic model used to estimate the provided flows to RVA.







9.0 Recommendations, Approval and Review Requirements

The proposed SWM measures outlined in this report have been developed in support of the recommendations developed though the Class EA planning and design process. The recommended infrastructure upgrades outlined in this document are expected to perform without flooding with the current TRCA's provided flows. Although it should be noted that based on the flow monitoring and rain gauge data analysis, RVA finds the flows provided by TRCA to be overestimated. However, the proposed culvert upgrades on RR50 and George Bolton Parkway are sized adequately to perform under a 10-year without overtopping and during 100-year with an approximately 123 mm ponding with the TRCA provided flows.

The Stormwater Management (SWM) Report is subject to review and approval from the following regulatory agencies:

- The Region of Peel
- Toronto and Region Conservation Authority (TRCA)
- Town of Caledon
- Ministry of Environment, Conservation and Parks (MECP)
- Public interest groups and stakeholders

The implementation of the recommended storm drainage systems described within this report has been developed in support of the EA recommendations and will control the site's runoff in accordance with the Region and TRCA.

Report Prepared By:

R.V. Anderson Associates Limited

Sadman Soumik, MASc., EIT Engineer-in-Training, Hydraulic Modeler Oliver Olberg Manager of Hydraulic Modelling







Appendix A

Background Information







STORMWATER MANAGEMENT IMPLEMENTATION REPORT

BOLTON GATEWAY DEVELOPMENTS INC.

TOWN OF CALEDON REGION OF PEEL

PREPARED BY:

C.F. CROZIER & ASSOCIATES INC. 2800 HIGH POINT DRIVE, SUITE 100 MILTON, ON L9T 6P4

> SEPTEMBER 2014 REVISED MARCH 2015

CFCA FILE NO. 649-3357

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Table 4: Proposed Controlled Peak Flow Rates

Return Period (Years)	Maximum Allowable Release Rate (Per Table 1) (m³/s)	PROPOSED SWM FACILITY		
		Proposed Discharge (m³/s)	Active Storage Required (m³)	Active Storage Provided* (m³)
2	0.16	0.06	1660	5770
5	0.21	0.13	2200	
10	0.24	0.18	2570	
25	0.29	0.25	3050	
50	0.34	0.30	3420	
100	0.38	0.34	3790	
Regional	¥	1.23	4720	

*Note: Active storage is measured from the permanent pool volume, elevation 229.1m, to the top of berm, elevation 230.60m

With the use of the proposed SWM facility, post development peak flows will respect the target flow rates for Robinson Creek for storms up to and including the 100 year event.

3.4 Stormwater Quality Control Requirements

As previously noted, the site drains to Robinson Creek and therefore is required to meet Enhanced level of protection (80% TSS removal) as defined by the MOE SWMPD Manual.

A wet pond has been selected as the preferred end of pipe treatment facility for the proposed development. Based on the MOE Stormwater Management Planning and Design Manual (2003), the following volumes are required for the contributing 8.1 ha site development area (65% composite imperviousness):

Permanent Pool: 1404 m³
 Extended Detention: 1098 m³

A composite imperviousness of 65% imperviousness is based on the conservative value of 68% imperviousness for catchment 201, and an imperviousness of 20% for catchment 202. It is to be noted that the contributing area from the Pannia lands have been excluded from the water quality calculations given that it is not the responsibility of the proponent to provide water quality control for external lands. Refer to **Appendix D** for sizing calculations and **Section 4.1** for the provided storage volumes in the SWM facility.



Sadman Soumik

From: Jairo Morelli < Jairo.Morelli@trca.ca>

Sent: February 24, 2021 12:33 PM

To: Andrew McGregor; emma.benko@trca.ca

Cc: Banuri, Syeda; Oliver Olberg; Peter Cho; Matthew de Wit; Sadman Soumik; Dilnesaw Chekol

Subject: FW: Hwy 50 Drainage Improvements Class EA

[CAUTION EXTERNAL EMAIL] Make Sure that it is legitimate before Replying or Clicking on any links

Hi Andrew

Staff had the opportunity to review the HEC RAS model and peak flows associated with the above-noted project. It appears that the flows resulting from the Humber River hydrology update are correct and consistent with those used in the hydraulic model.

We run a series of scenarios to simulate the conveyance of flow through the complex crossing at Geroge Bolton and Highway #50 and the analysis demonstrated that the rating curve used in the model is acceptable. The fact that flooding has not been frequently recorded within the intersection may be among others due to:

- The model is run in a steady-state assuming an unlimited supply of water to represent worst-case scenarios.
- The average ground elevations along the curb (parallel to the west side ditch =238.70m), appear to be a bit higher than the one assumed for the centreline of the road (238.50m). Please refer to the cross-sections shown below, which illustrate ground elevations across the intersection, cut from left to right looking north.



Photos - Geroge.png

+ Add to See all photos RAS Mapper File Project Help Tools Selected Layer: Cross Sections RASMapper Plot ☐ ☐ Features - Profile Lines Table ☐ ✓ Geometries B Robinson Creek Extension **Terrain Profile Plot** ⊕ Rivers Storage Areas - Terra 238.6 H- 20 Flow Areas. ⊕ Bridges/Culverts ⊕ □ Inline Stroctures **★**- □ Lateral Structures ₱- S4/2D Connections 238.4 - Aump Stations □ BC Lines Manning's n Elevation [m] 238.2 ⊕- □ /ofi/tration : + Percent Impervious Reference Points Errovs ☐ Robinson Creek Extension Rivers **★** Cross Sections Storage Areas ● 20 Flow Areas 237.8 ⊕ Bridges/Culverts Inline Structures Lateral Structures **⊕**- □ S4/20 Connections. **⊕**- Pump Stations 237.6 BC Lines ⊕ ∏ Marmoy's n ◆ Lotifization RASMapper Plot Percent Impervious - Reference Points □ Enovs Table ☐ Event Conditions **⊟** ✓ Results **Terrain Profile Plot** T C (C ... (V ...)

2



File Options Help 94.71, 237.38 River: CEG_DESIGN + 100 Reach: REACH1 ▼ River Sta.: 2223.44 Robinson Creek Extension Project Plan: Plan 03 2/22/2021 .025 .025 239.5 35 239.0 WS 2-Year 238.5 (58.27, 238.53) 238.0 Elevation (m) 237.5 237.0 236.5



TRCA staff run delineation using high-resolution LiDAR topography and the results show that generally, the drainage areas included in the hydrology model are more or less similar with this delineation except for some minor changes.

TRCA has no concerns, in case your team would like to take a closer look at the hydrology and/or hydraulic model. If that is the case, please let us know if your findings suggest further revisions to the models.

Due to staff and resource constraints, TRCA has a list of criteria to determine installations of stream gauges across its jurisdiction. The idea of installing a new stream gauge at the project site was discussed with the department that is responsible for the installation of stream gauges and the team indicated the site, does not satisfy the majority of the criteria. However, if the Region has an interest in installing and running it, the TRCA would be willing to provide advice.

Please contact me should you have any questions or concerns.

Regards

Jairo Morelli

Andrew McGregor < AMcGregor@rvanderson.com>; Emma Benko < emma.benko@trca.ca>

From: Andrew McGregor < AMcGregor@rvanderson.com >

Sent: Thursday, February 4, 2021 12:45 PM

To: Jairo Morelli <Jairo.Morelli@trca.ca>; Emma Benko <emma.benko@trca.ca>

Cc: Banuri, Syeda <<u>syeda.banuri@peelregion.ca</u>>; Oliver Olberg <<u>OOlberg@rvanderson.com</u>>; Peter Cho <<u>pcho@rvanderson.com</u>>; Matthew de Wit

 $<\!\!\underline{MdeWit@rvanderson.com}\!\!>; Sadman Soumik <\!\!\underline{ssoumik@rvanderson.com}\!\!>$

Subject: RE: Hwy 50 Drainage Improvements Class EA ~ Meeting Availability

Thanks Jairo,

The SWM report is attached. Let us know if you need anything else.

Kind regards,



RVA IS GROWING!

Our NEW <u>Halton</u> and <u>Halifax</u> offices are now open.







Andrew McGregor, MCIP, RPP

Senior Planner, EA & Approvals

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From: Jairo Morelli < Jairo. Morelli@trca.ca >

Sent: February 4, 2021 11:01 AM

To: Andrew McGregor < AMcGregor@rvanderson.com>; emma.benko@trca.ca

Cc: Banuri, Syeda <syeda.banuri@peelregion.ca>; Oliver Olberg@rvanderson.com>; Peter Cho <pcho@rvanderson.com>; Matthew de Wit

< <u>MdeWit@rvanderson.com</u>>; Sadman Soumik < <u>ssoumik@rvanderson.com</u>> **Subject:** RE: Hwy 50 Drainage Improvements Class EA ~ Meeting Availability

[CAUTION EXTERNAL EMAIL] Make Sure that it is legitimate <u>before</u> Replying or Clicking on any links

Hi Andrew

We will review the HEC RAS model and associated peak flows at the George Bolton Parkway/Hwy 50 intersection. You mentioned that you received the SWM report associated with the development site, discussed with regards to Figure 3. Would you mind providing us with that report?

I will bring the Region's proposal in regards to the monitoring of the Robinson creek into the discussion with my team next Tuesday. We aim to provide you with our suggestions/findings by the end of next week.

Regards

Jairo.

From: Andrew McGregor < AMcGregor@rvanderson.com >

Sent: Wednesday, February 3, 2021 5:21 PM

To: Jairo Morelli <Jairo.Morelli@trca.ca>; Emma Benko <emma.benko@trca.ca>

Cc: Banuri, Syeda <<u>syeda.banuri@peelregion.ca</u>>; Oliver Olberg <<u>OOlberg@rvanderson.com</u>>; Peter Cho <<u>pcho@rvanderson.com</u>>; Matthew de Wit



<<u>MdeWit@rvanderson.com</u>>; Sadman Soumik <<u>ssoumik@rvanderson.com</u>>

Subject: RE: Hwy 50 Drainage Improvements Class EA ~ Meeting Availability

Jairo and Emma,

Thanks for taking the time to meet earlier. Kindly review the attached notes from our meeting and note any errors or omissions. We would appreciate your follow up on the action items noted (eg. review of HEC-Ras model flows) at your earliest convenience. Any questions, please don't hesitate to ask.

Kind regards,



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Andrew McGregor, MCIP, RPP

Senior Planner, EA & Approvals

P: (905) 685-5049 ext. 4211 **C:** (905) 964-4056

R.V. Anderson Associates Limited 43 Church Street, Suite 104, St. Catharines, ON L2R 7E1

rvanderson.com



From: Jairo Morelli < <u>Jairo.Morelli@trca.ca</u>>

Sent: January 29, 2021 11:39 AM

To: Andrew McGregor < <u>AMcGregor@rvanderson.com</u>>

Cc: Banuri, Syeda <<u>syeda.banuri@peelregion.ca</u>>; Oliver Olberg <<u>OOlberg@rvanderson.com</u>>; Peter Cho <<u>pcho@rvanderson.com</u>>; Matthew de Wit

<MdeWit@rvanderson.com>; Sadman Soumik <ssoumik@rvanderson.com>; emma.benko@trca.ca

Subject: RE: Hwy 50 Drainage Improvements Class EA ~ Meeting Availability

[CAUTION EXTERNAL EMAIL] Make Sure that it is legitimate <u>before</u> Replying or Clicking on any links

Hi Andrew

Emma from our team will provide you with potential dates

In order to facilitate our meeting/discussion I would like to provide you with the following information:



We welcome the refinement or addition of drainage areas that were not part of the existing hydraulic model and may impact your study area. If feasible please provide us with the estimated flows and supporting calculations/assumptions before our meeting.

Figure 2

I will take a further look at this project next week. However, as far as I remember the description that you mentioned for the arrangement of the 900 and 600mm culverts that join to a large box under RR50 is correct.

We do not have any evidence of flooding on this intersection (Peel Region may know). However, we cannot predict it would not be flooded in the future. Due to the complex "hydraulics" of the three culverts that join at the George Bolton Parkway/RR50 intersection, the hydraulic update concluded that using a rating curve at the crossing will better simulate the existing conditions. You may refer to the COLE Hydraulic Report dated March 24, 2015, for further details.

The flows that were used in the HEC RAS model were derived from the hydrology study update we had at the time the HEC RAS was completed. We encourage your team to use these flows. However, if you would like to reassess them feel free to do so and provide us with the supporting calculations/modeling, hydrological parameters and relevant documentation.

We suggest the consultant investigate further opportunities to upgrade this crossing and reduce the existing flooding conditions as much as possible.

Figure 3

TRCA supports any measure that alleviates or enhance the existing flooding condition throughout the study area based on available BMP. Please go ahead and provide us with the rationale that supports your findings. We will take a look at TRCA database and provide you with details (if any) on the controlled flows from the mentioned new development.

Please contact me should you have further questions or concerns

Regards

Jairo Morelli

From: Andrew McGregor < AMcGregor@rvanderson.com >

Sent: Thursday, January 28, 2021 12:24 PM To: Jairo Morelli <Jairo.Morelli@trca.ca>

Cc: Banuri, Syeda <<u>syeda.banuri@peelregion.ca</u>>; Oliver Olberg <<u>OOlberg@rvanderson.com</u>>; Peter Cho <<u>pcho@rvanderson.com</u>>; Matthew de Wit

<<u>MdeWit@rvanderson.com</u>>; Sadman Soumik <<u>ssoumik@rvanderson.com</u>>

Subject: Hwy 50 Drainage Improvements Class EA ~ Meeting Availability

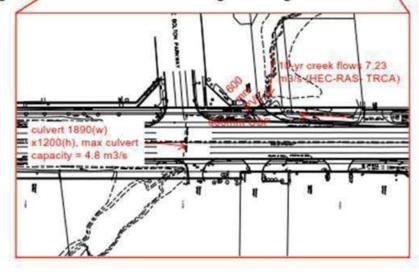
Hello Jairo,



Region
of Peel
working with you
We'd like to arrange for a meeting with the TRCA to discuss HEC-Ras hydraulics and flows in Rainbow Creek. Specifically, why creek flows to the west side of RR50 are so high so that road flooding is predicted from a 2-year storm event onwards (see insert below and Figure 2 writeup attached). Kindly let us know your availability to meet (digitally)... hopefully some time next week. Feel free to include other staff as required.



Figure 2: Rainbow Creek crossing at George Bolton Intersection



Thanks and kind regards,





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Andrew McGregor, MCIP, RPP

Senior Planner, EA & Approvals

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Appendix B

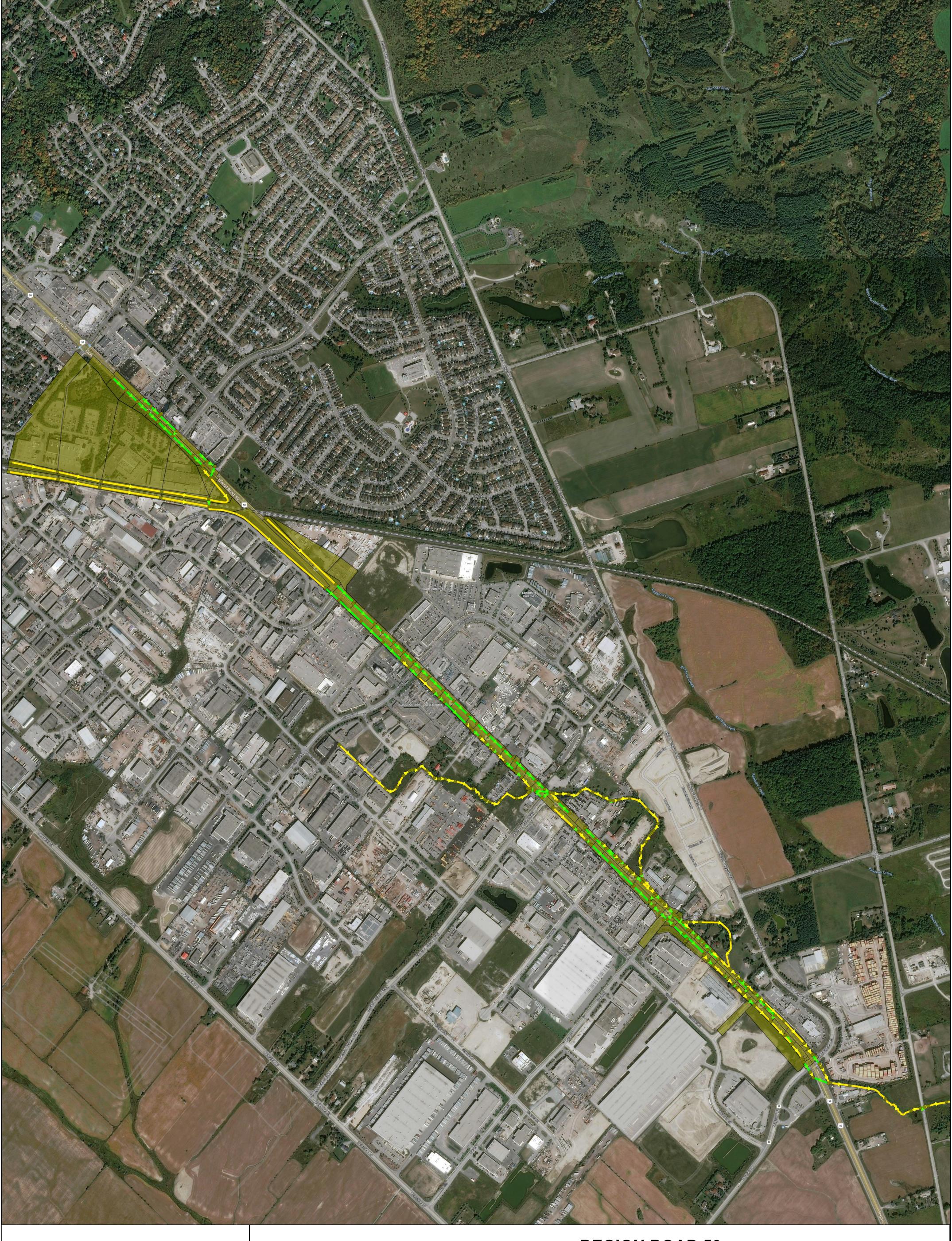
Hydraulic Modelling Outputs







Date: 2021-08-31



Legend

NODES/MANHOLES

CHANNEL/CREEK/DITCH

STORM SEWERS/CULVERTS

SUBCATCHMENTS

REGION ROAD 50 FROM MAYFIELD ROAD TO HEALEY ROAD

EXISTING DRAINAGE OVERVIEW MAP

RVA PROJECT NO: 194615





125 250

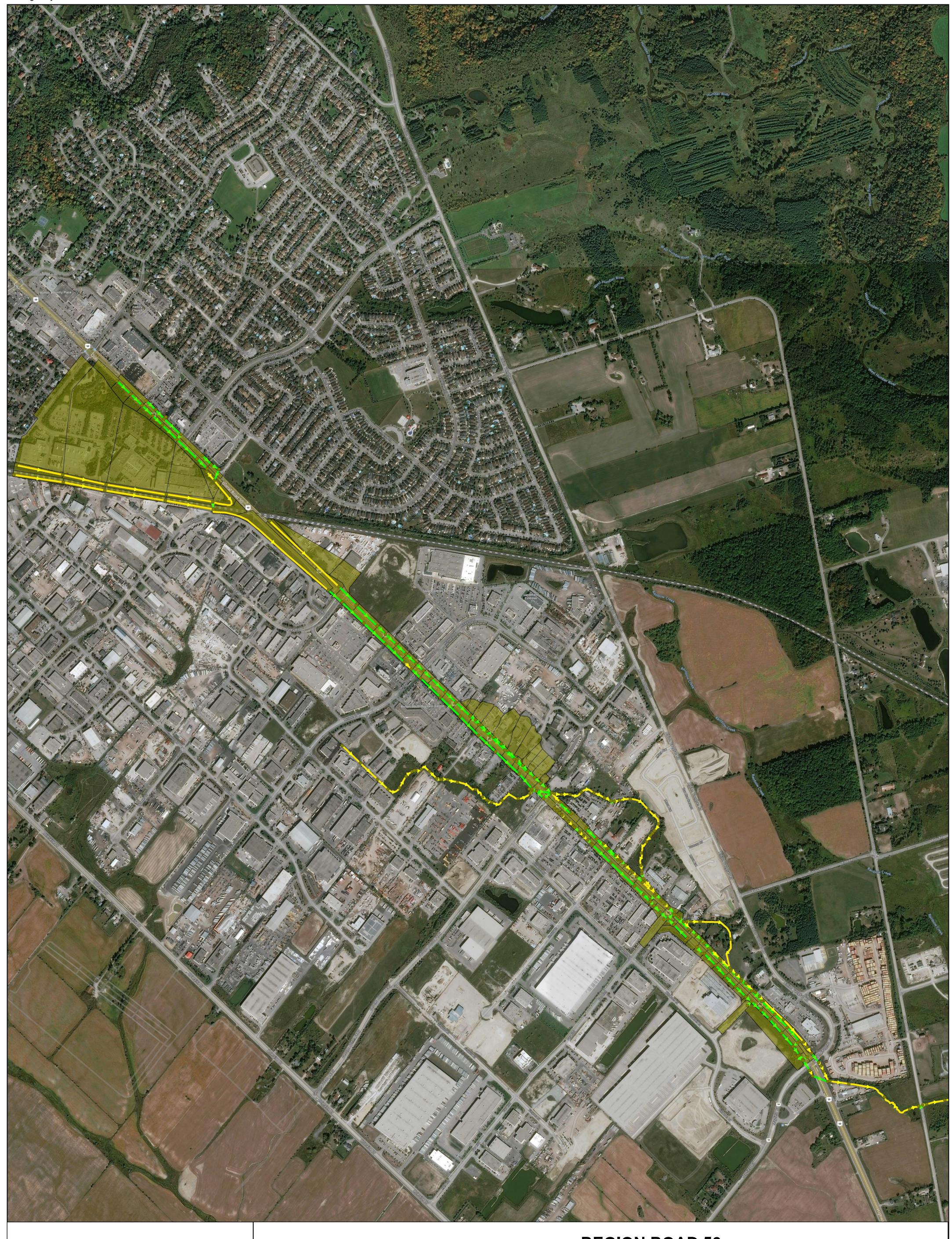
500

750

1,000 Meters



Date: 2021-08-31



Legend

NODES/MANHOLES

CHANNEL/CREEK/DITCH

STORM SEWERS/CULVERTS

SUBCATCHMENTS

REGION ROAD 50 FROM MAYFIELD ROAD TO HEALEY ROAD

PROPOSED DRAINAGE OVERVIEW MAP

2

RVA PROJECT NO: 194615





0 125 250

500

750 1,000 Meters



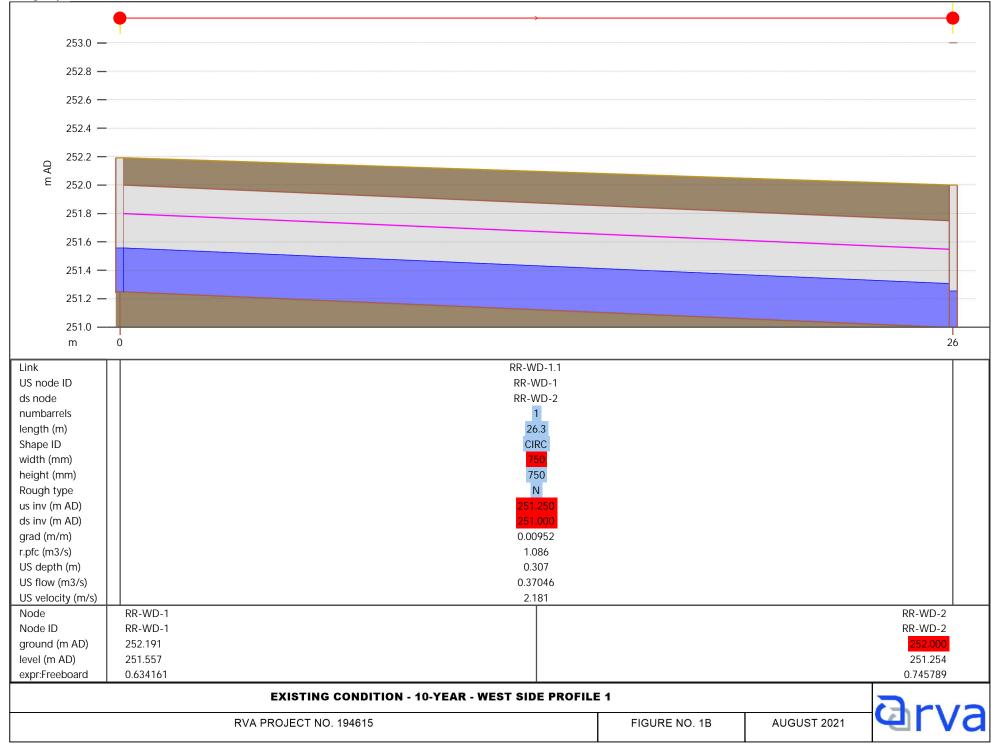






FIGURE NO. 1A





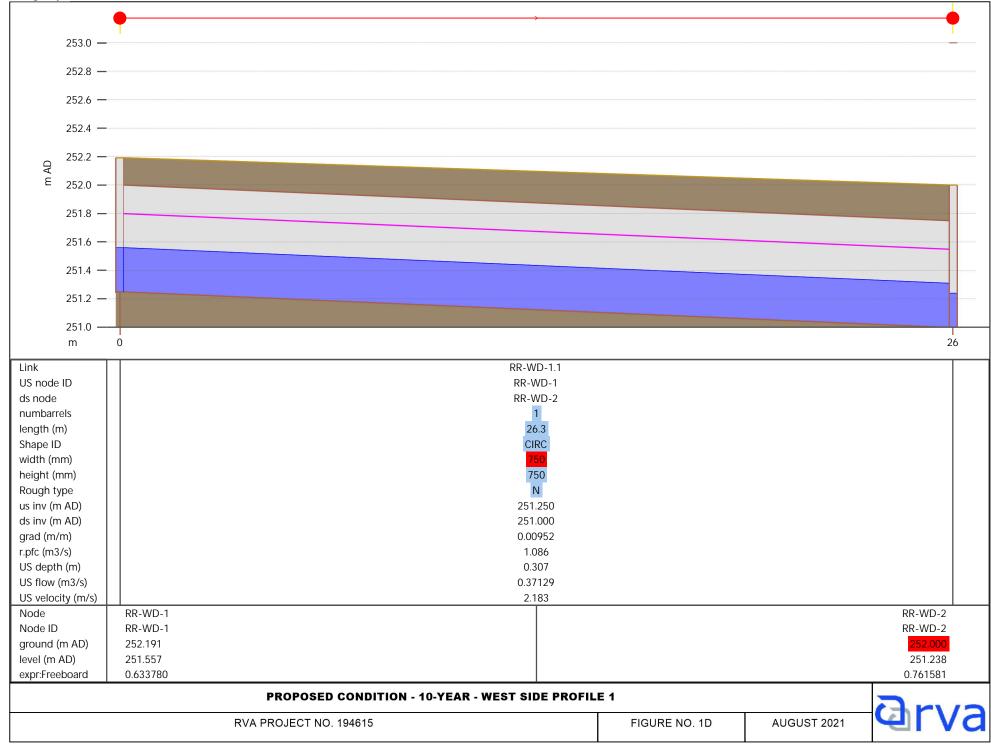




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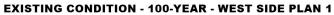










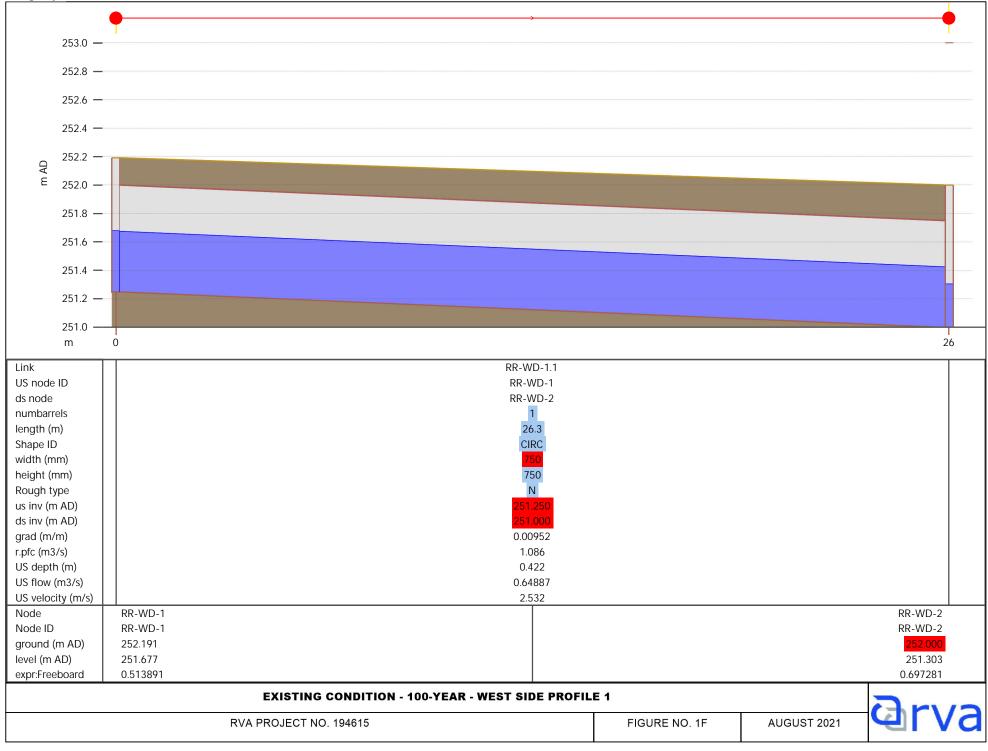


RVA PROJECT NO. 194615

FIGURE NO. 1E











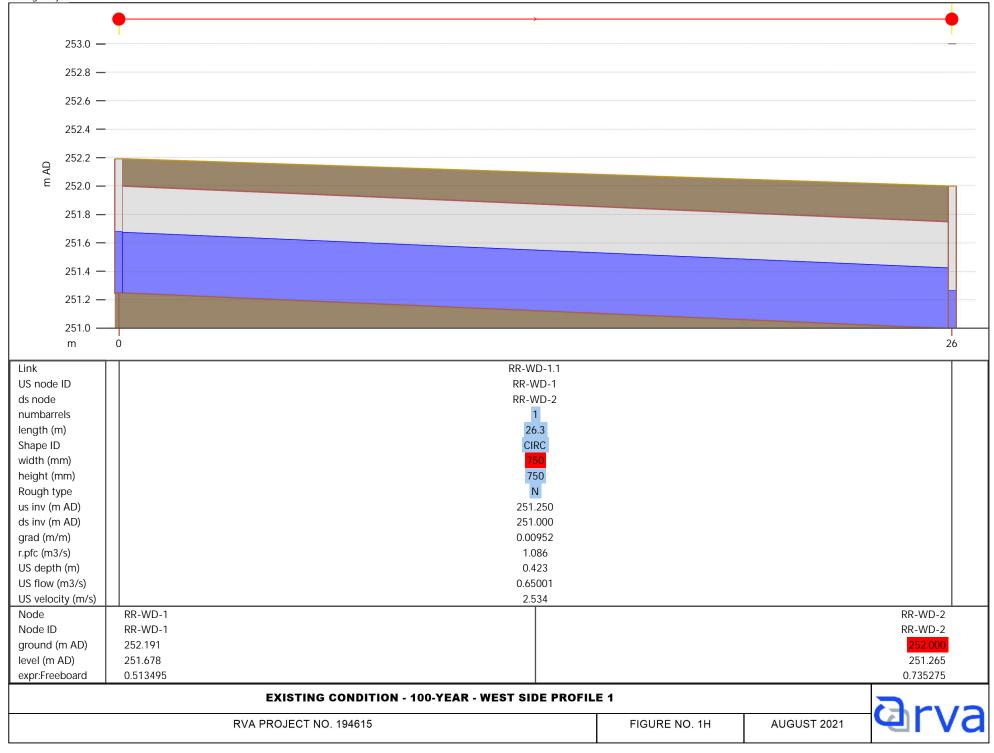
PROPOSED CONDITION - 100-YEAR - WEST SIDE PLAN 1

RVA PROJECT NO. 194615

FIGURE NO. 1G

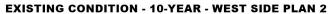










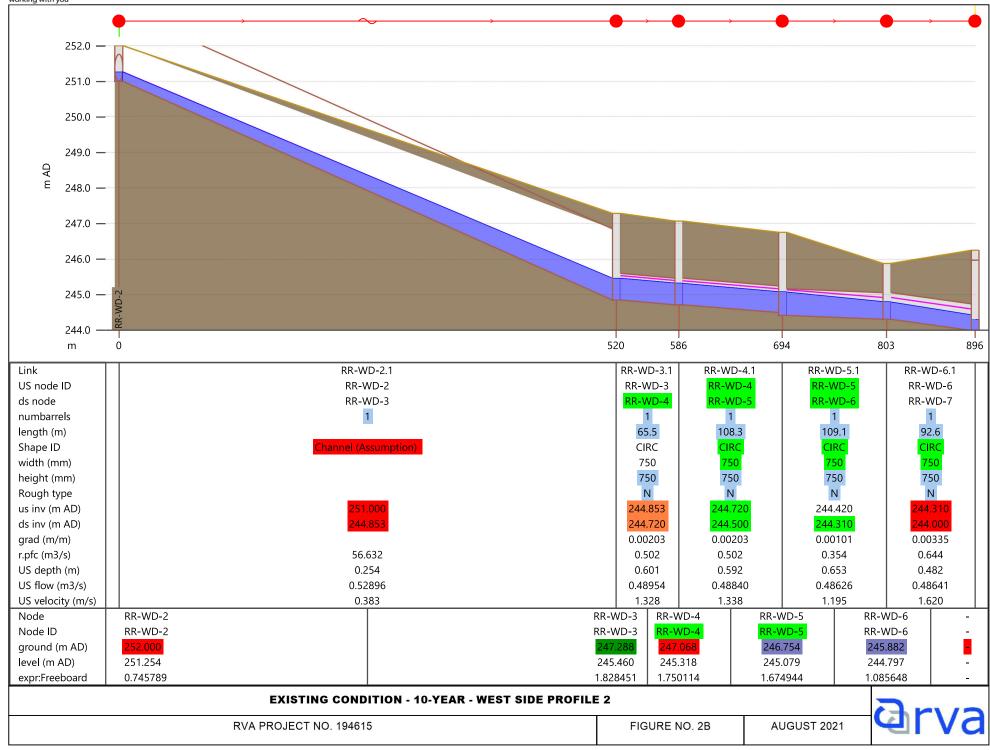


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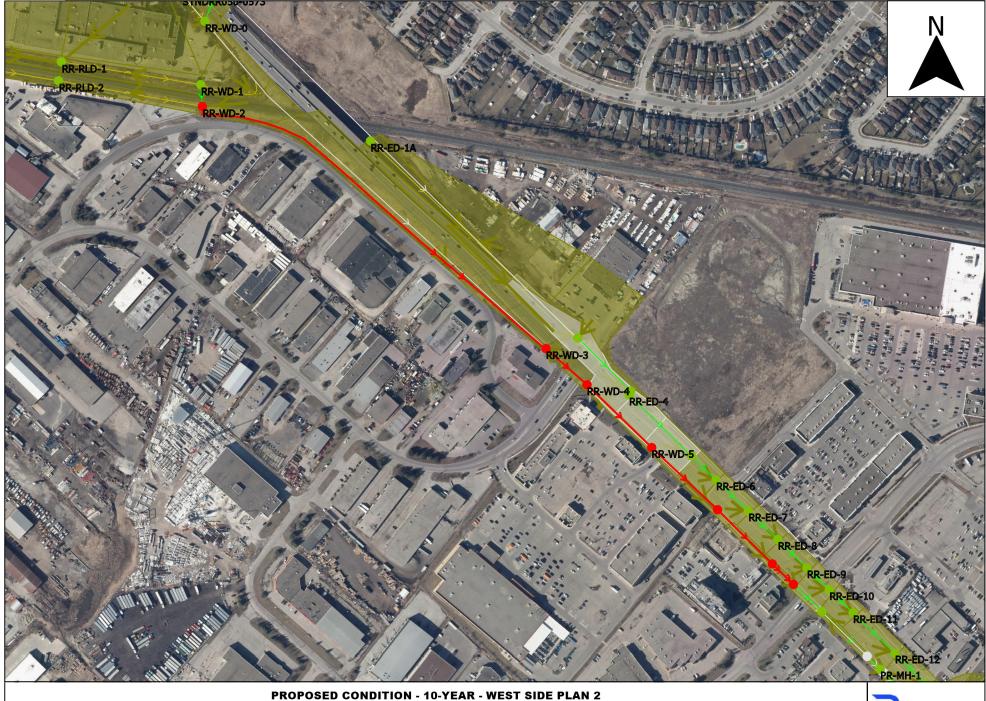
FIGURE NO. 2A









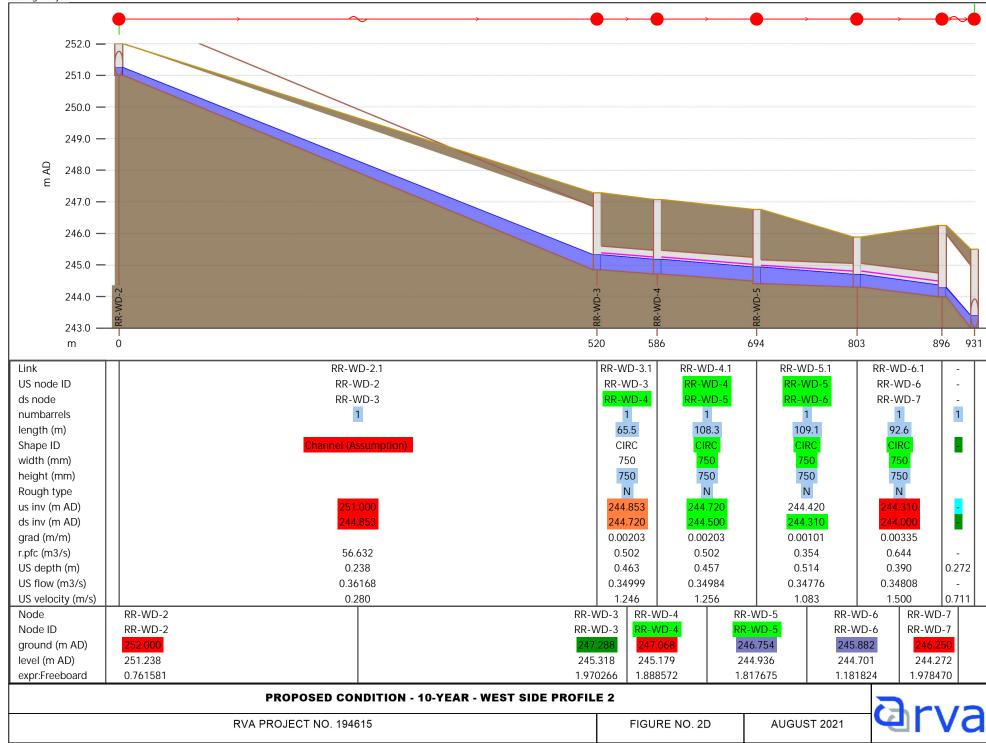


PROPOSED CONDITION - 10-YEAR - WEST SIDE PLAN 2

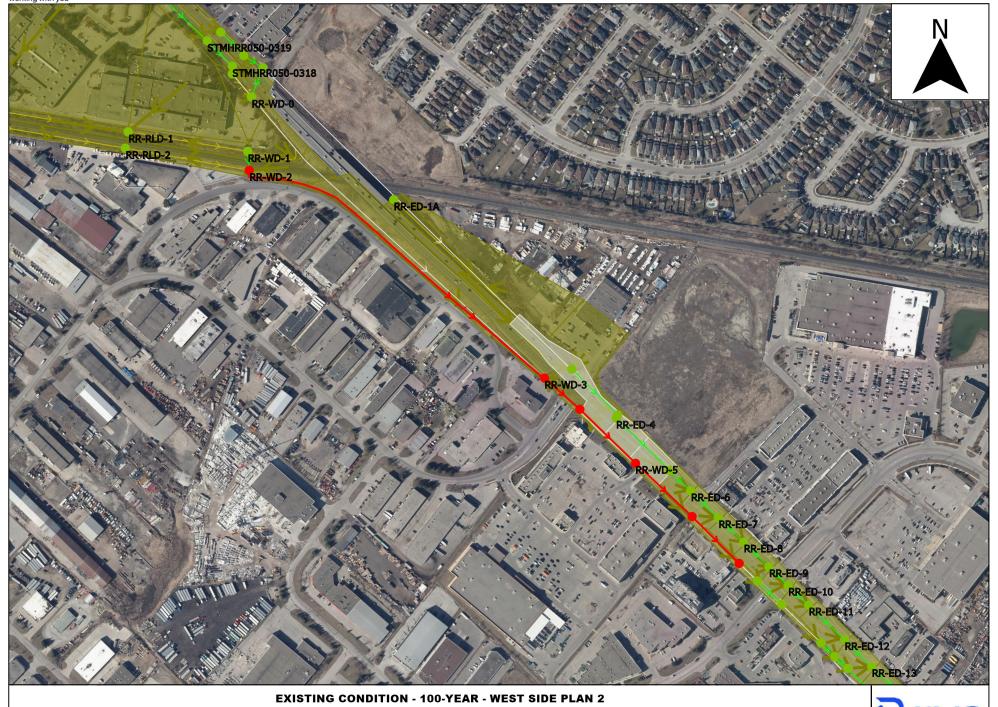
RVA PROJECT NO. 194615

FIGURE NO. 2C









AUGUST 2021

FIGURE NO. 2E



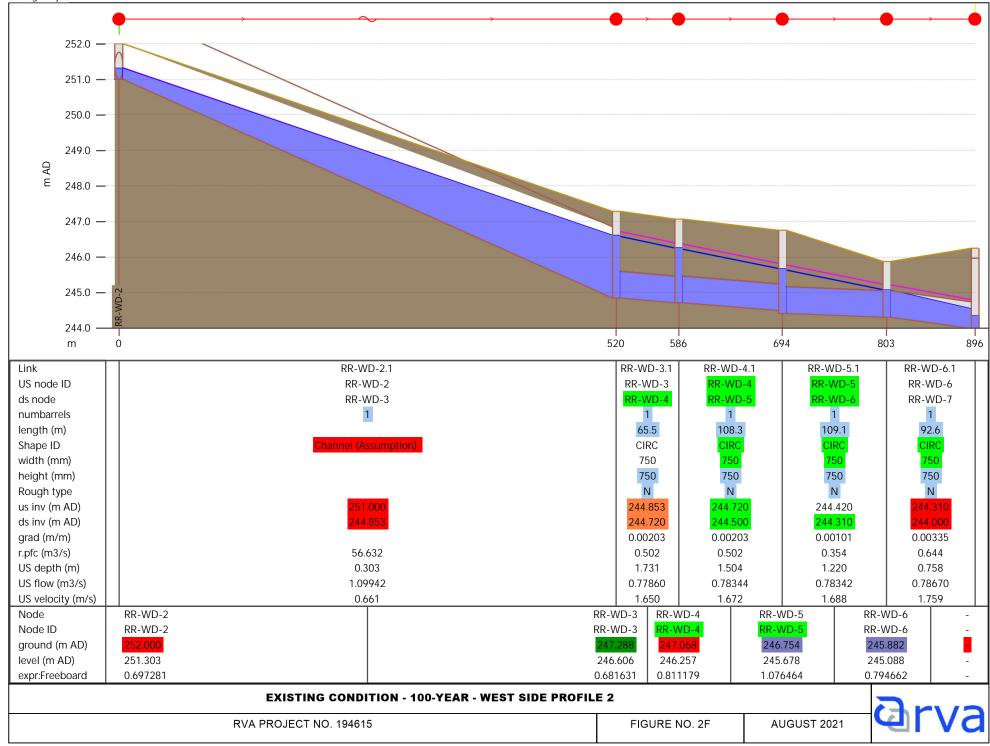




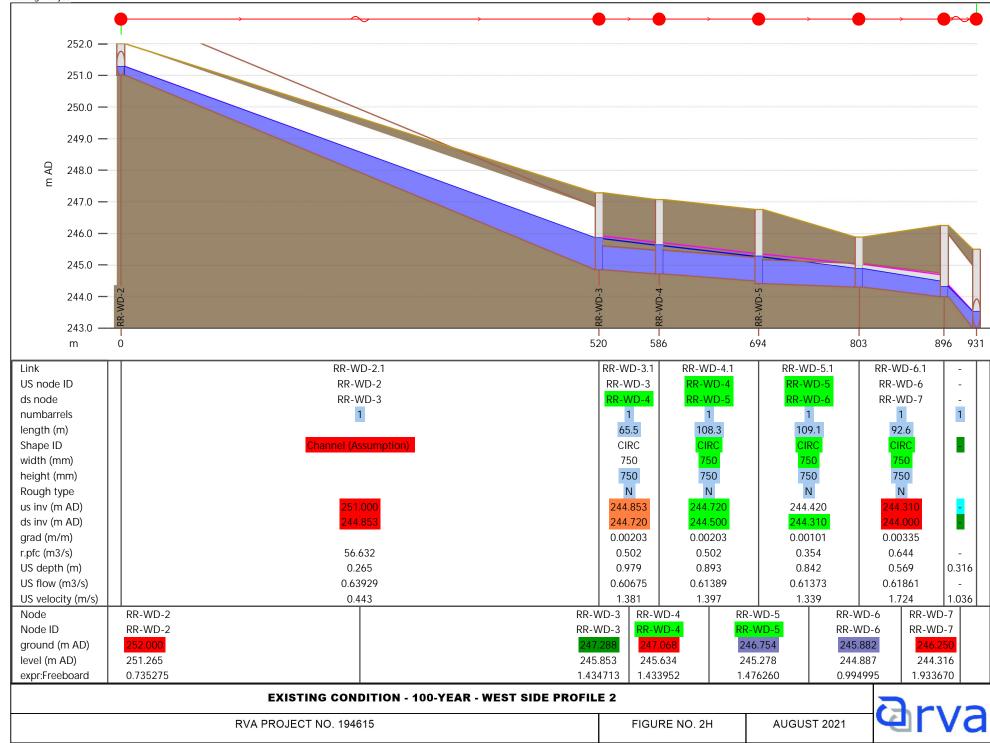




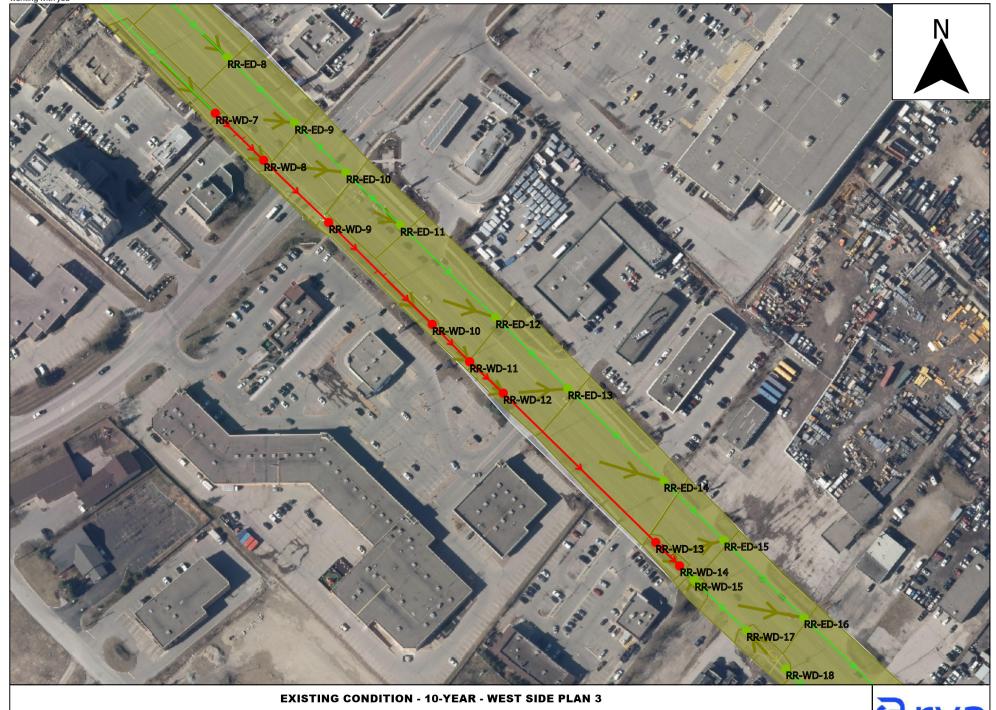
FIGURE NO. 2G







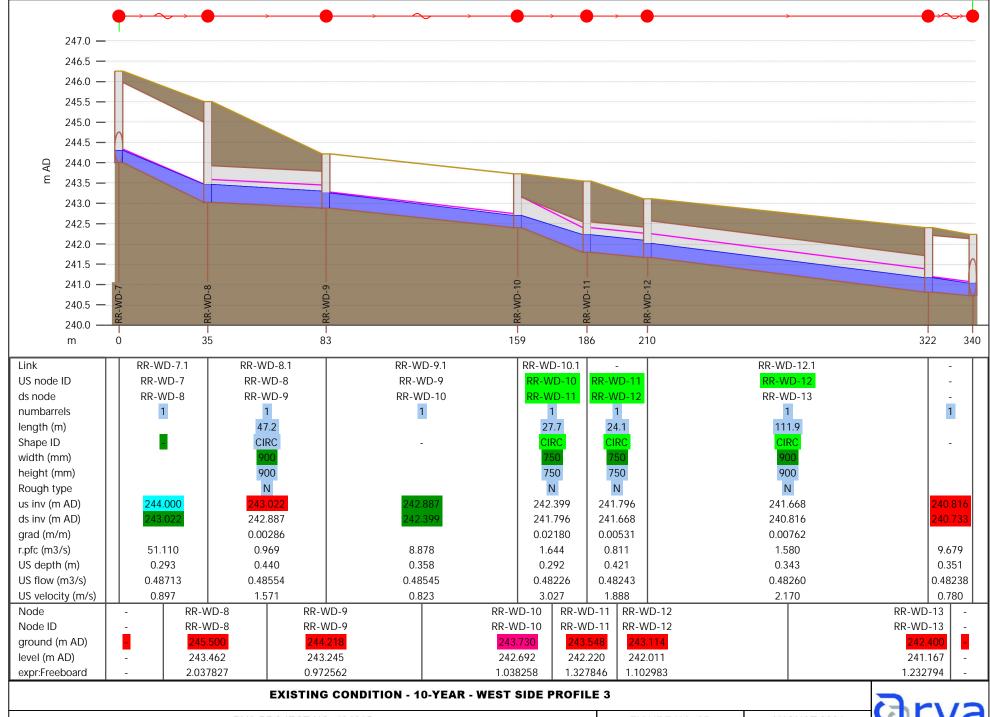




AUGUST 2021

FIGURE NO. 3A







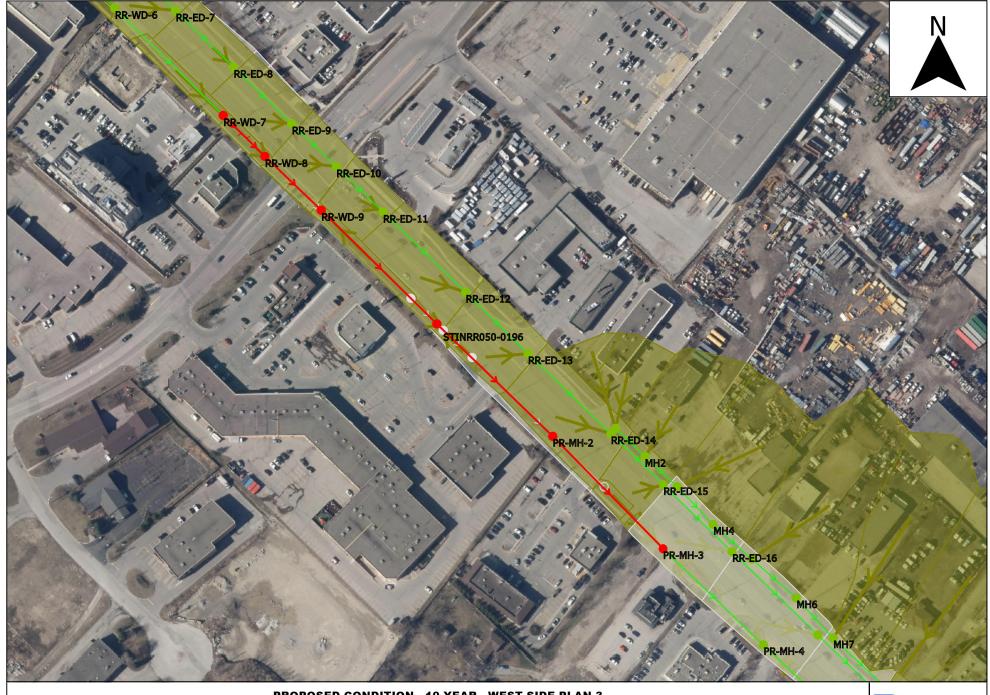
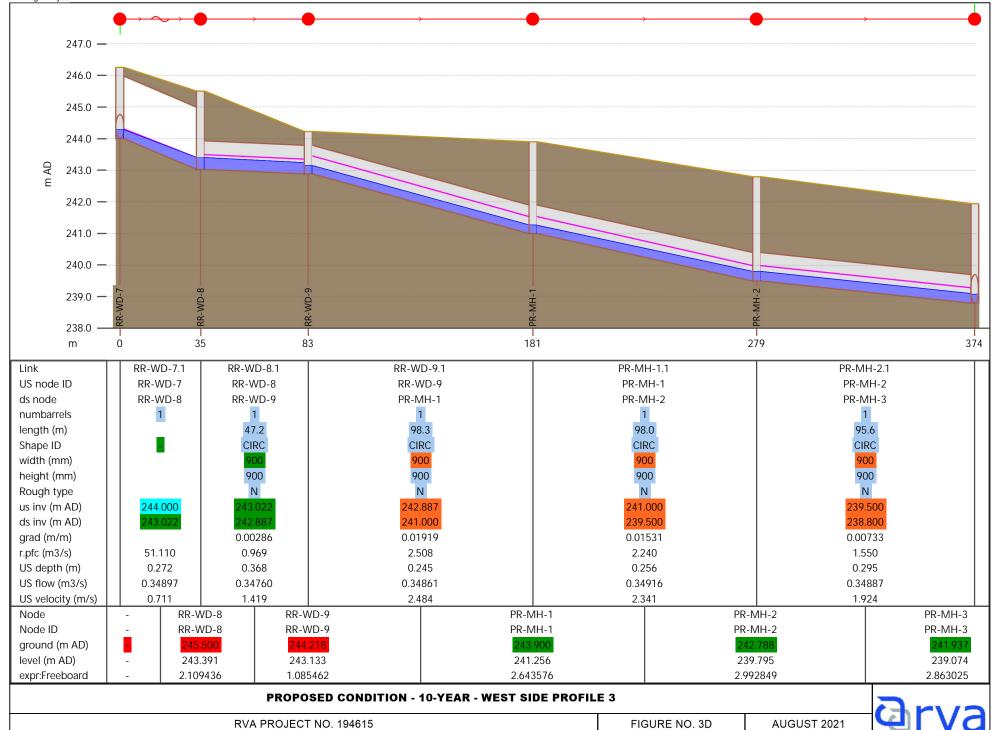




FIGURE NO. 3C









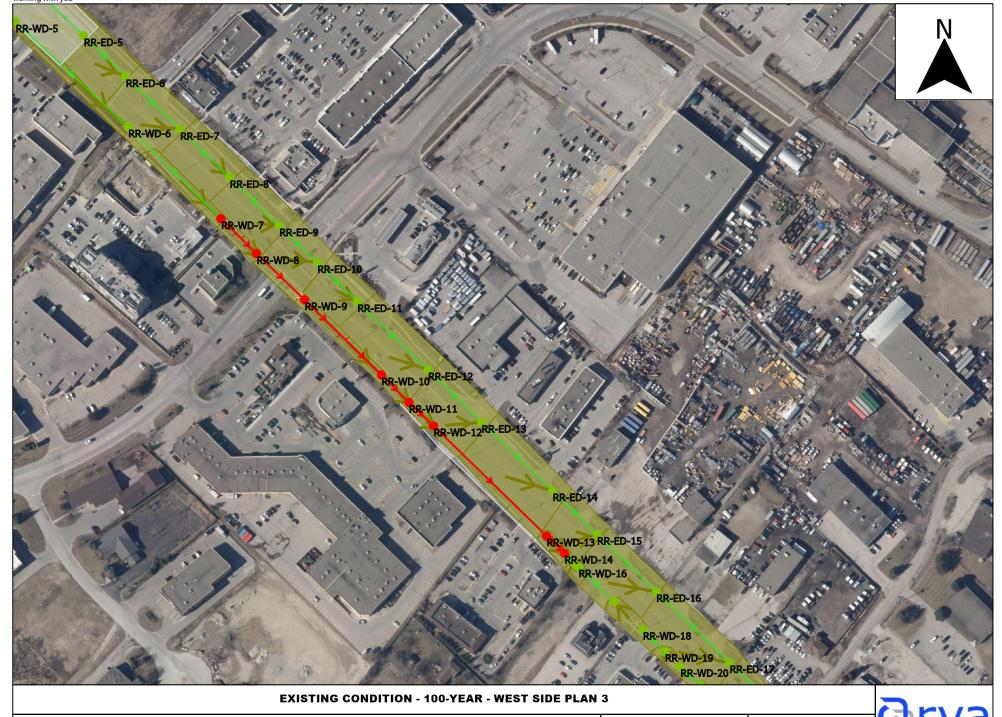
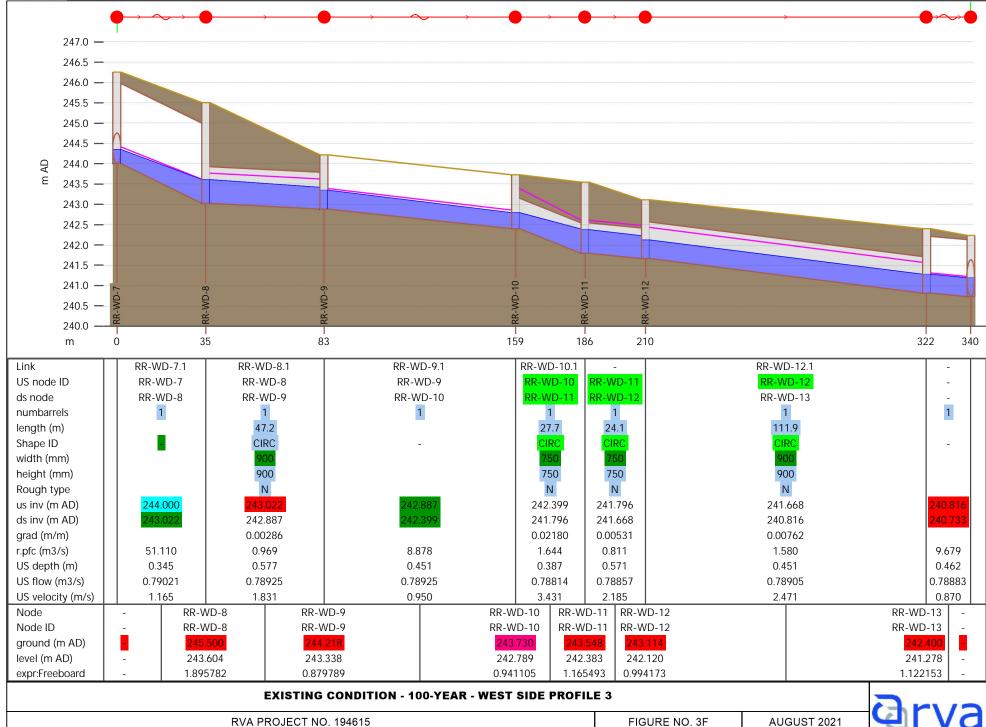
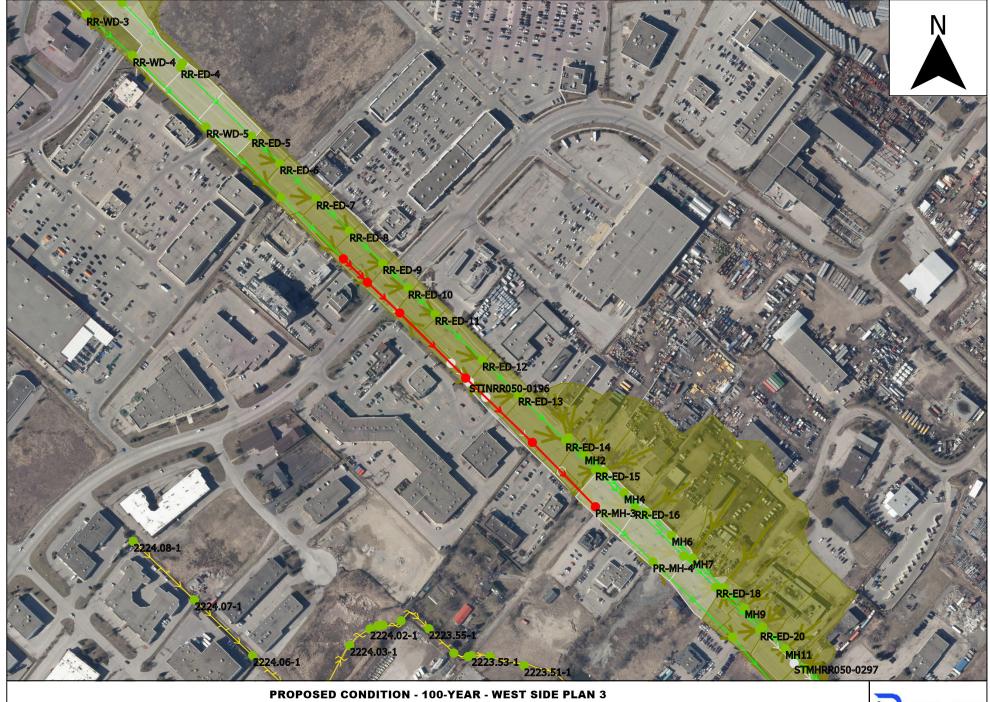


FIGURE NO. 3E









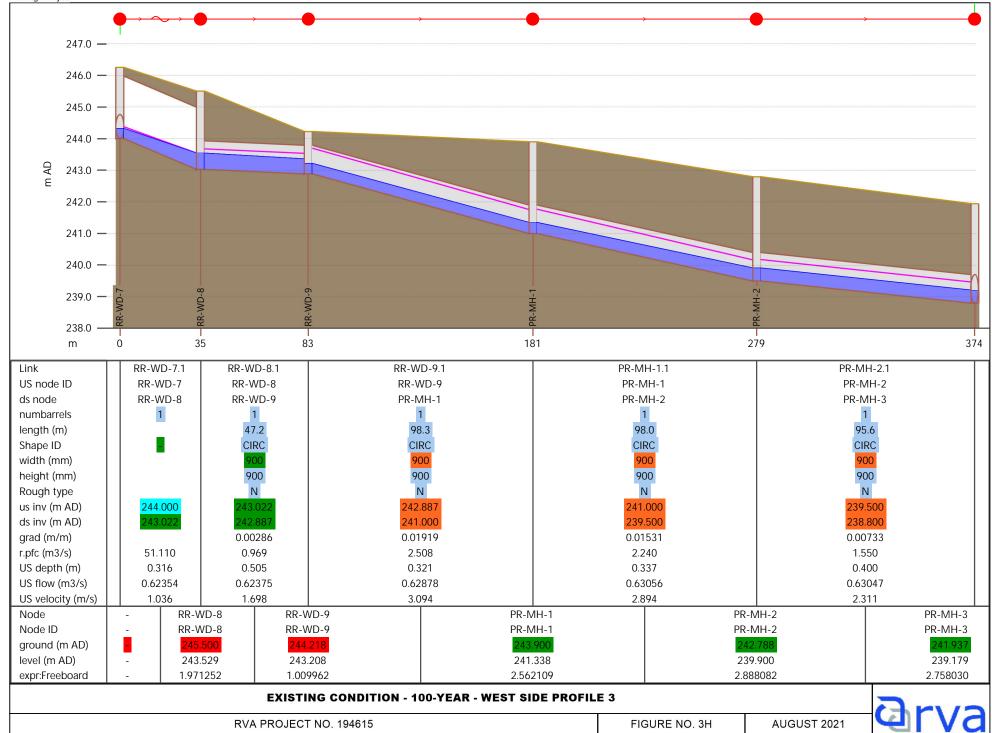
THE TOTAL TOTAL TEST SIDE TERMS

RVA PROJECT NO. 194615

FIGURE NO. 3G









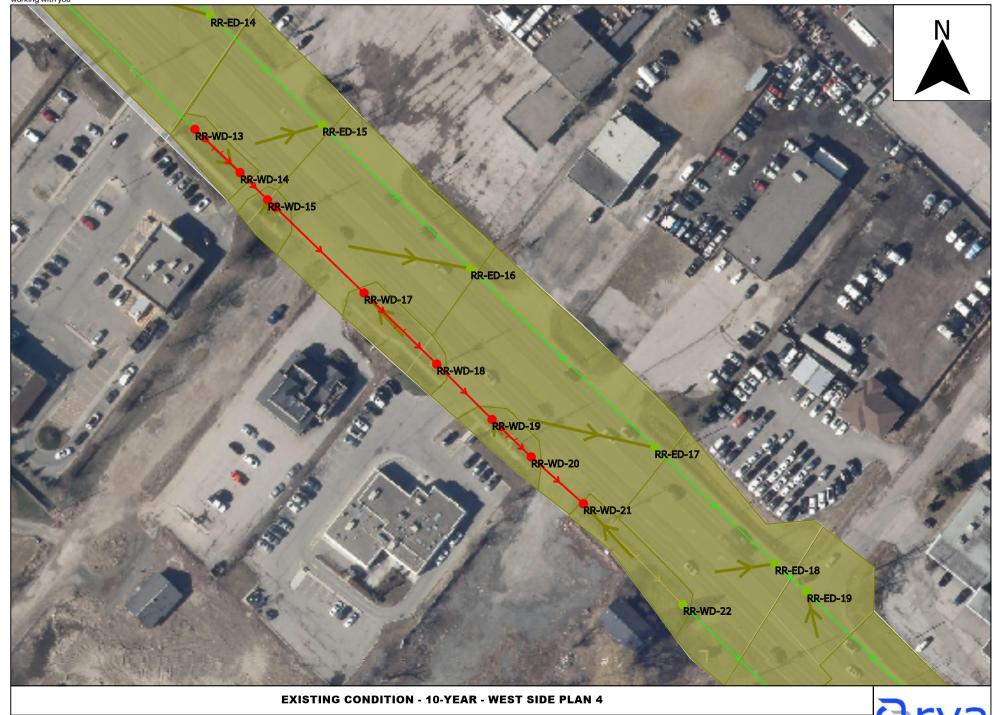
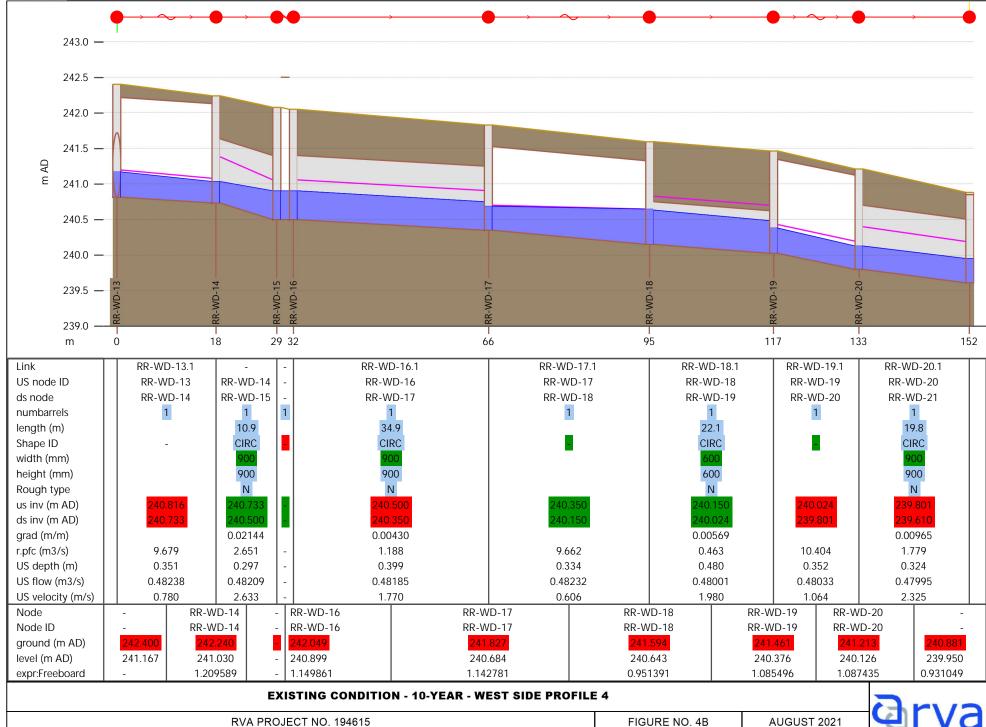
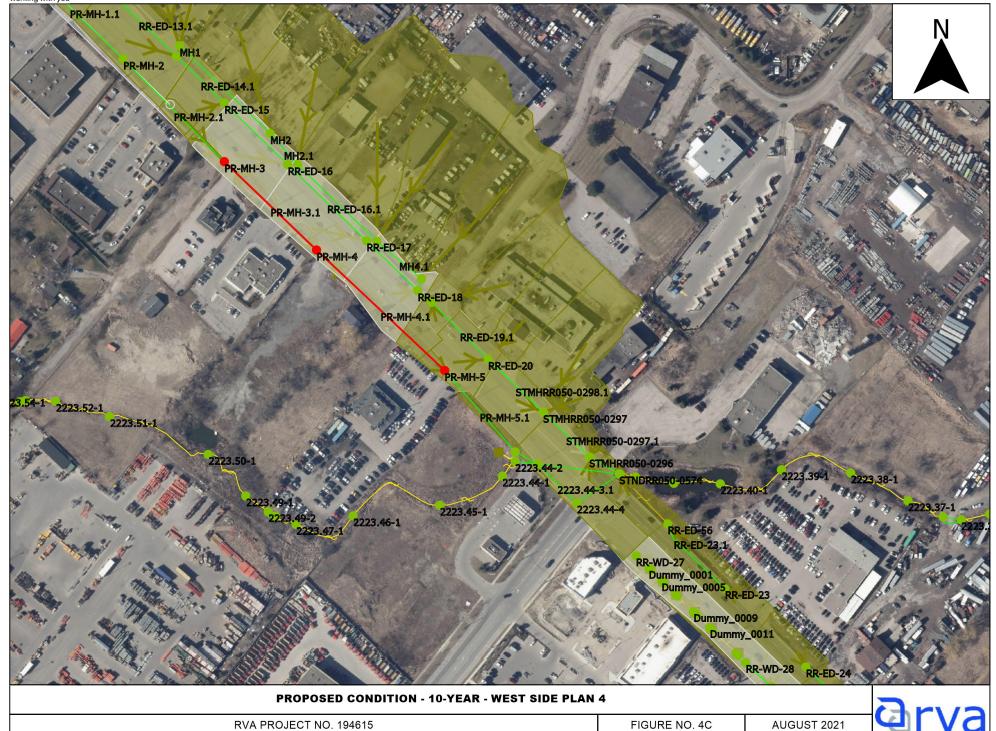


FIGURE NO. 4A











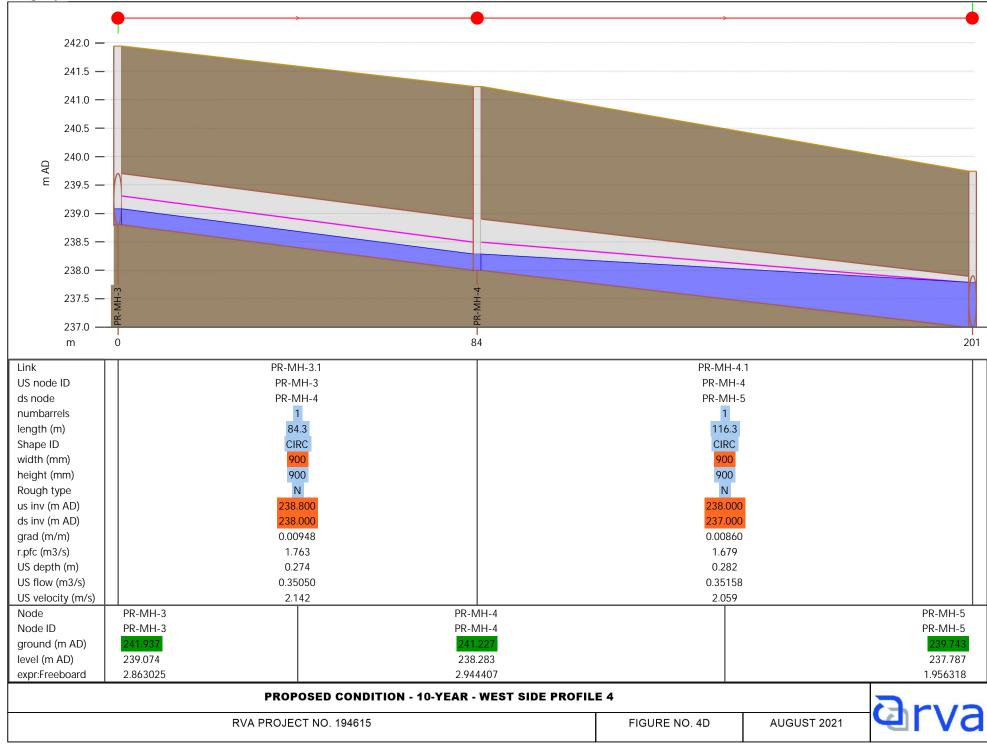
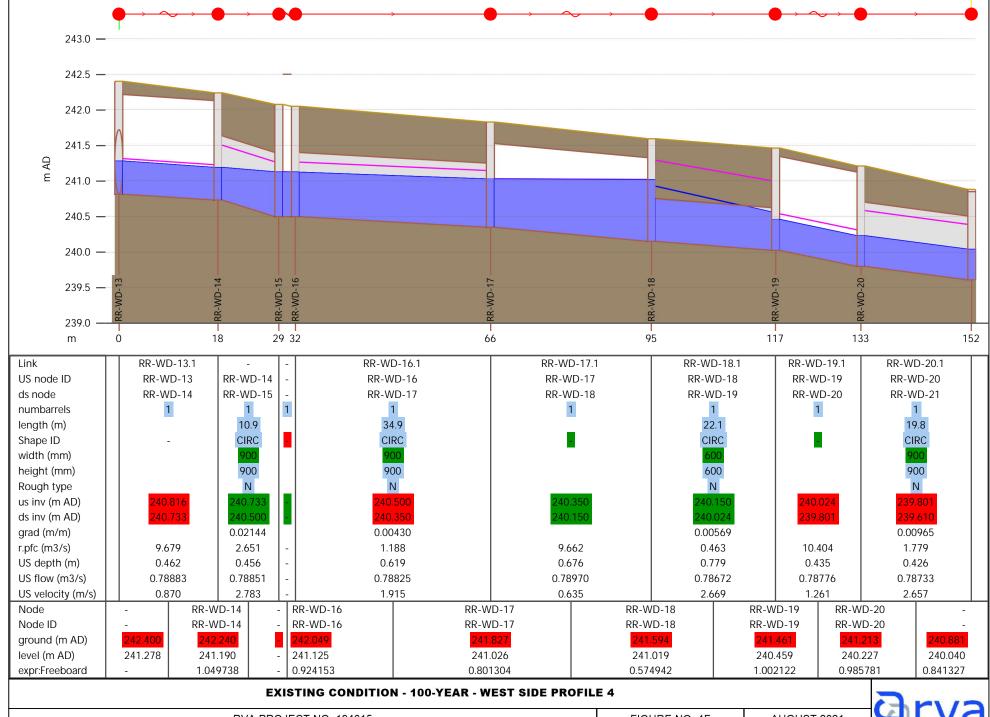




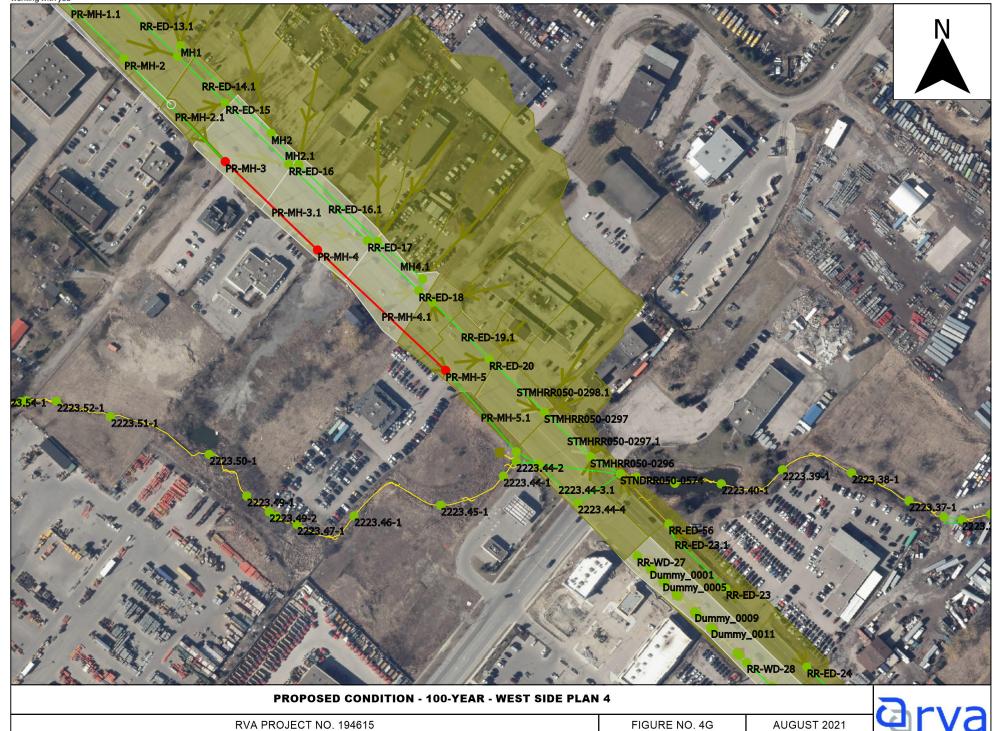


FIGURE NO. 4E

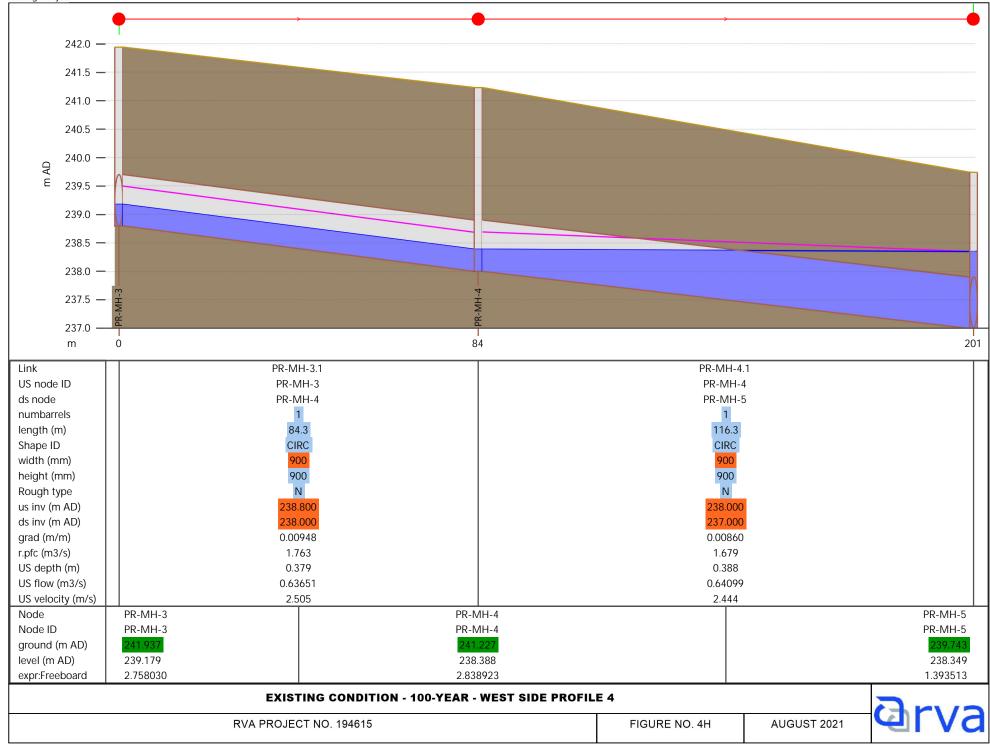




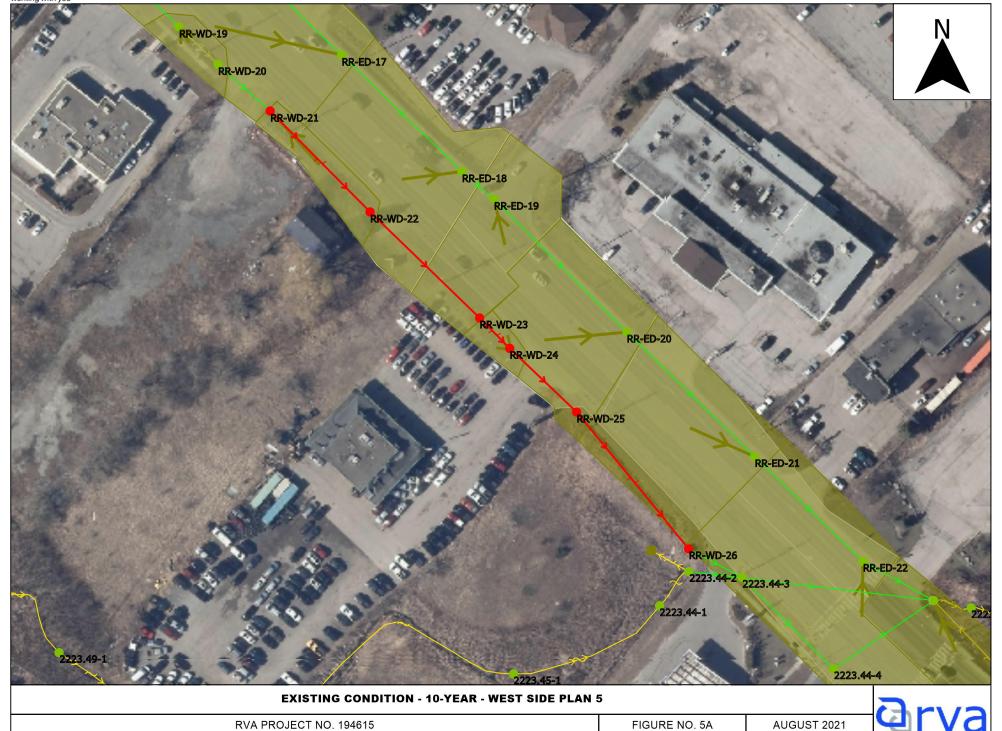




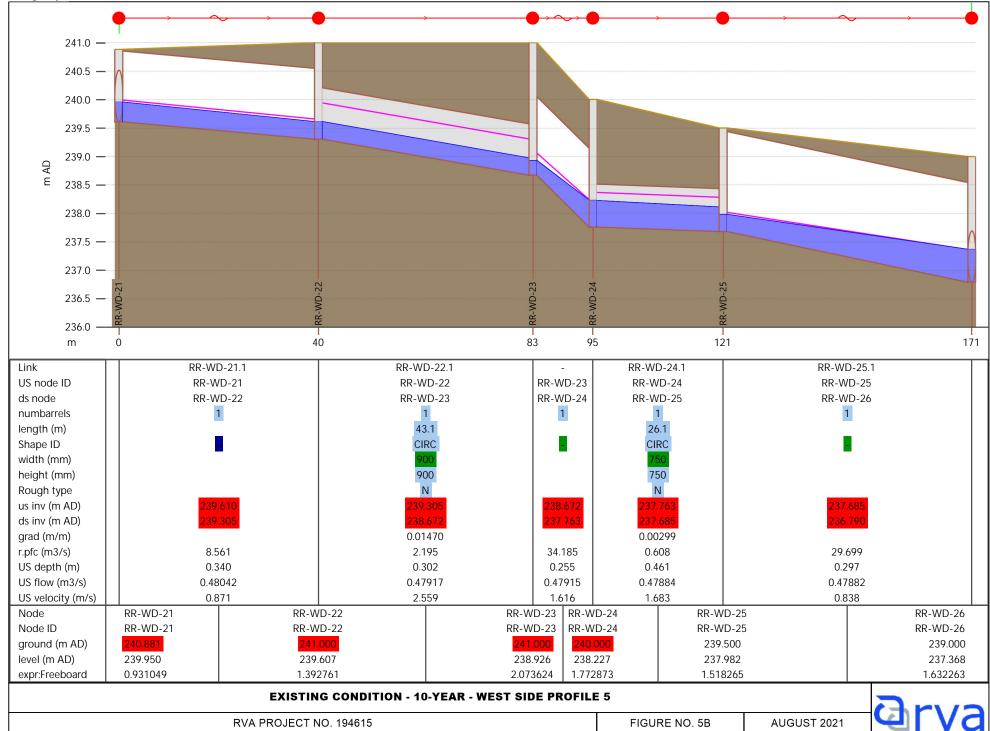




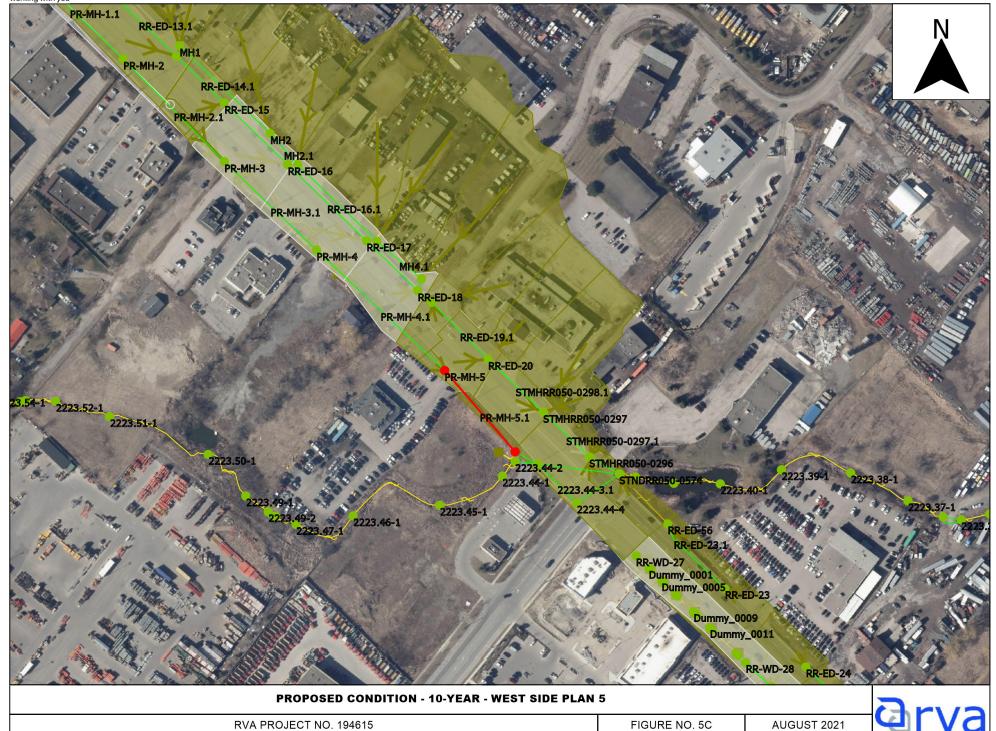




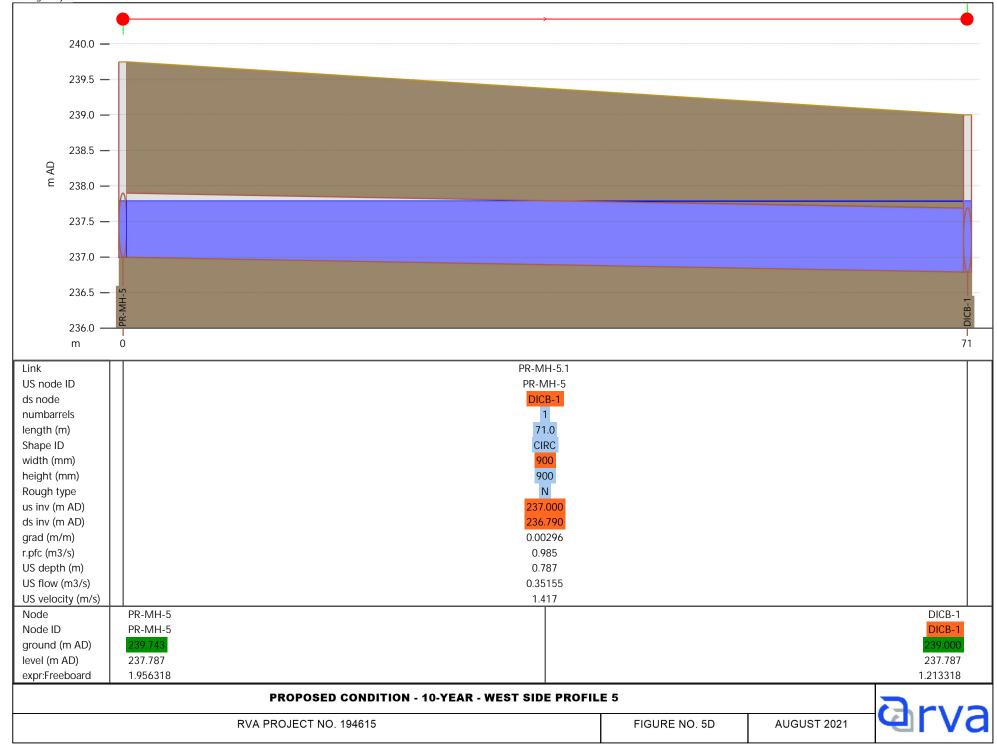








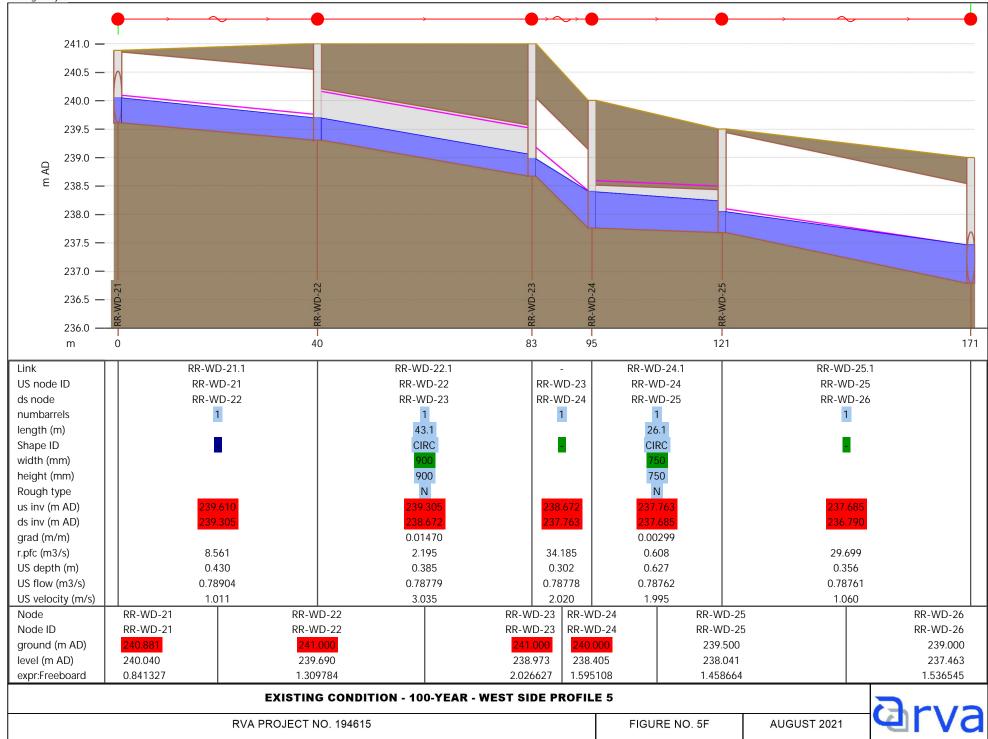




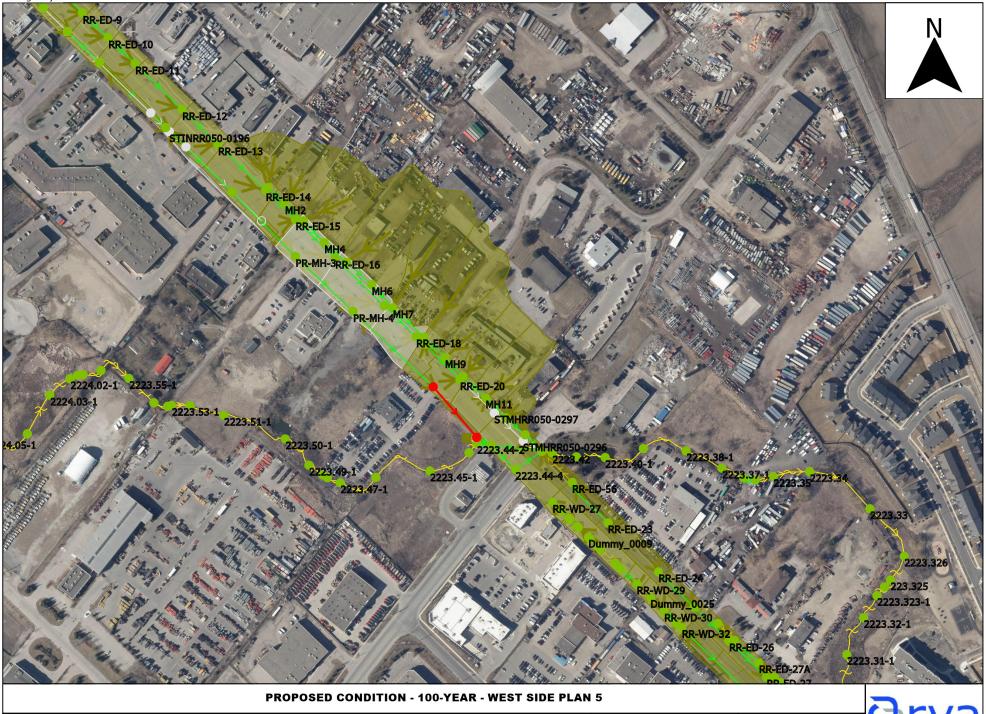






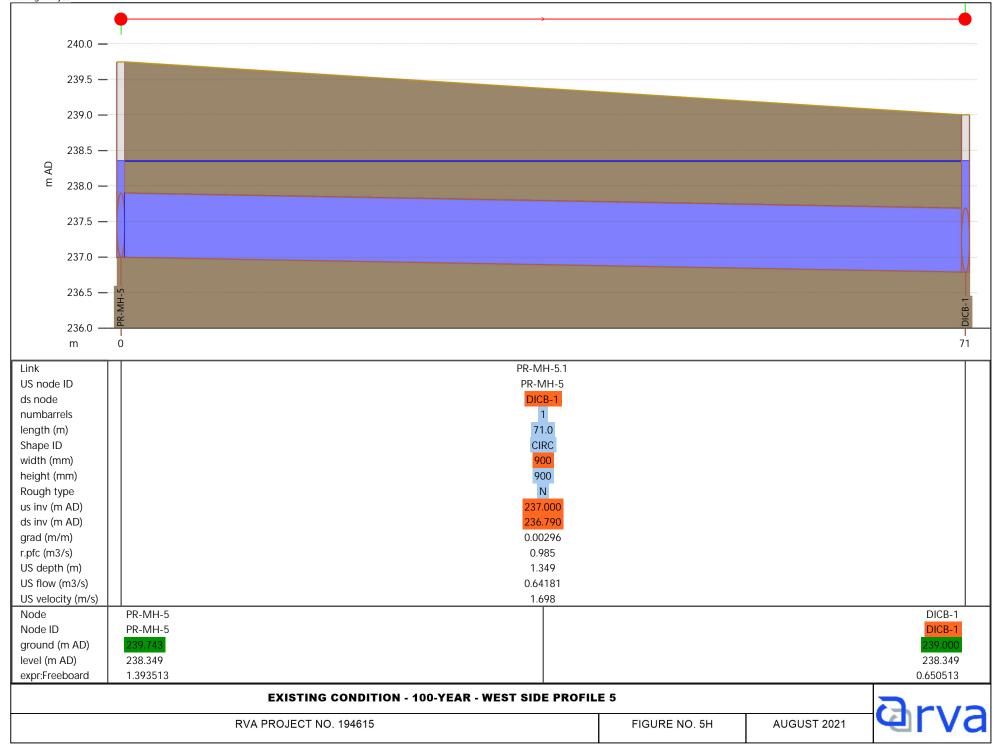




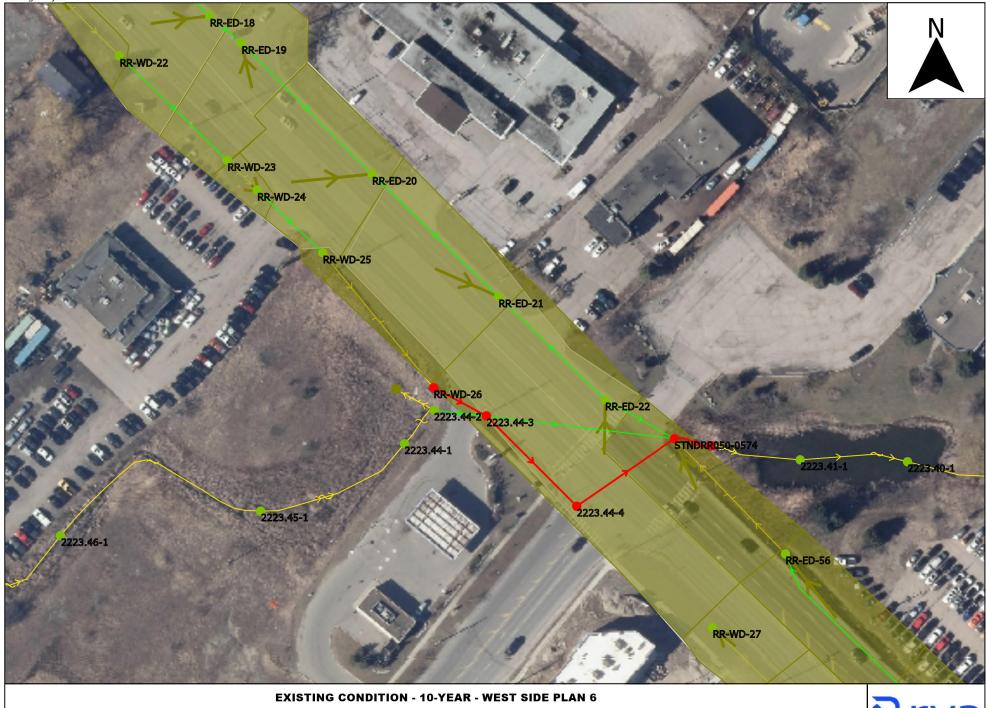


@rva







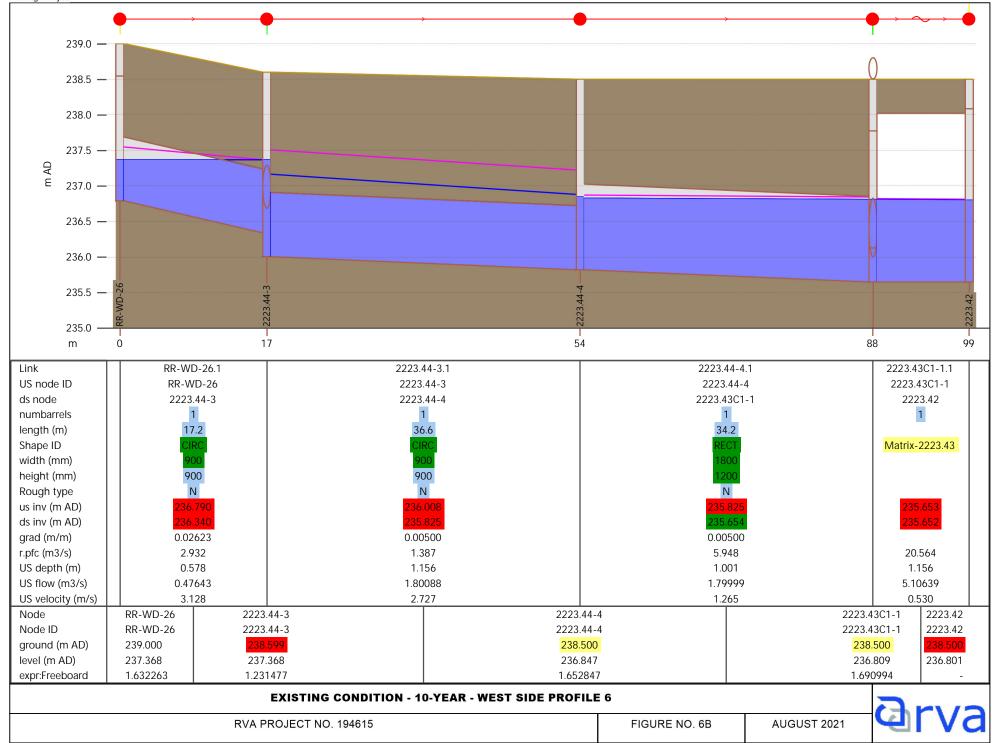


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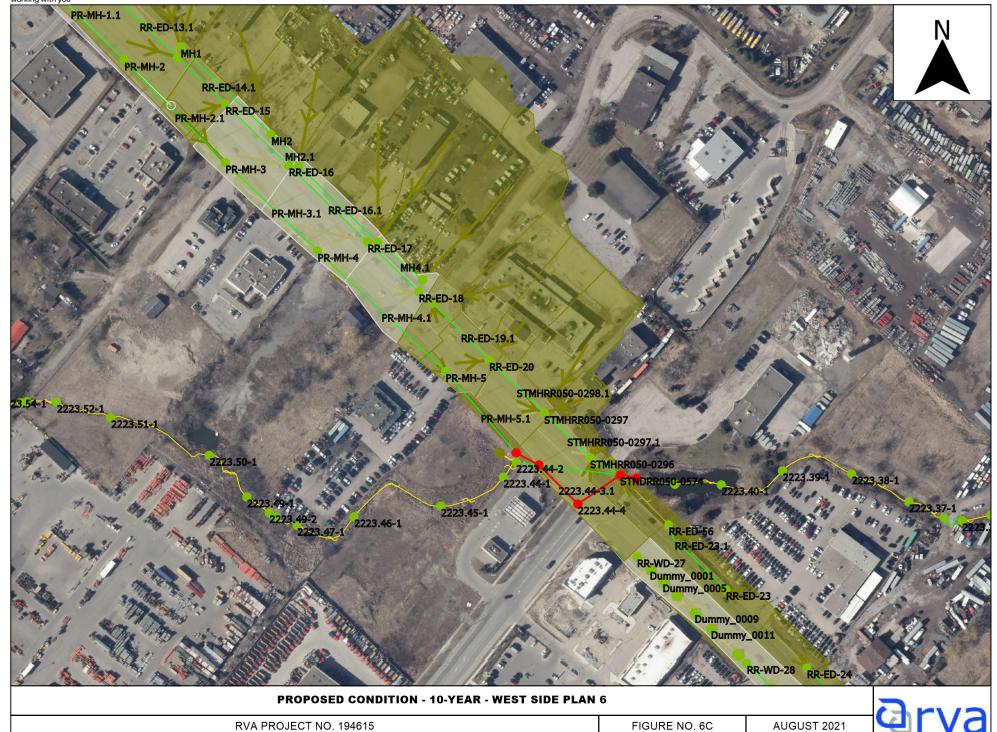
FIGURE NO. 6A



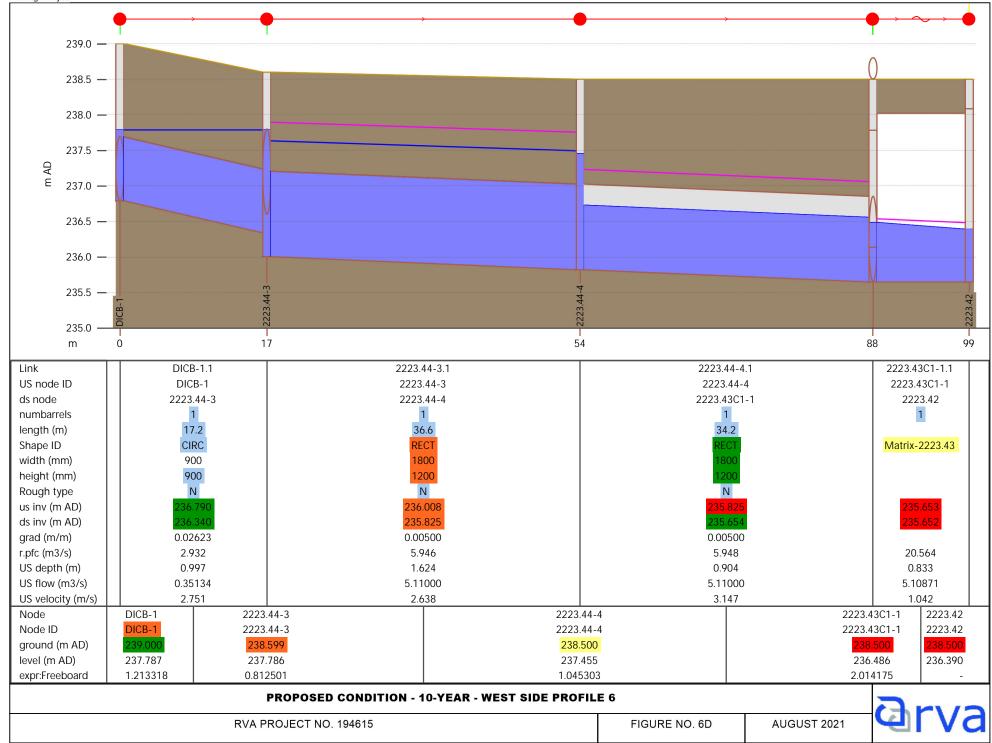




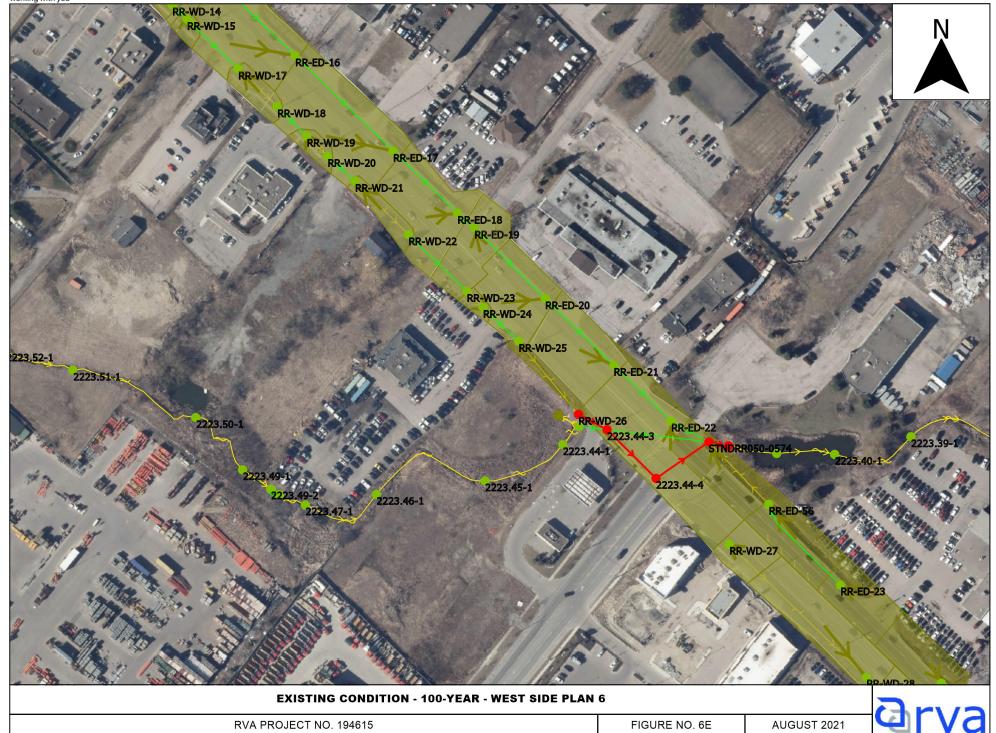




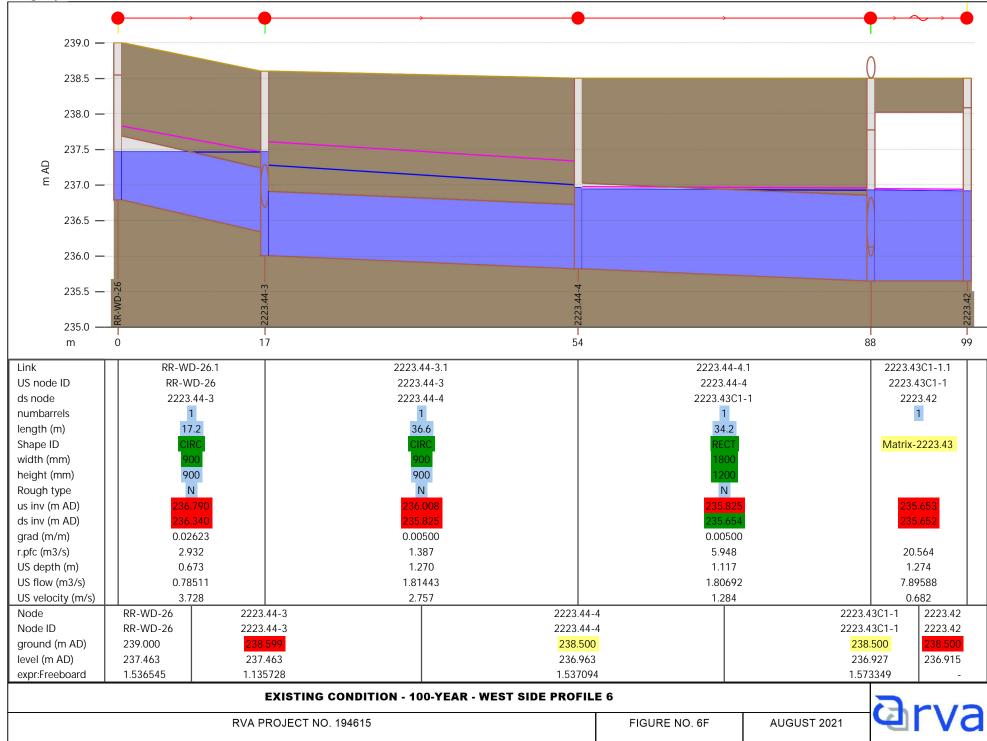




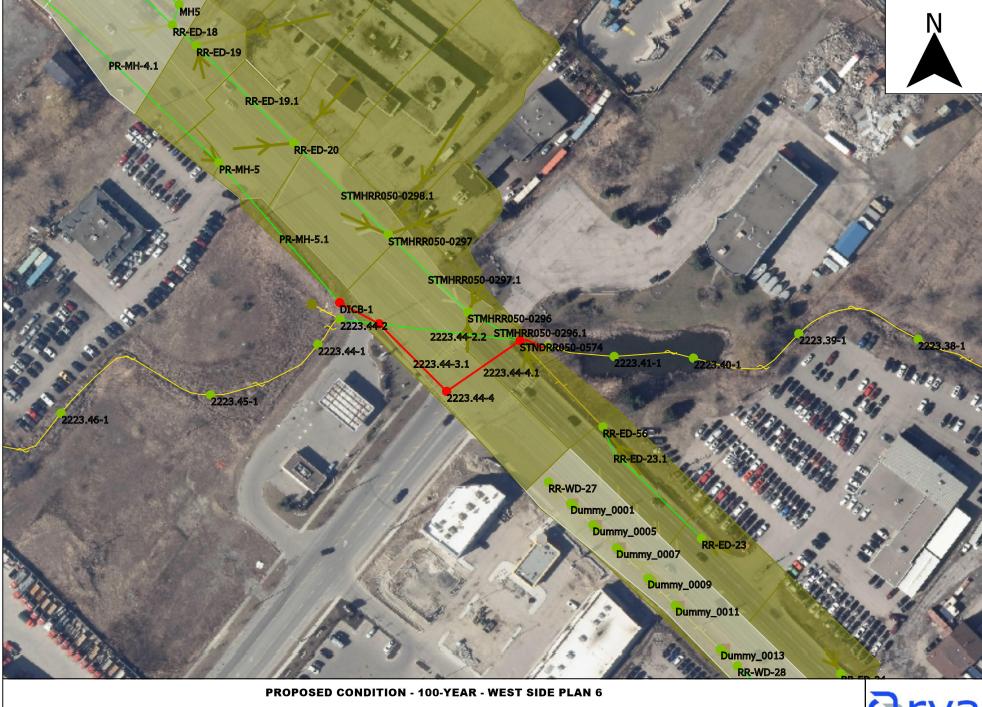






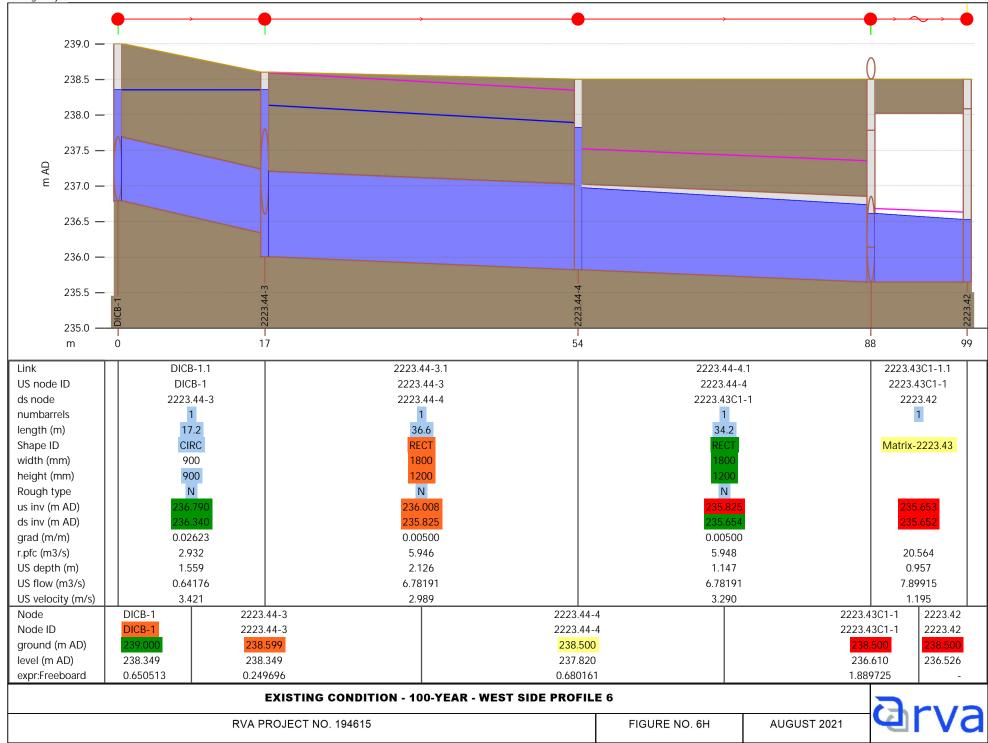




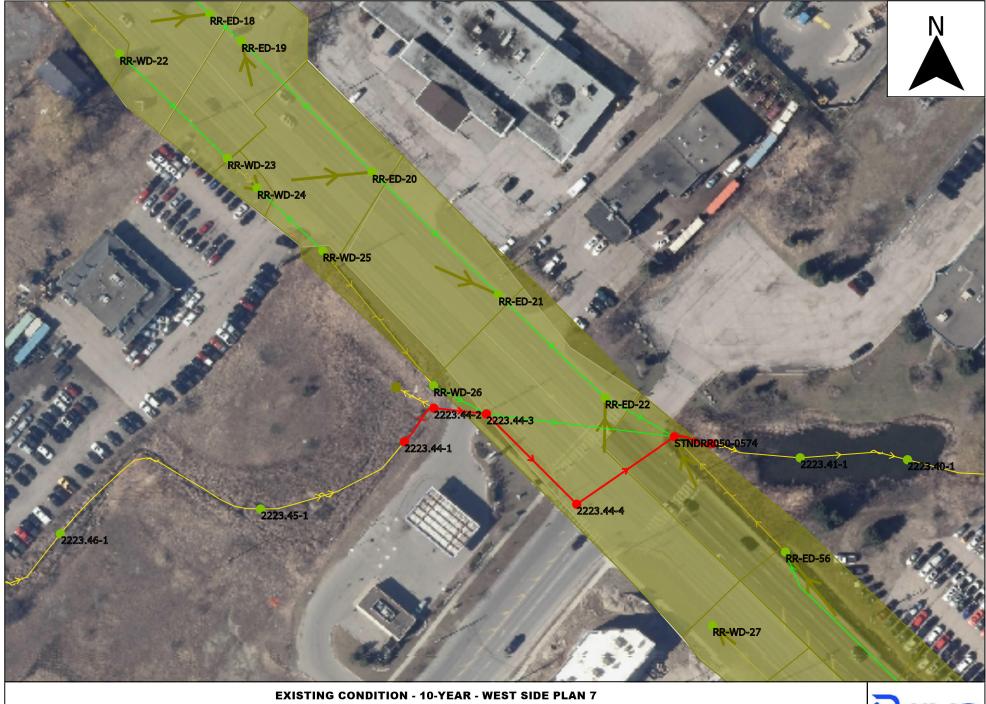


D. 6G AUGUST 2021





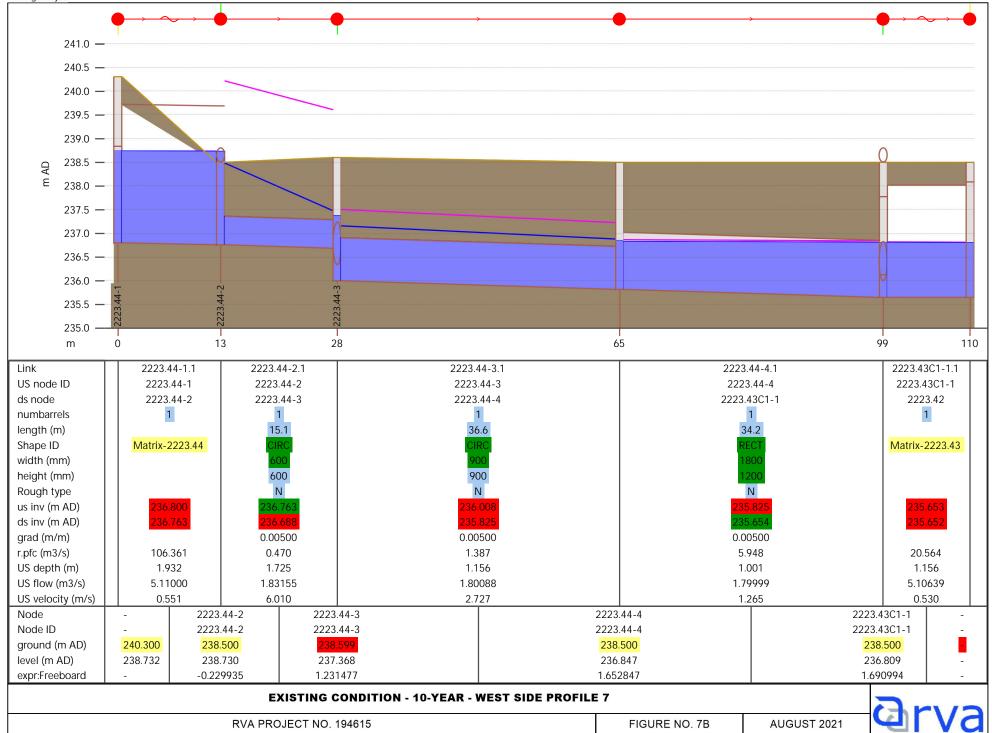




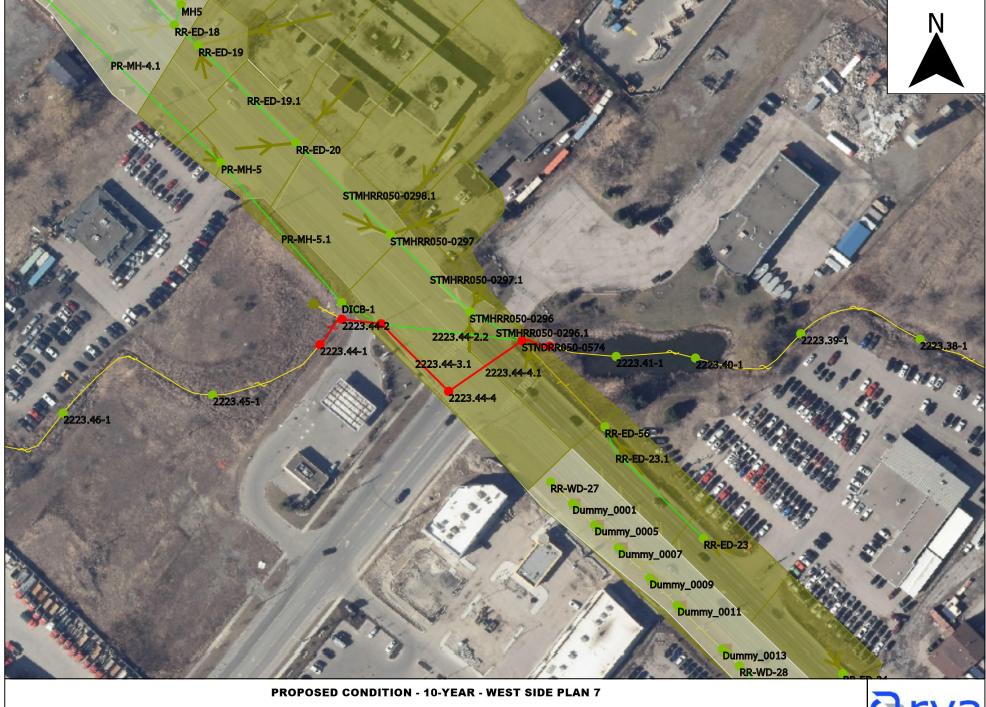
EXISTING CONDITION - 10-1 EAR - WEST SIDE I EAR /

RVA PROJECT NO. 194615 FIGURE NO. 7A



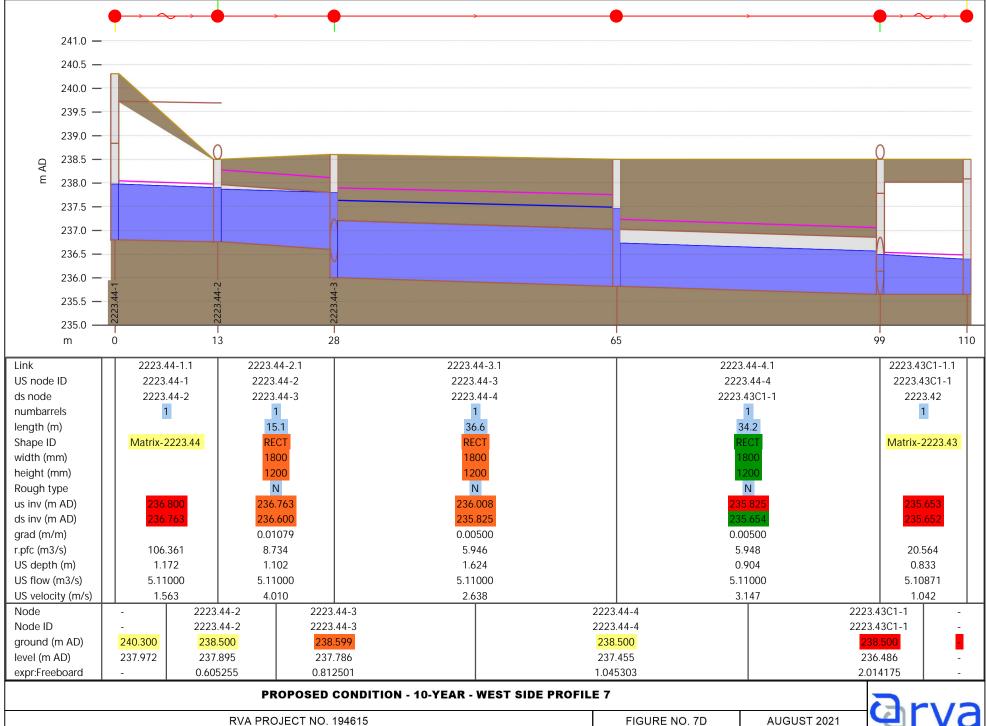




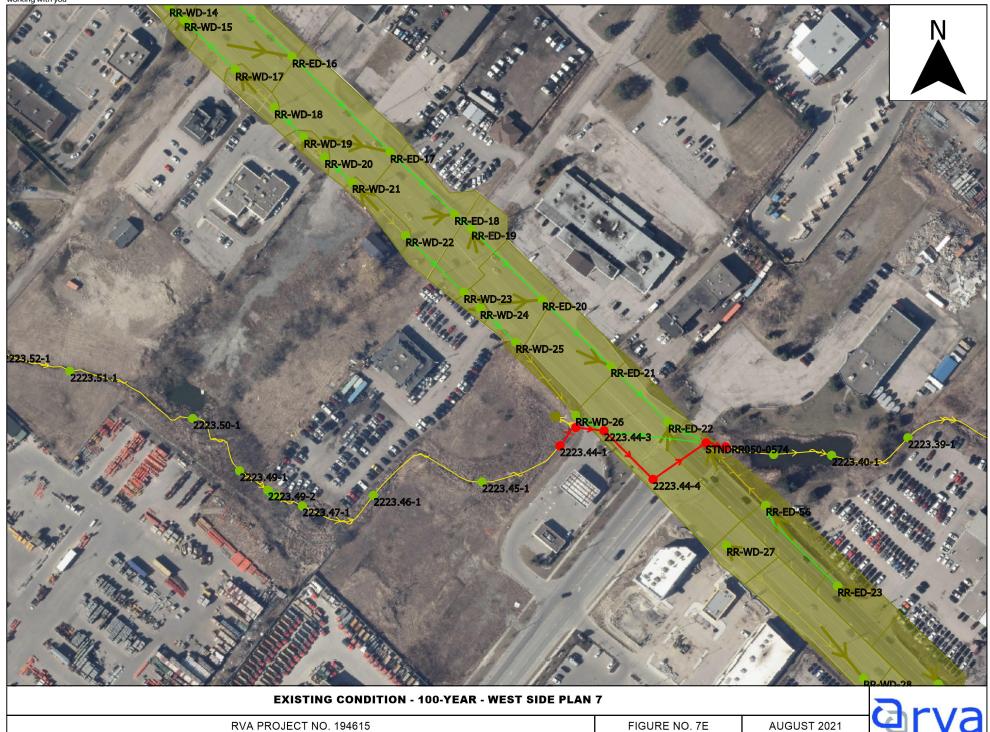


⁰²¹ **arva**

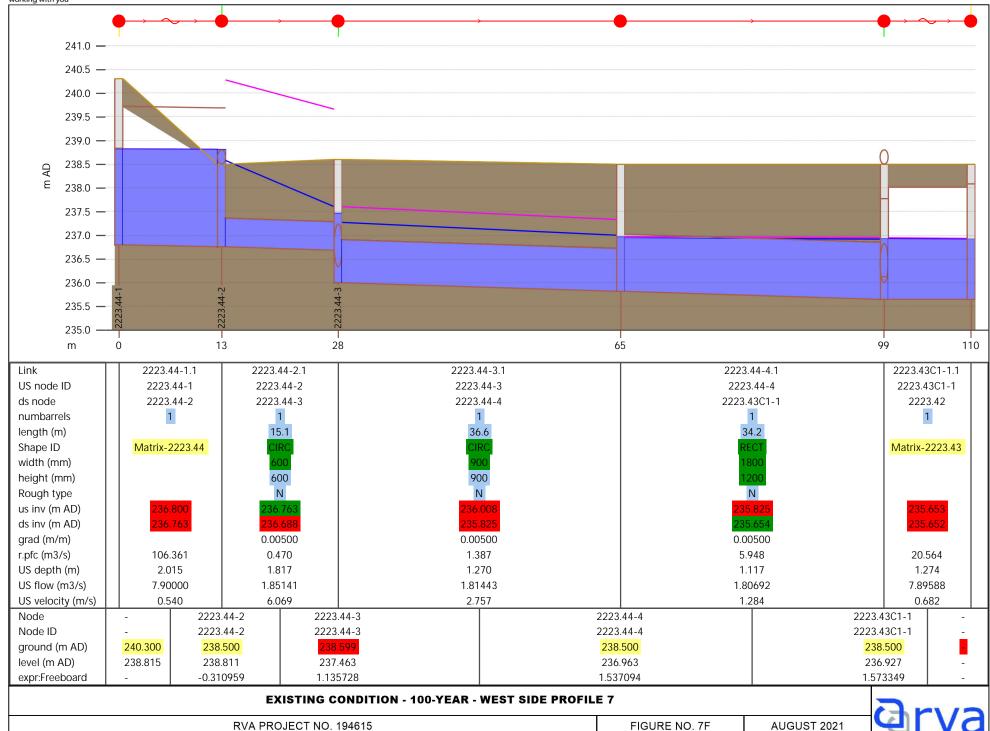




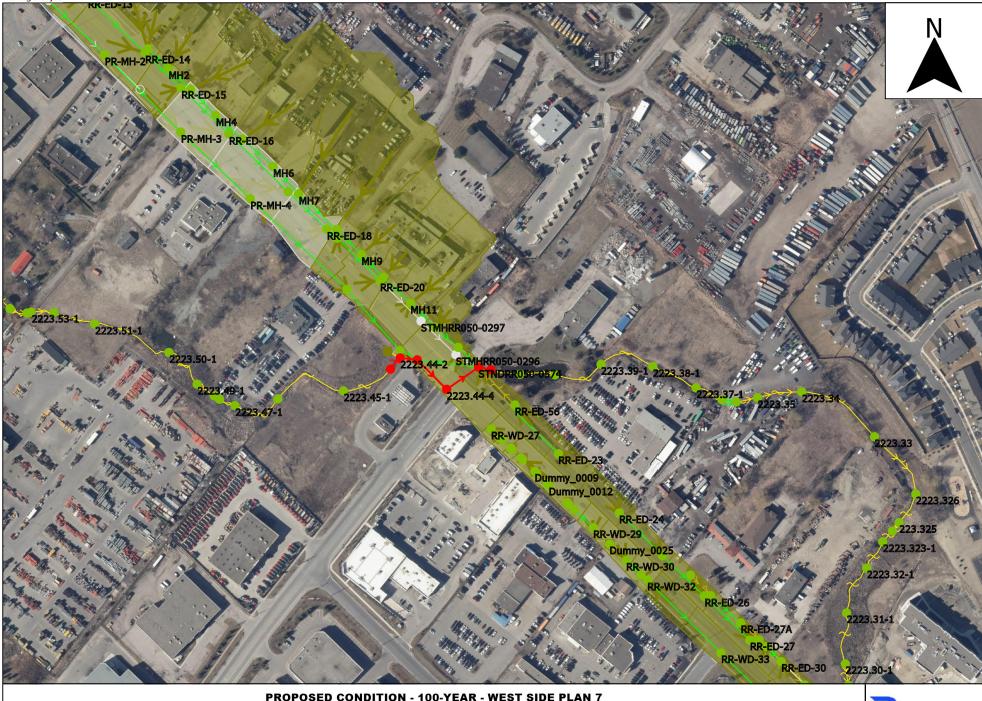












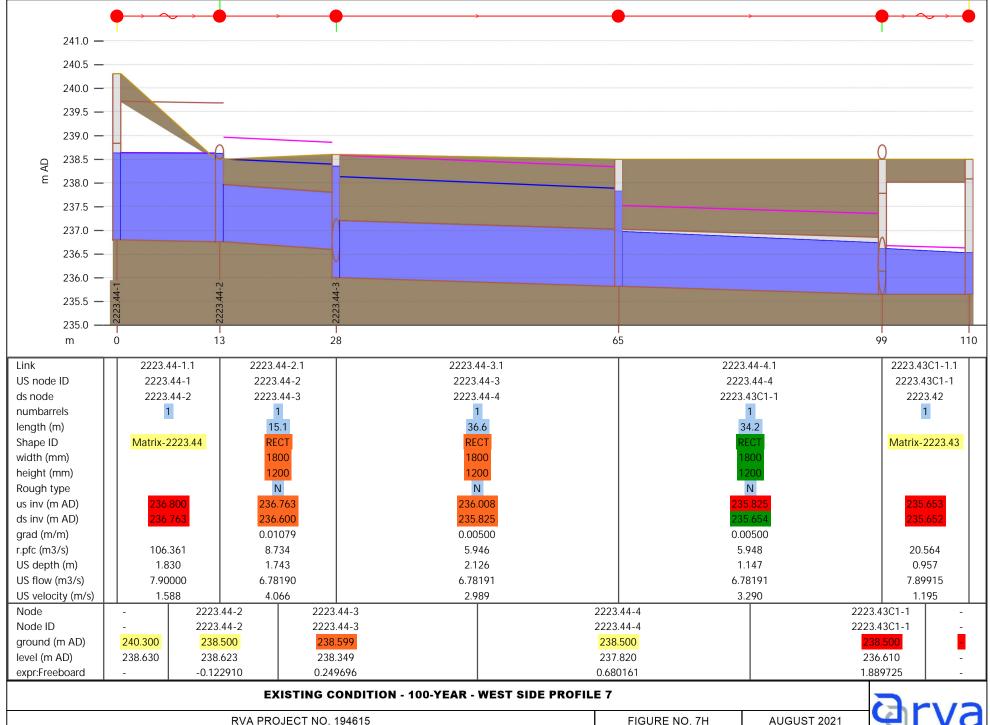
PROPOSED CONDITION - 100-YEAR - WEST SIDE PLAN 7

RVA PROJECT NO. 194615

FIGURE NO. 7G









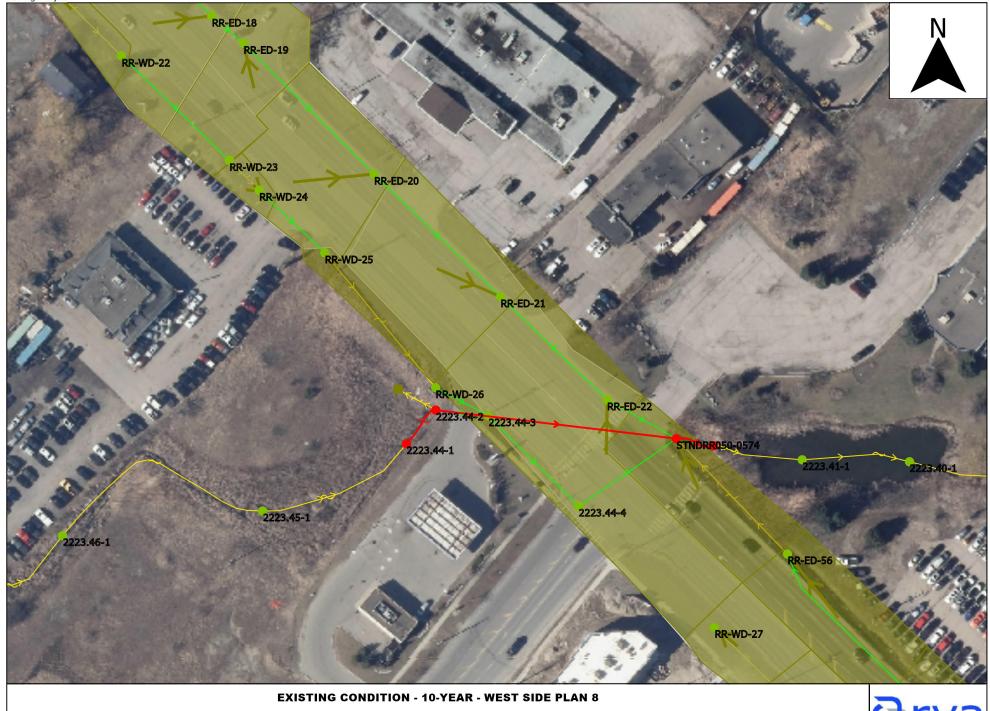
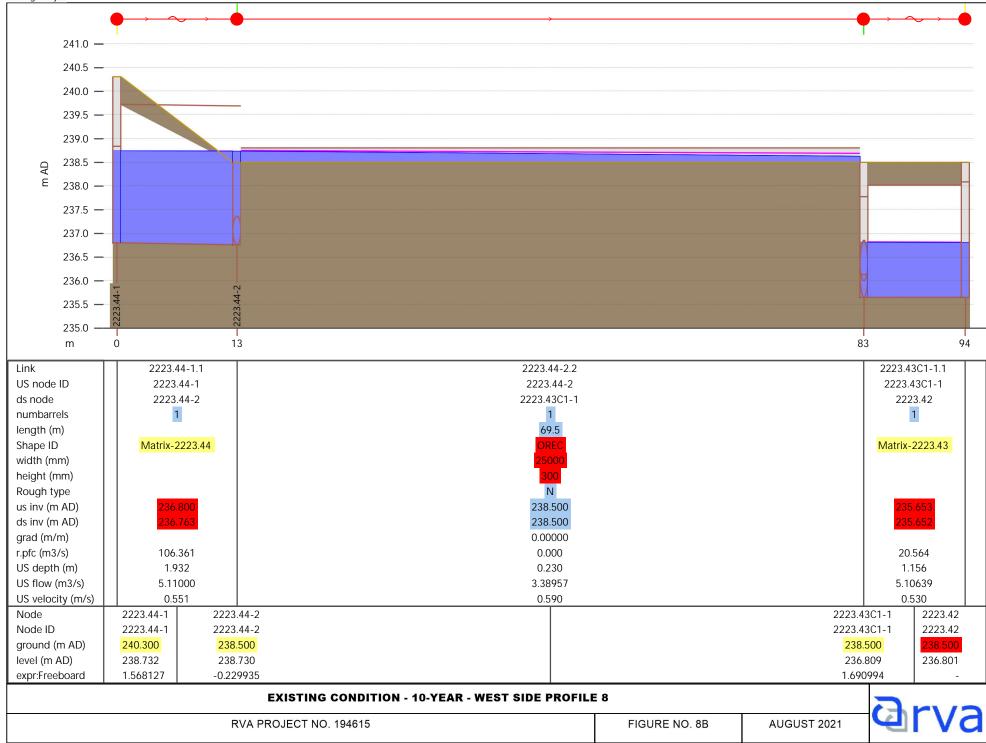


FIGURE NO. 8A







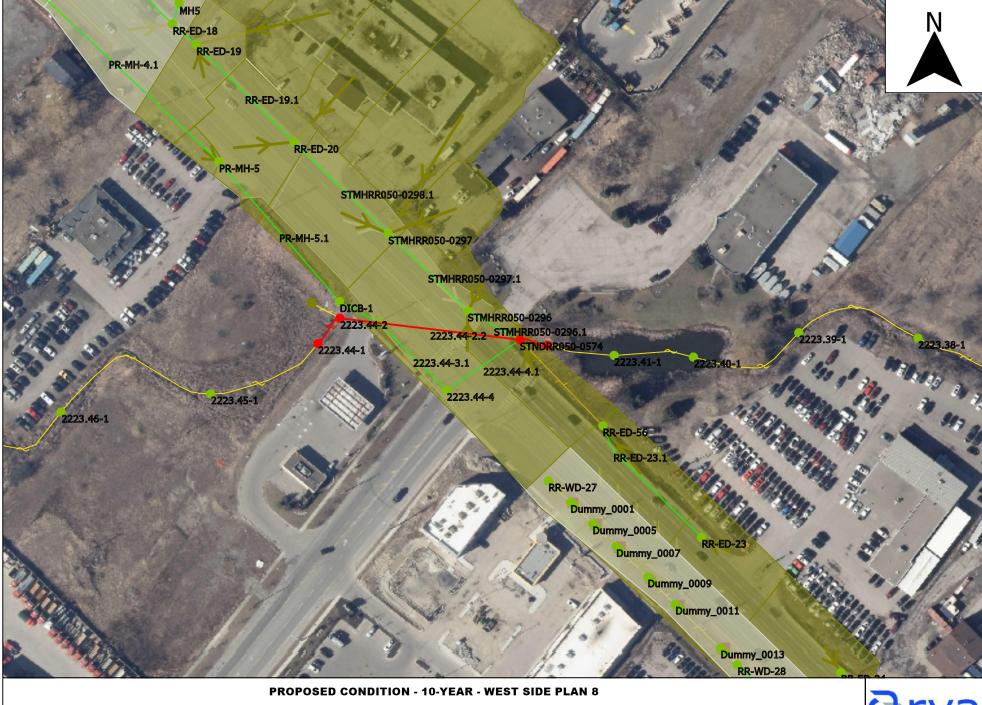
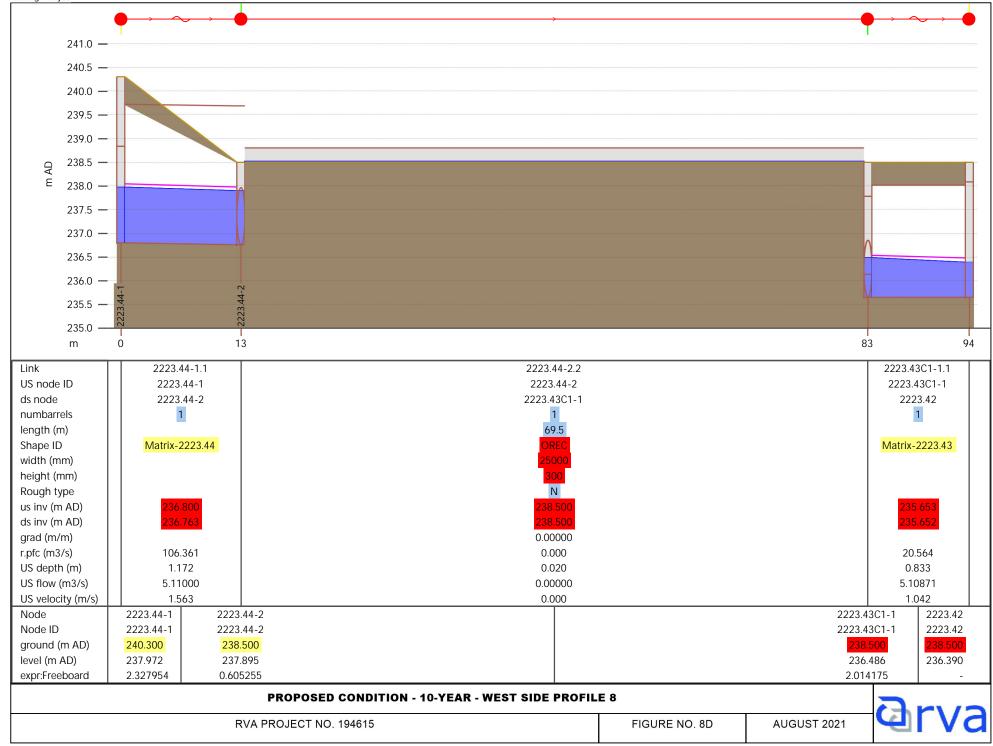
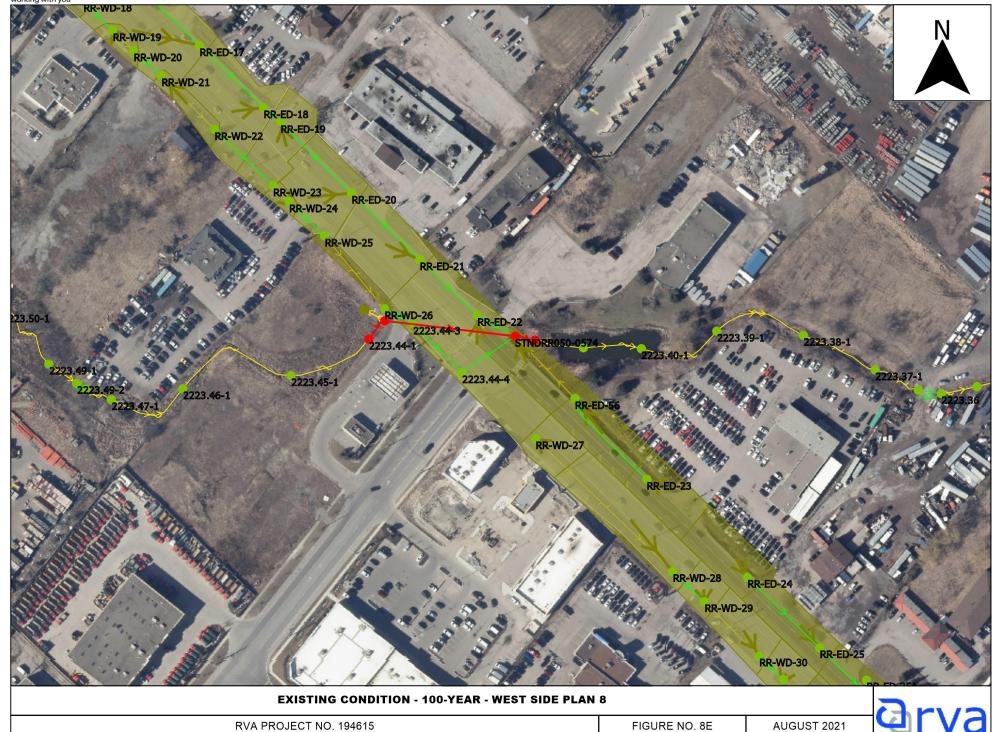


FIGURE NO. 8C AUGUST 2021

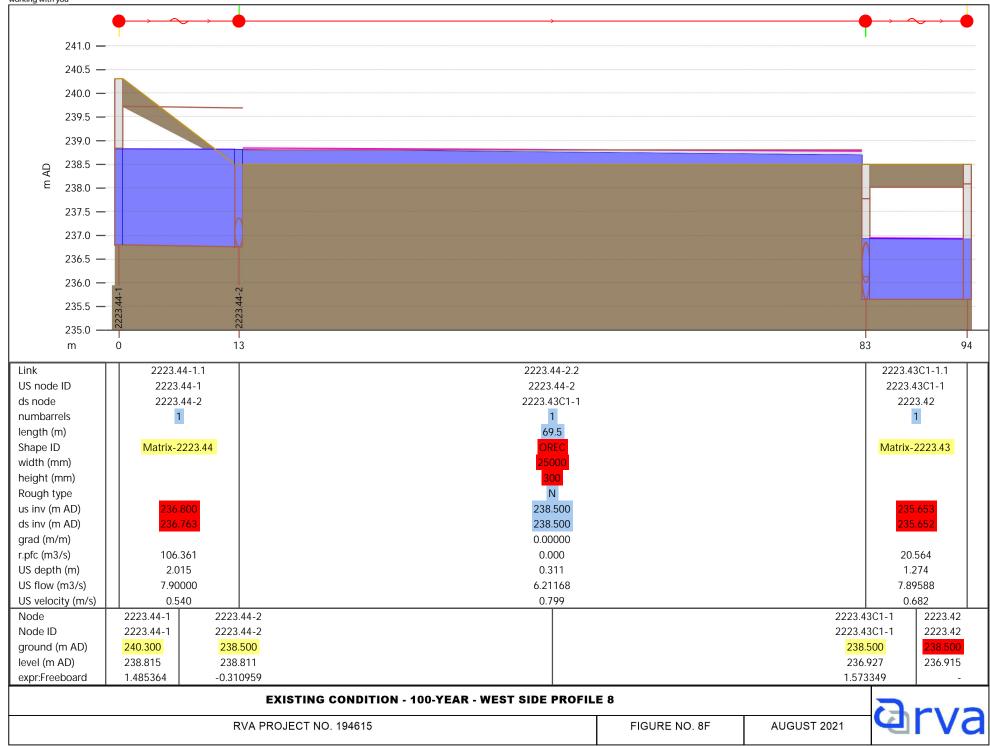














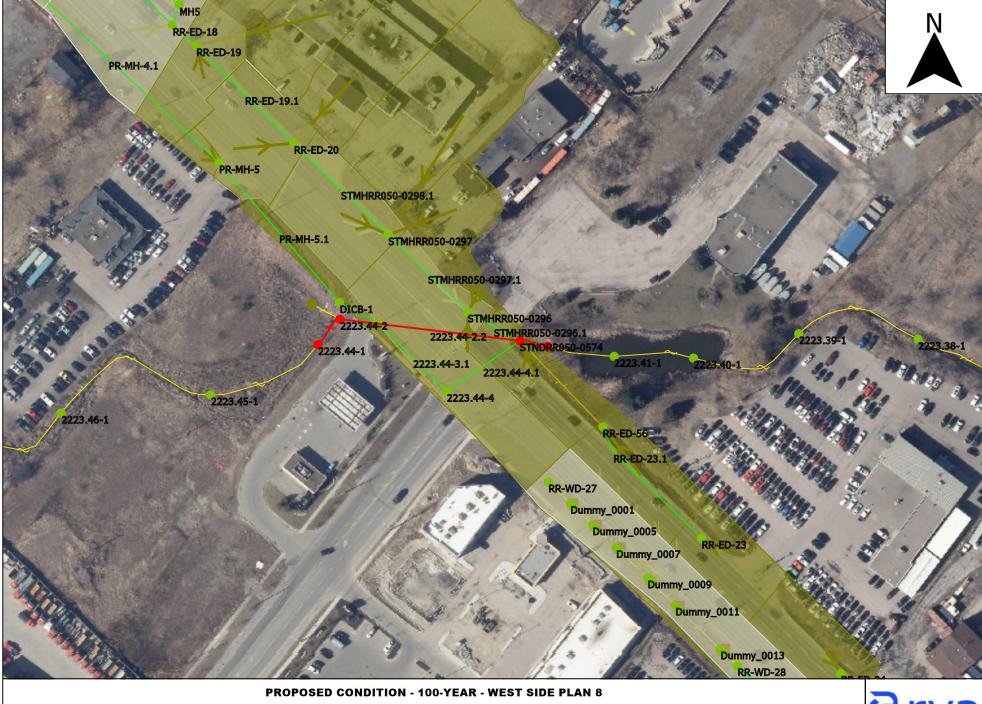
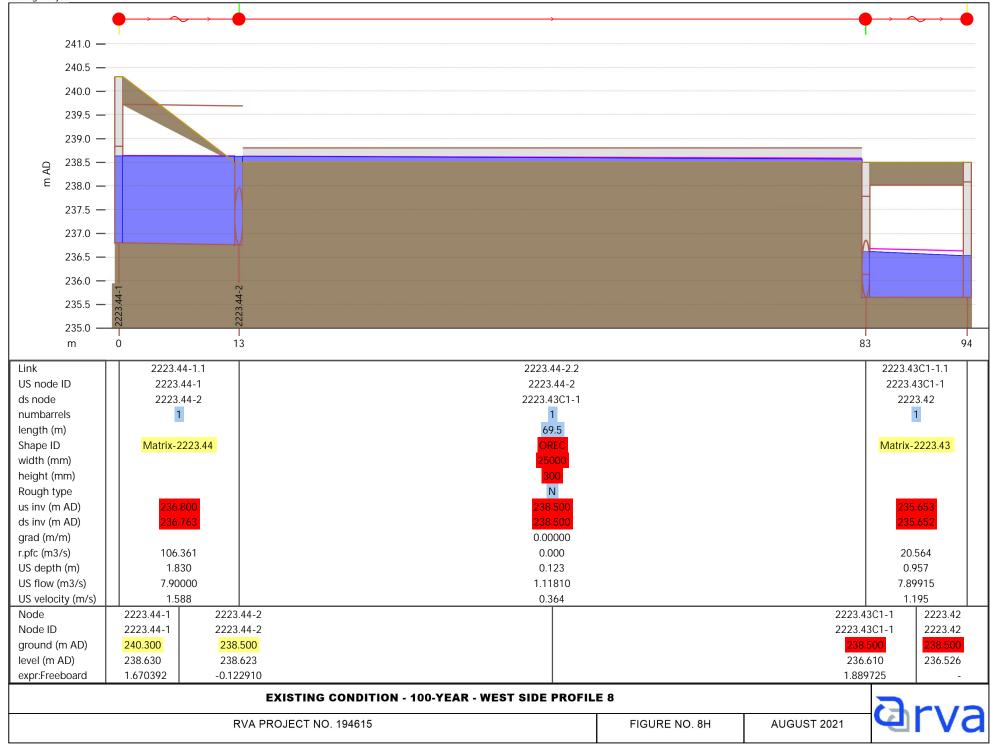


FIGURE NO. 8G AUGUST 2021







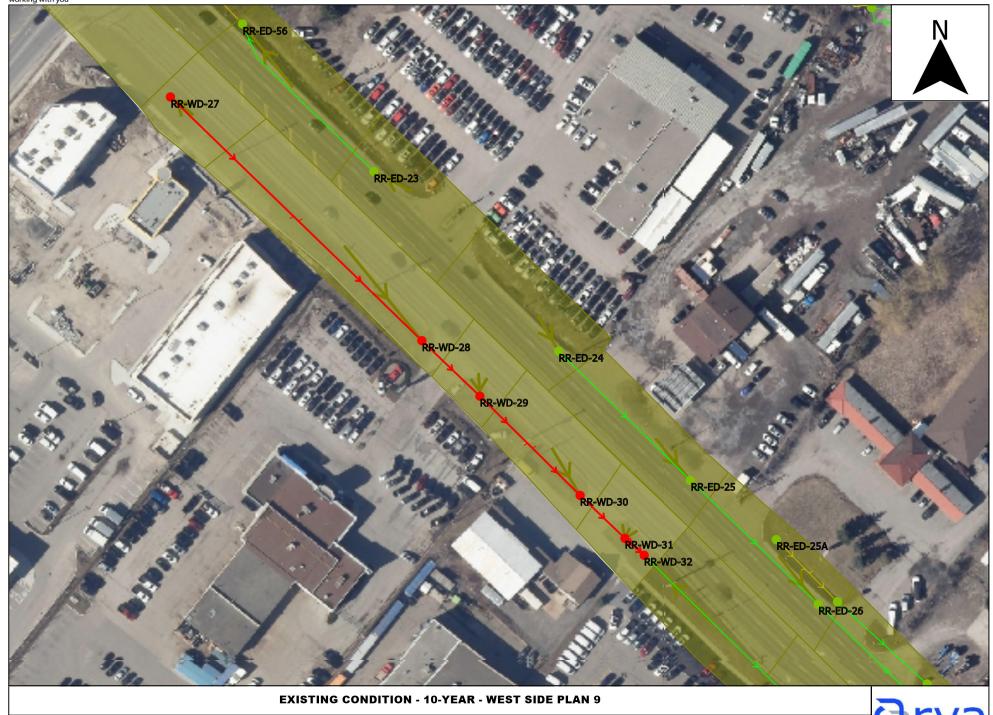
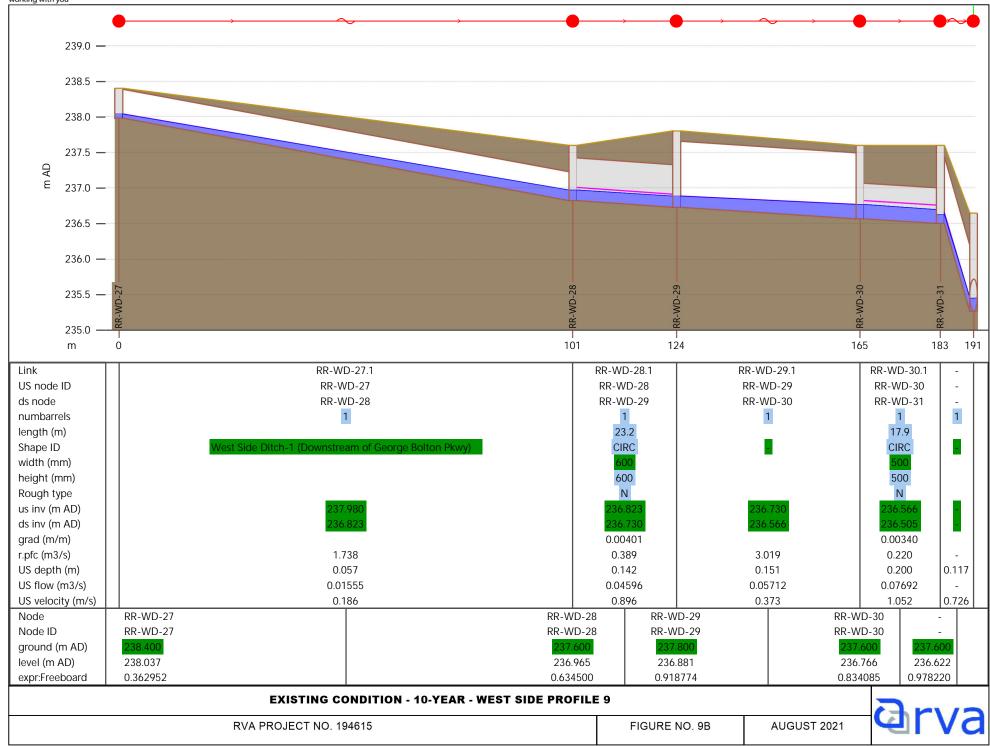
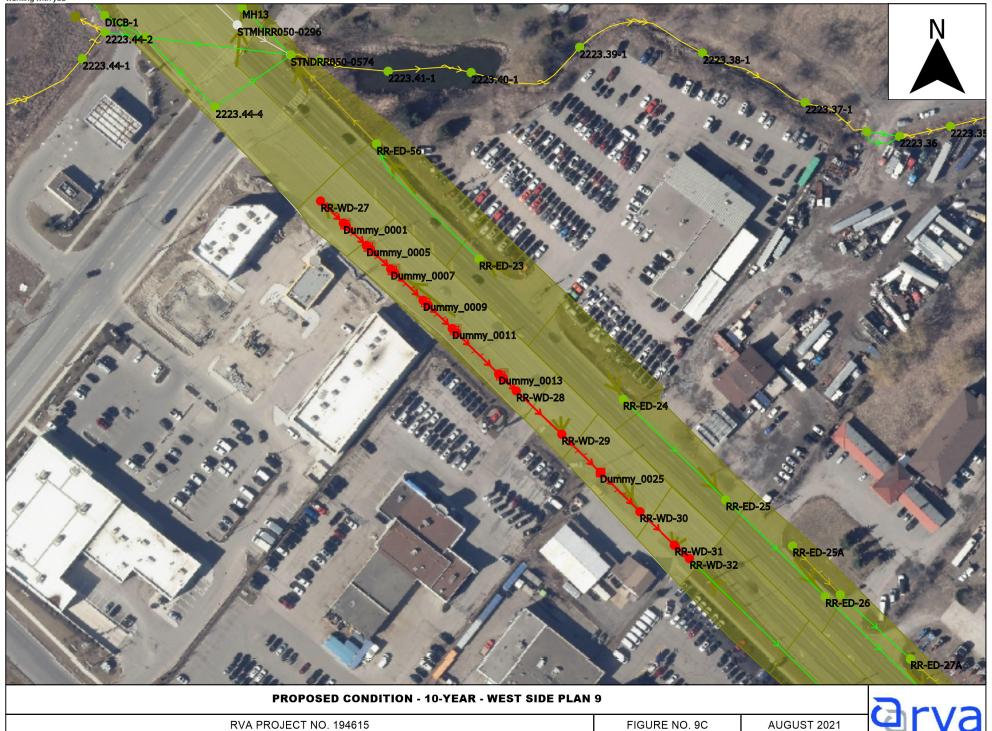


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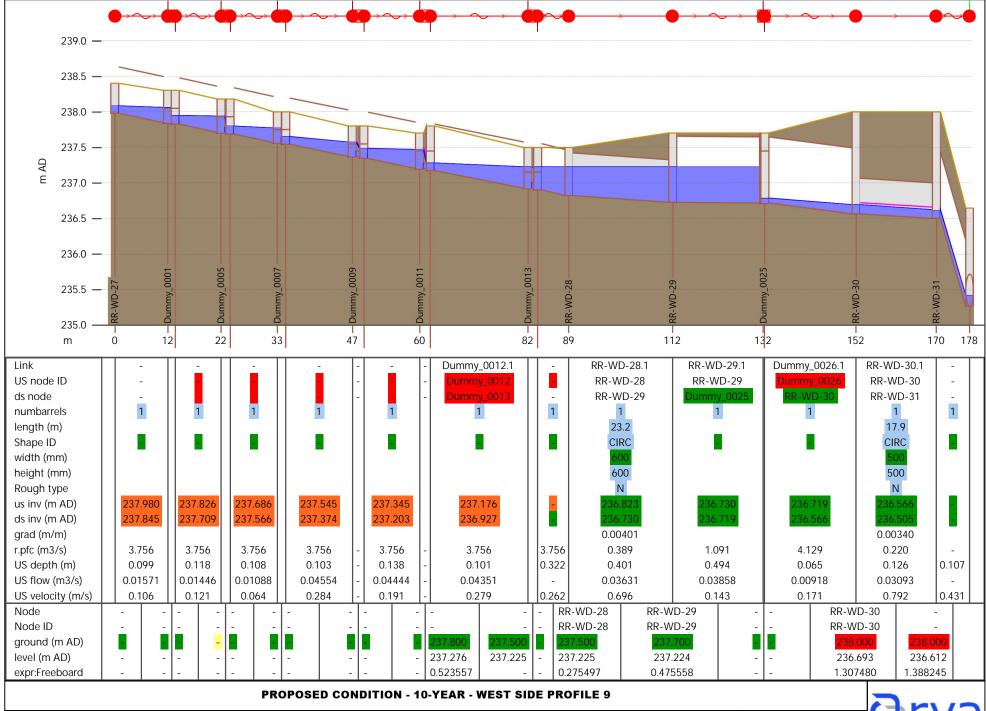












RVA PROJECT NO. 194615 FIGURE NO. 9D









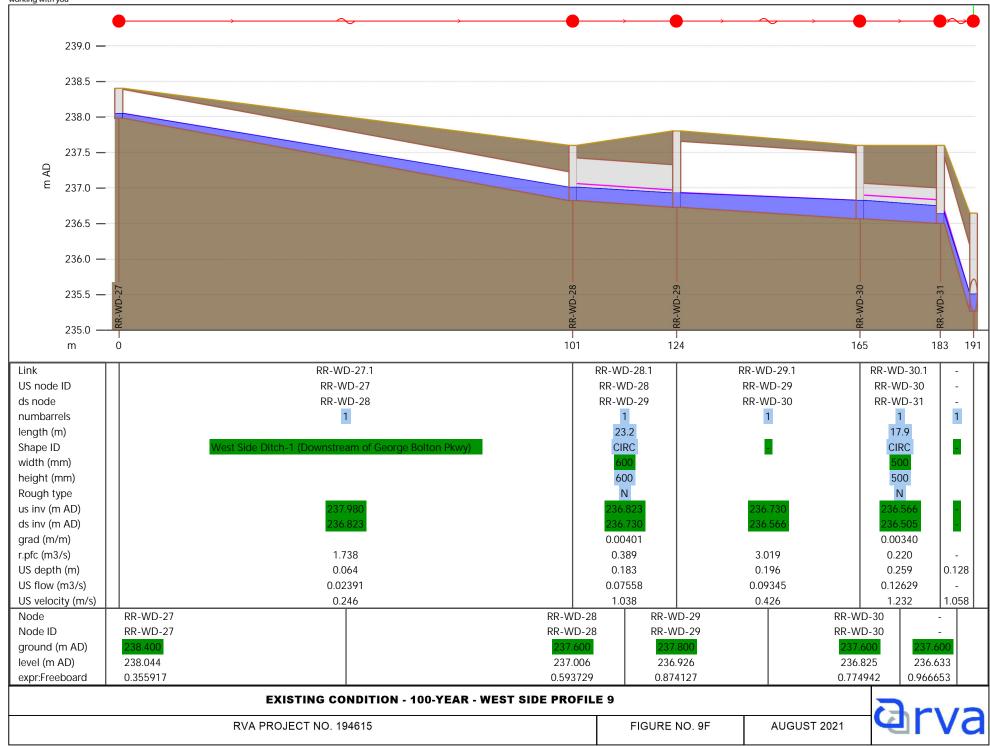
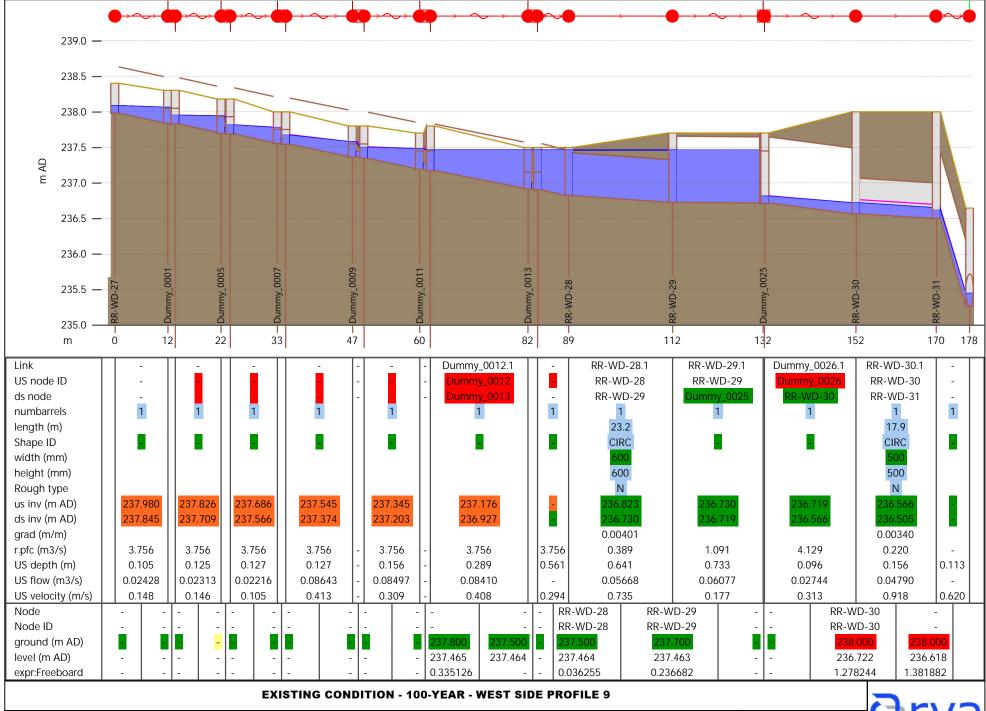






FIGURE NO. 9G

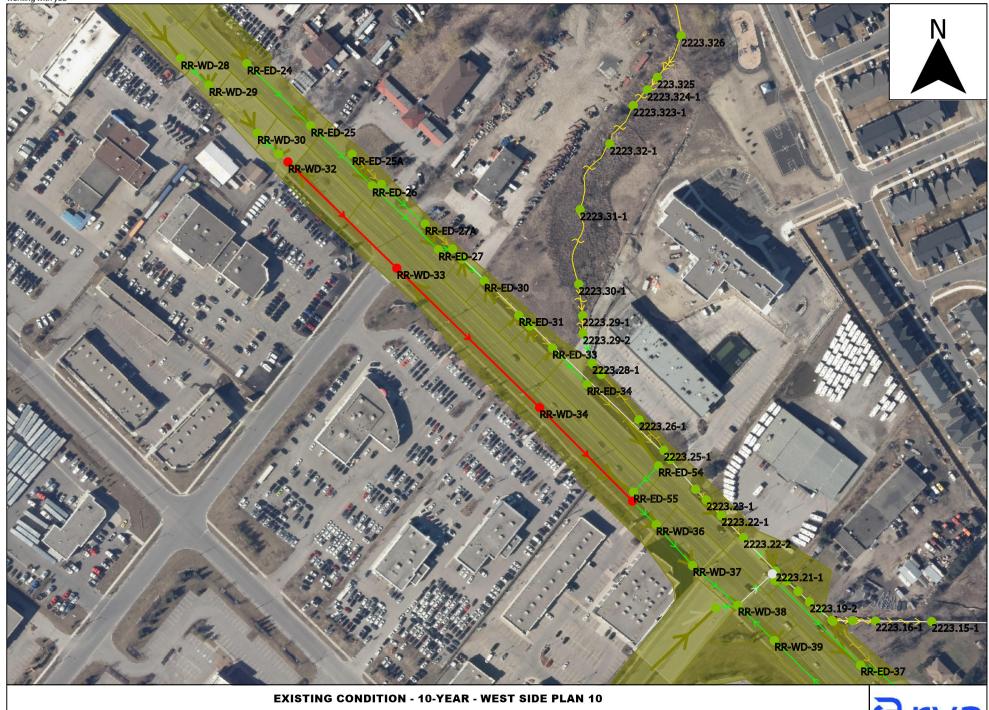




RVA PROJECT NO. 194615 FIGURE NO. 9H



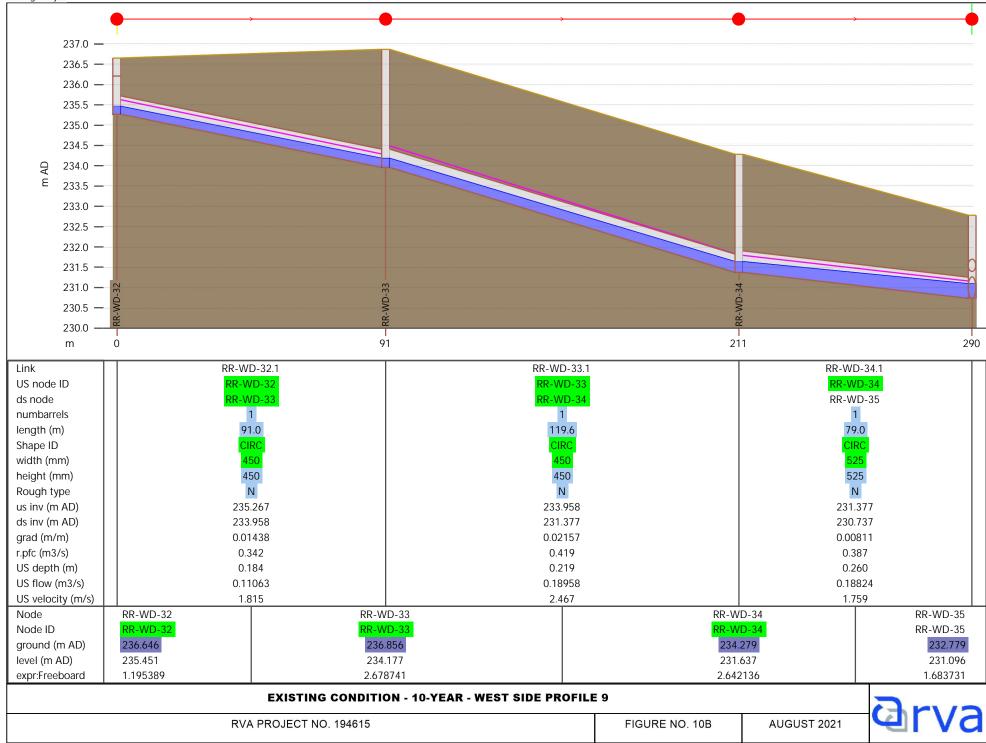




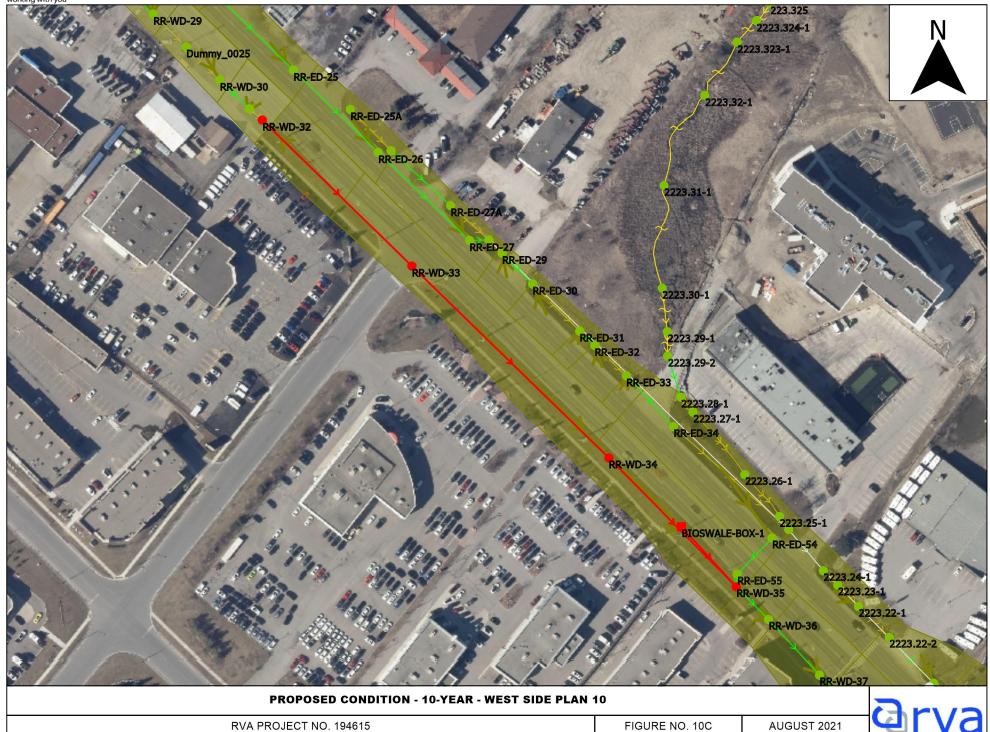
AUGUST 2021

FIGURE NO. 10A

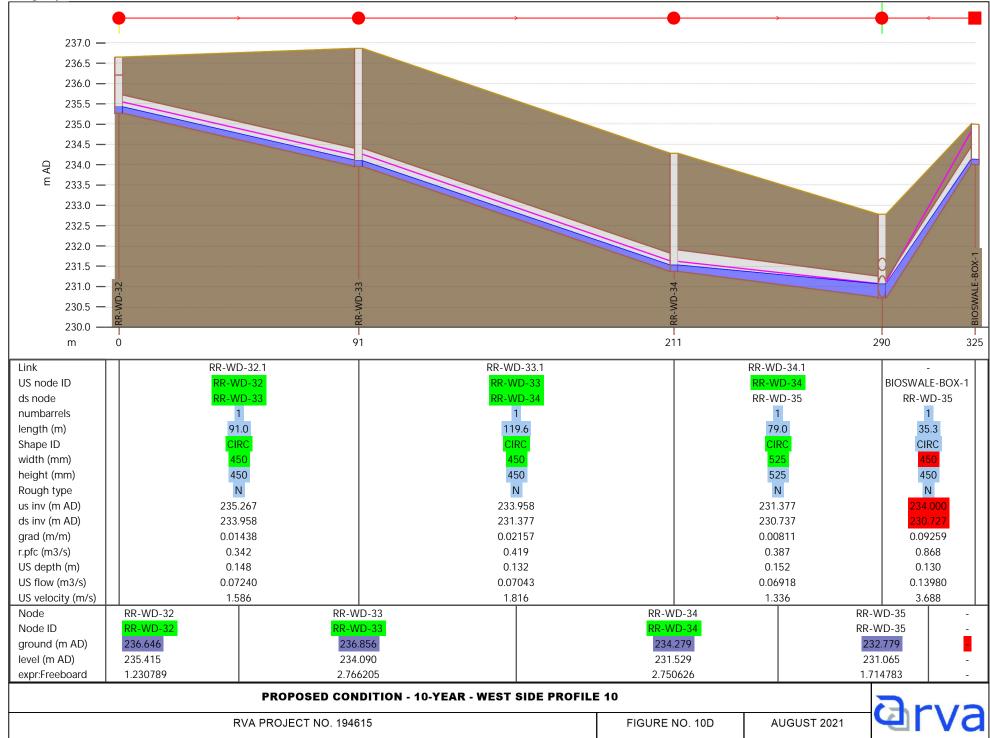




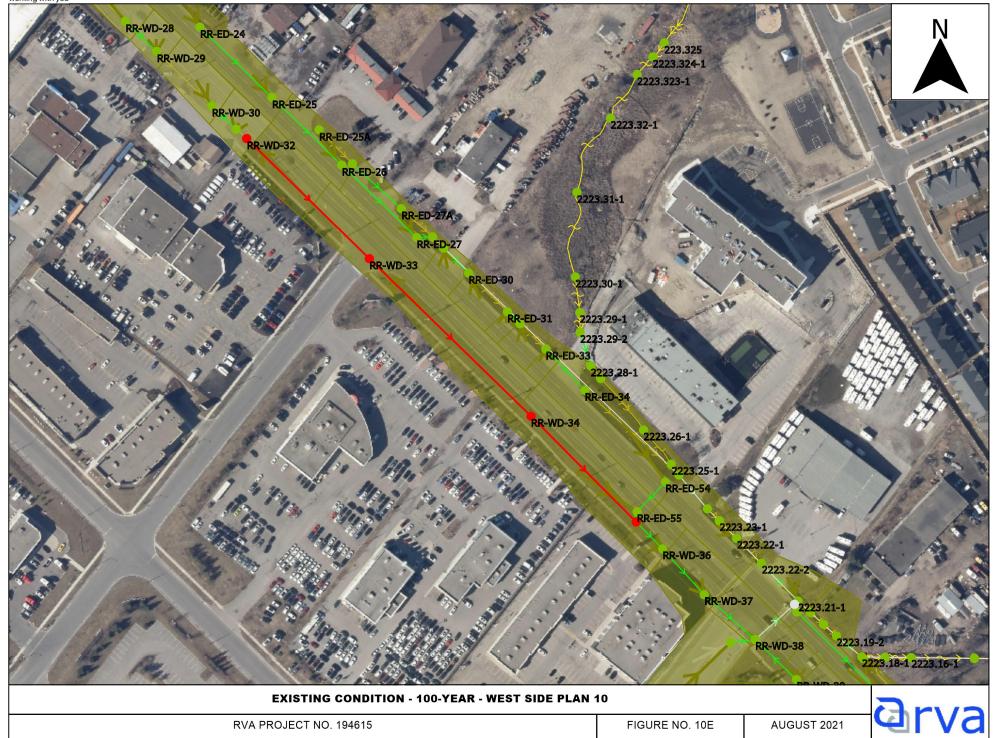




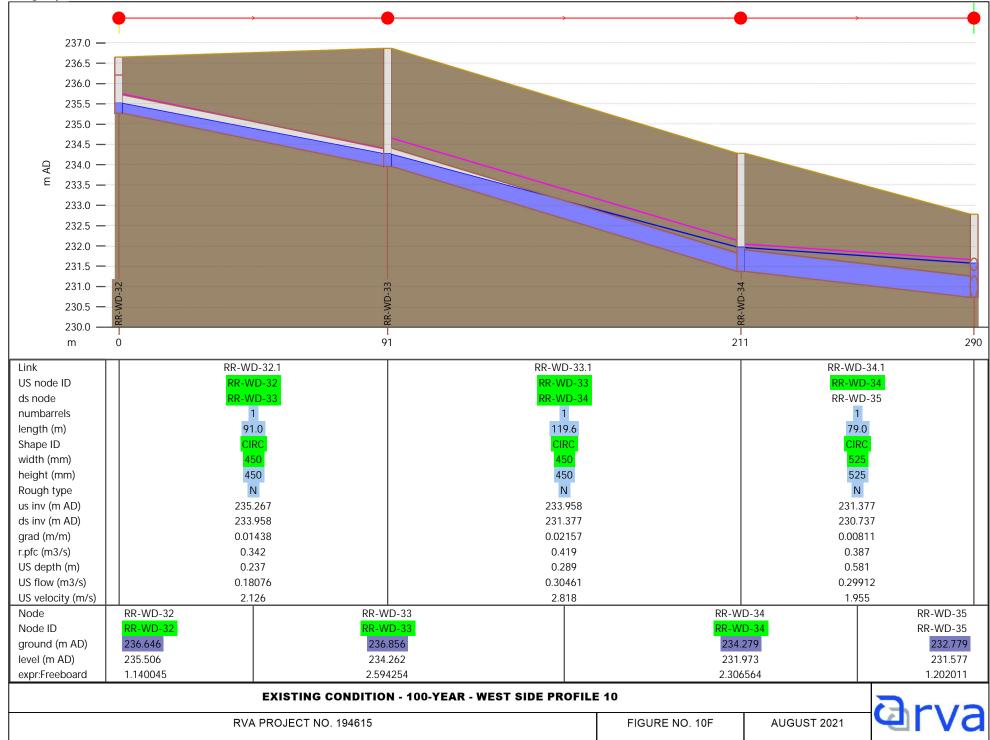




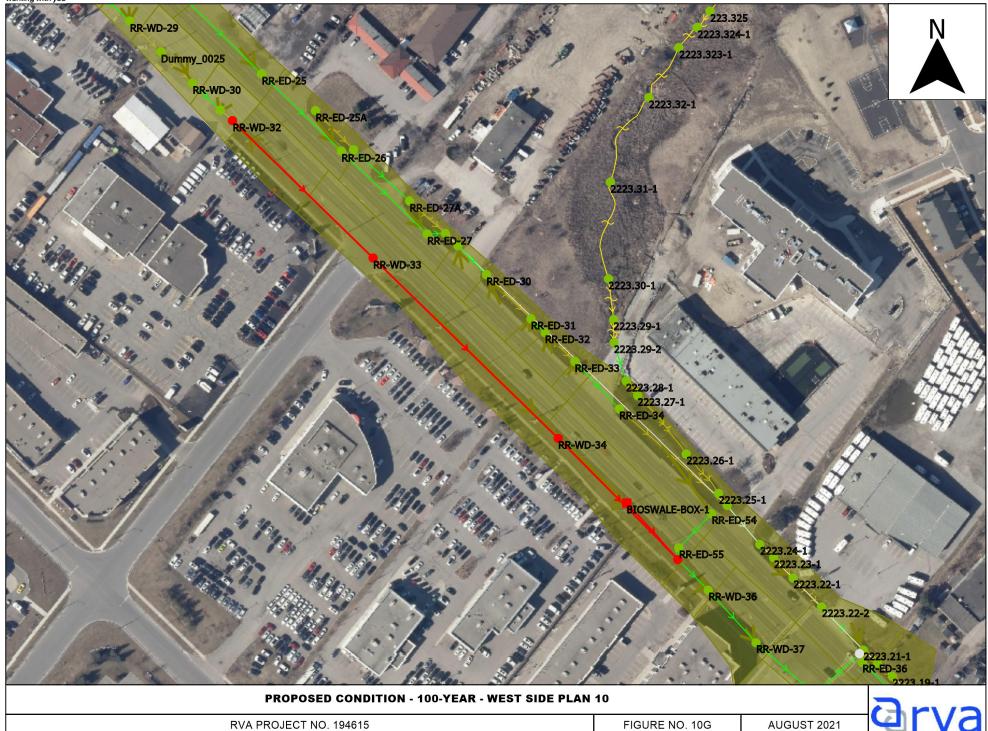




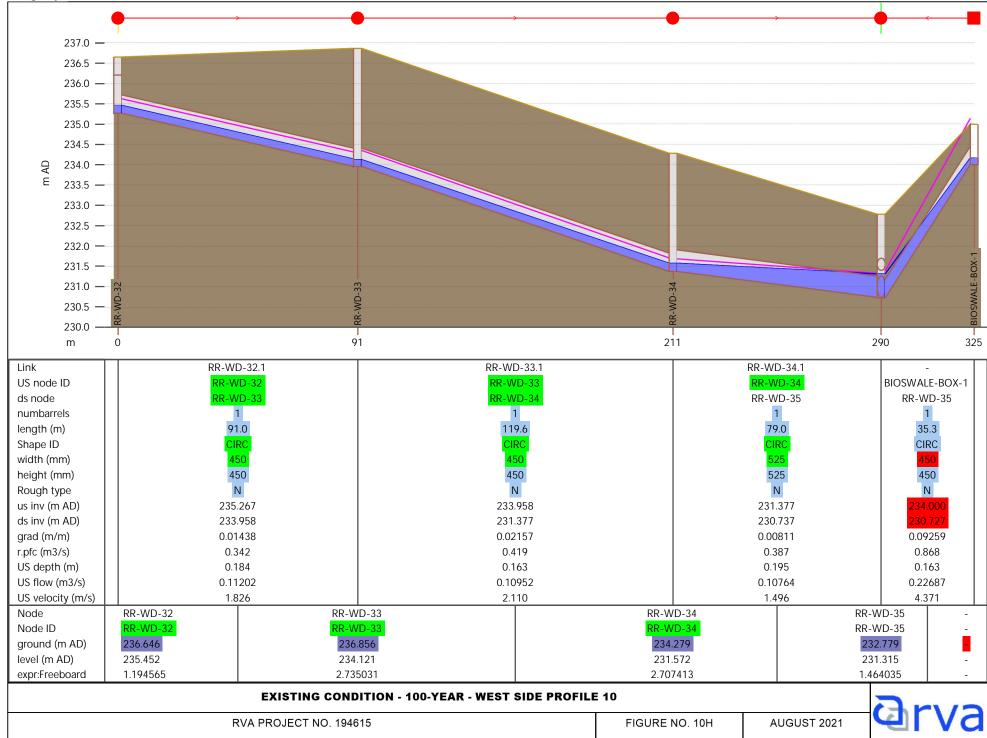




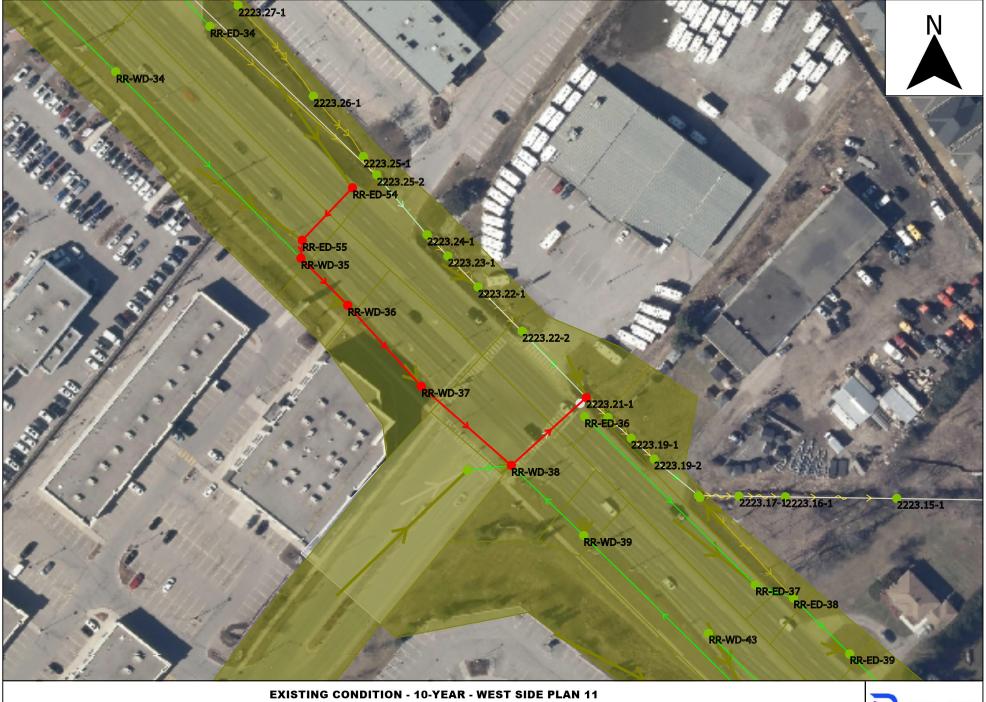






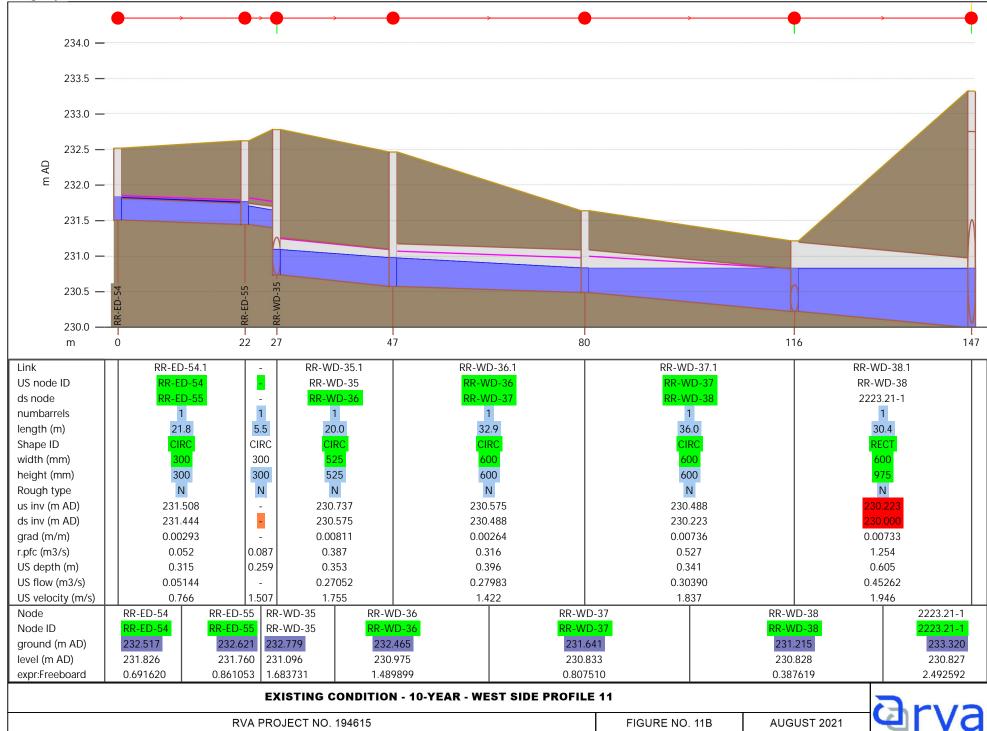




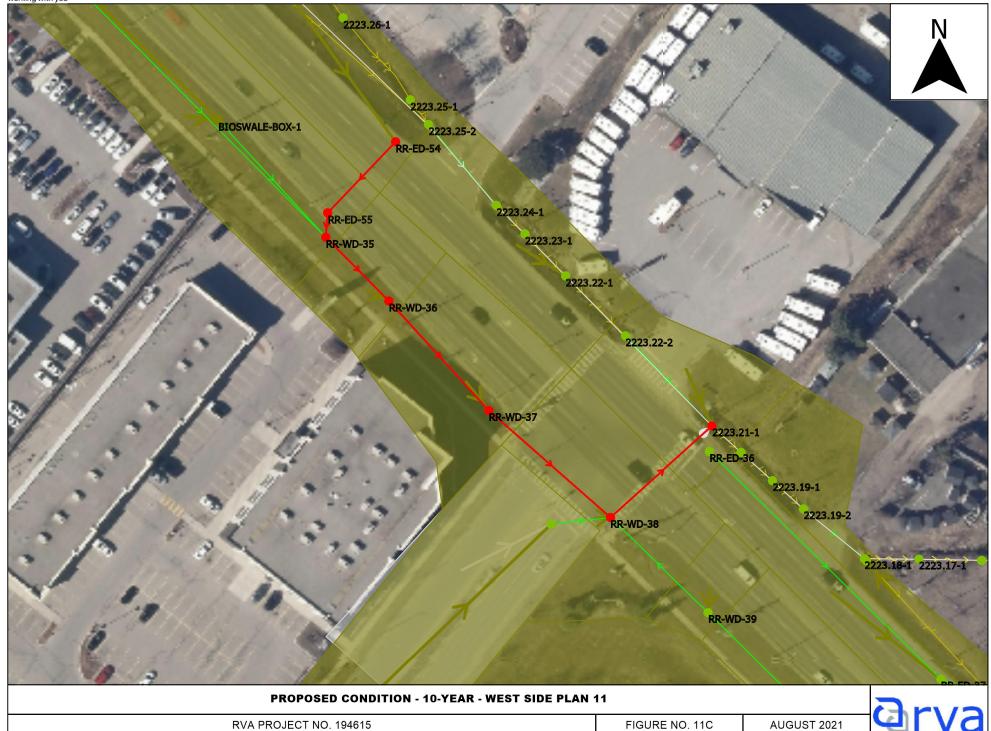


RVA PROJECT NO. 194615 FIGURE NO. 11A

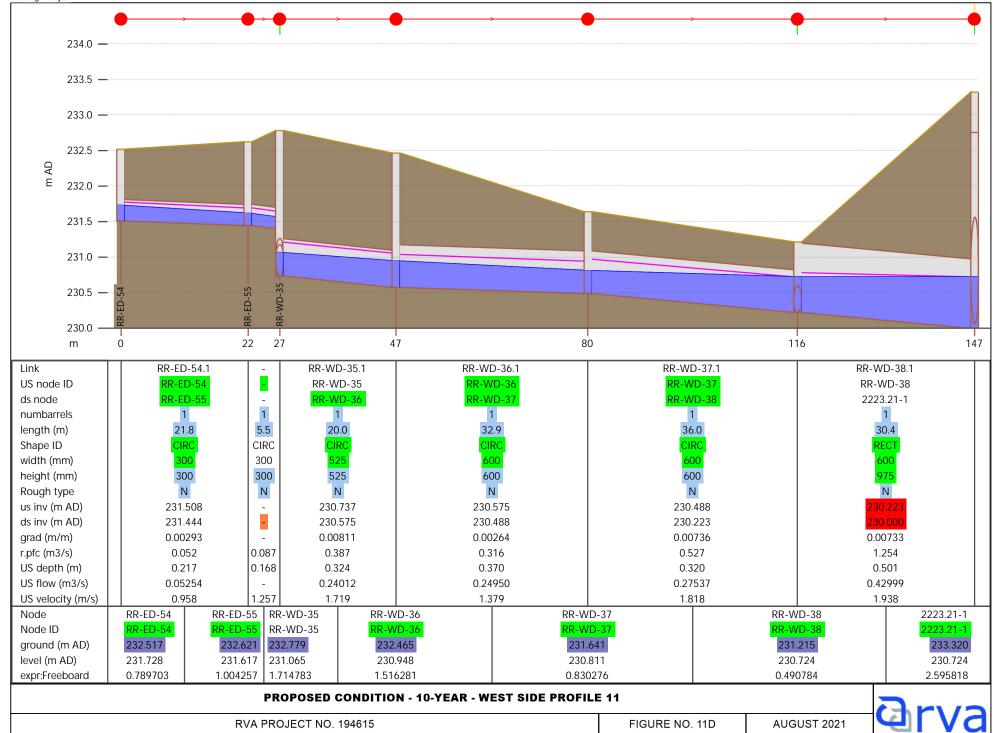














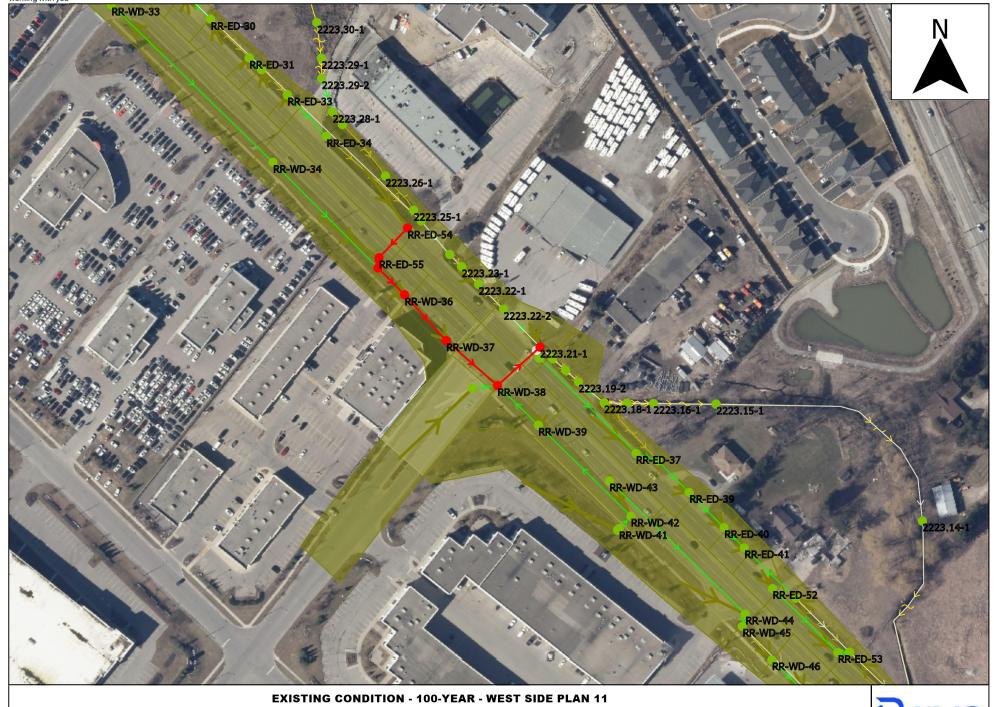
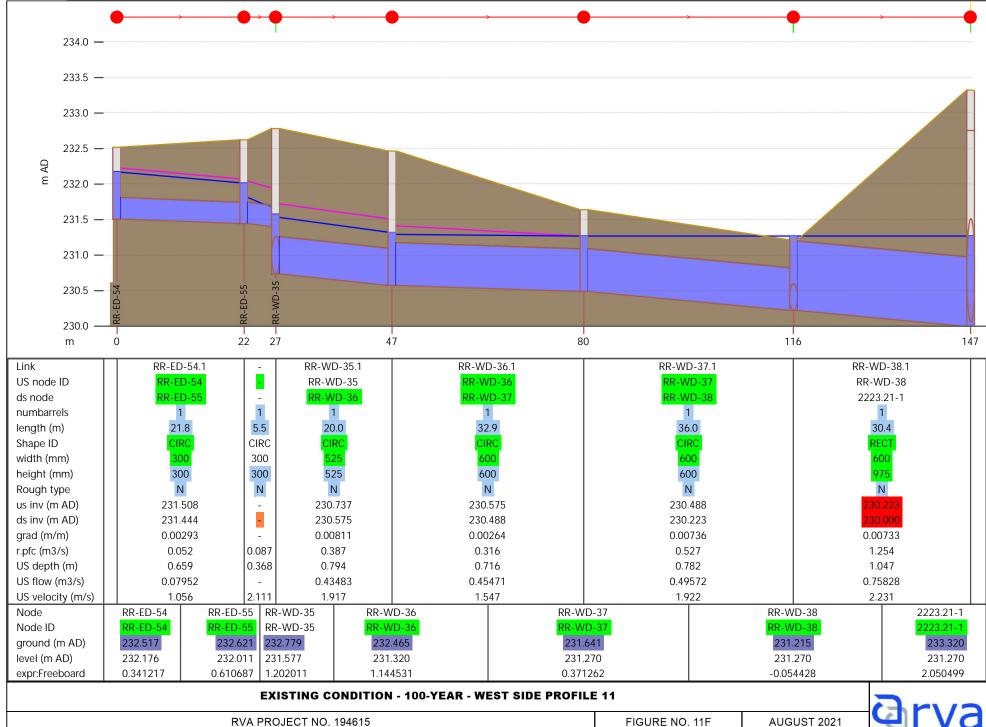
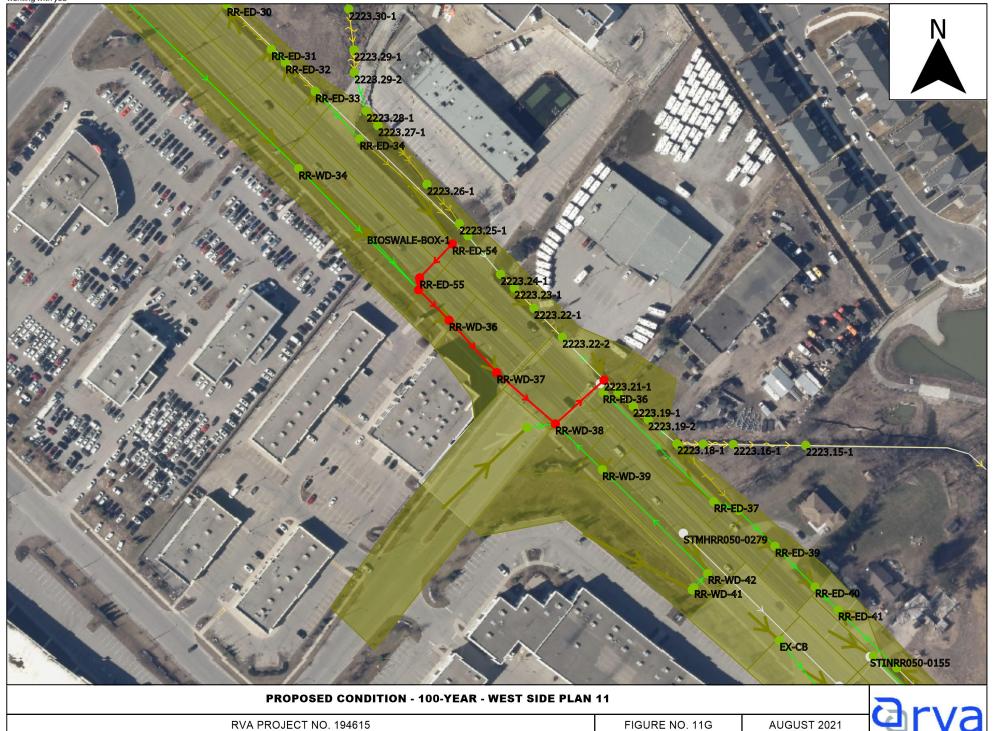


FIGURE NO. 11E AUGUST 2021

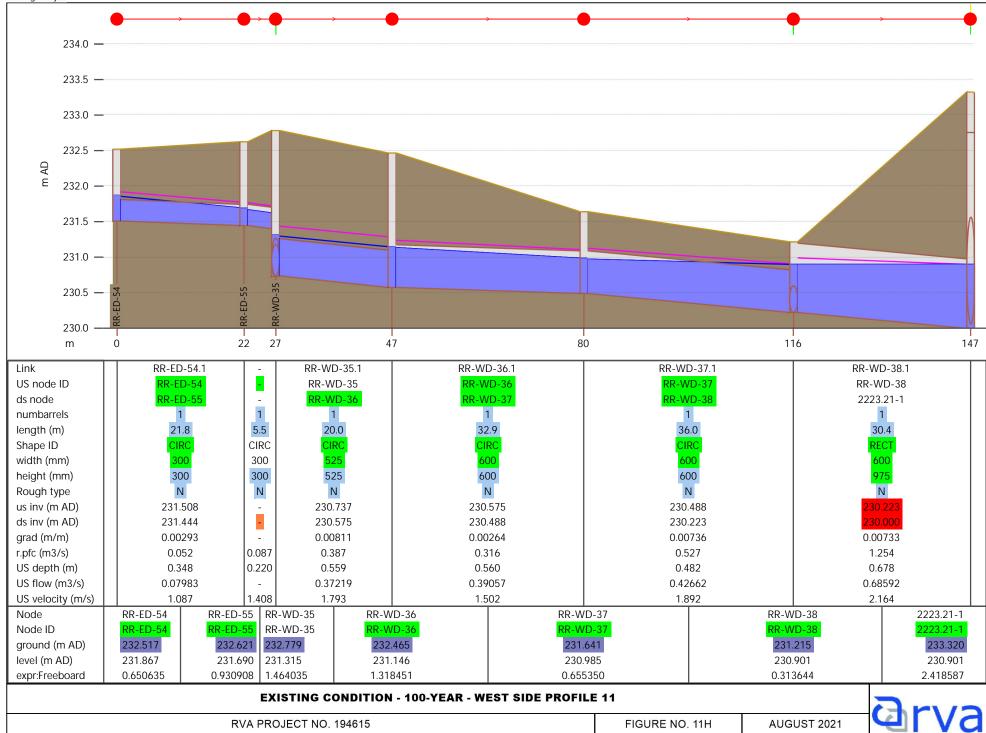




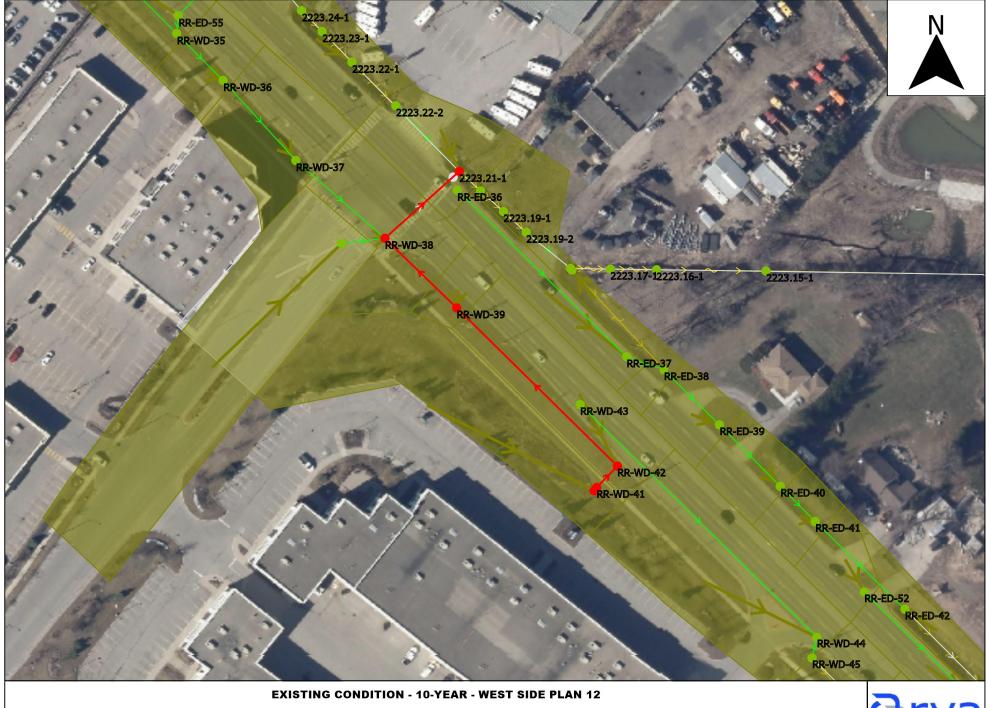






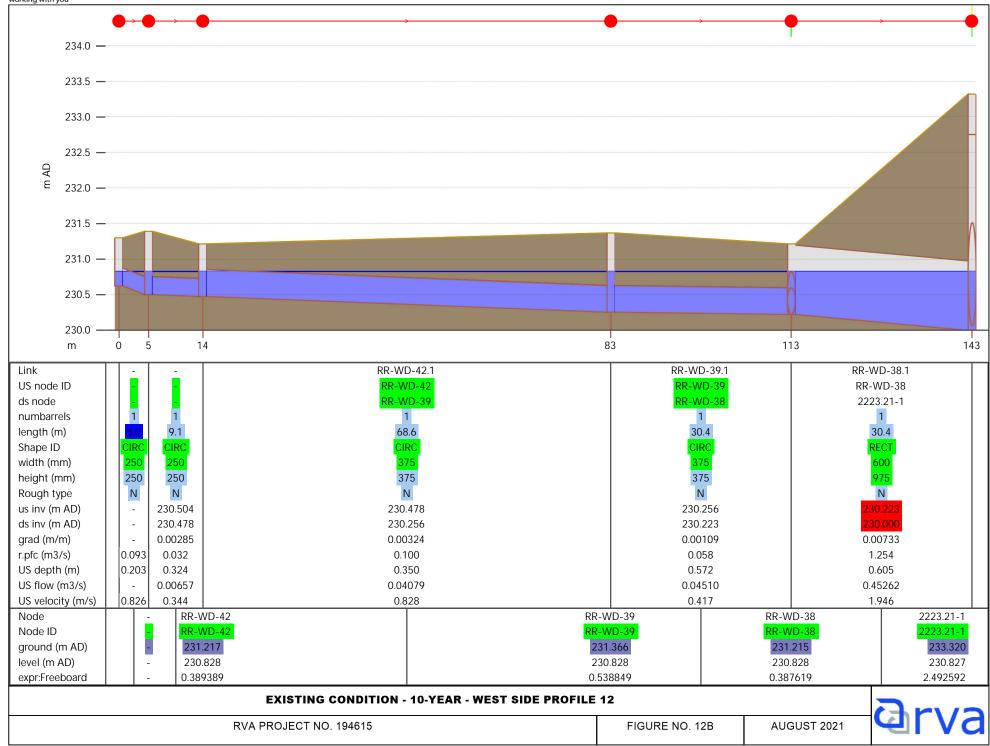




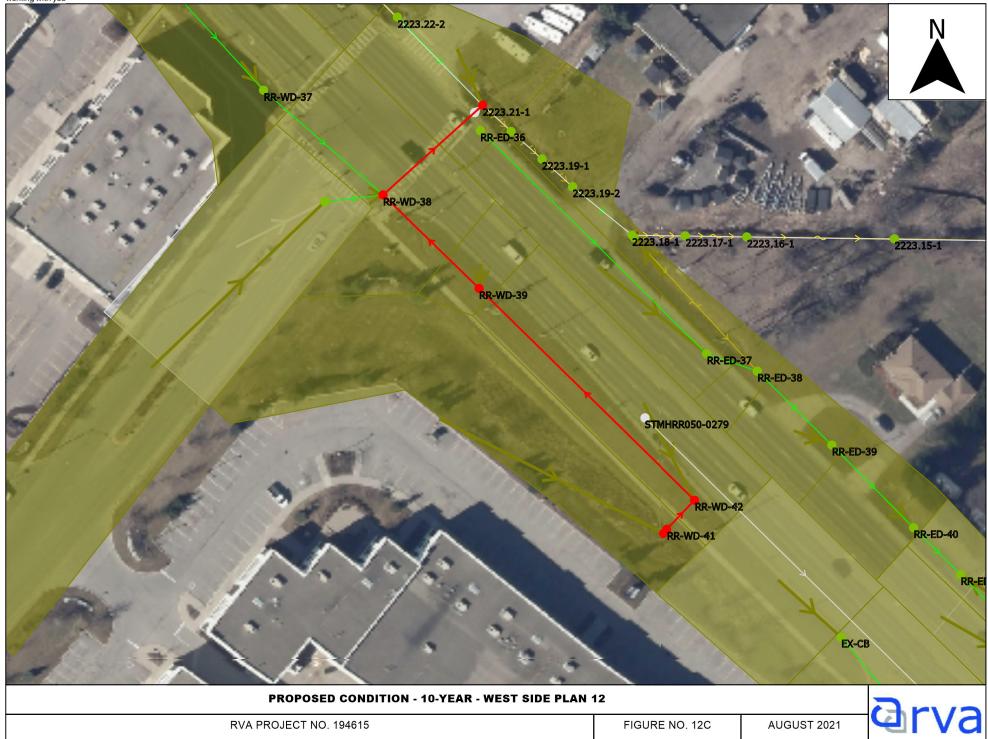




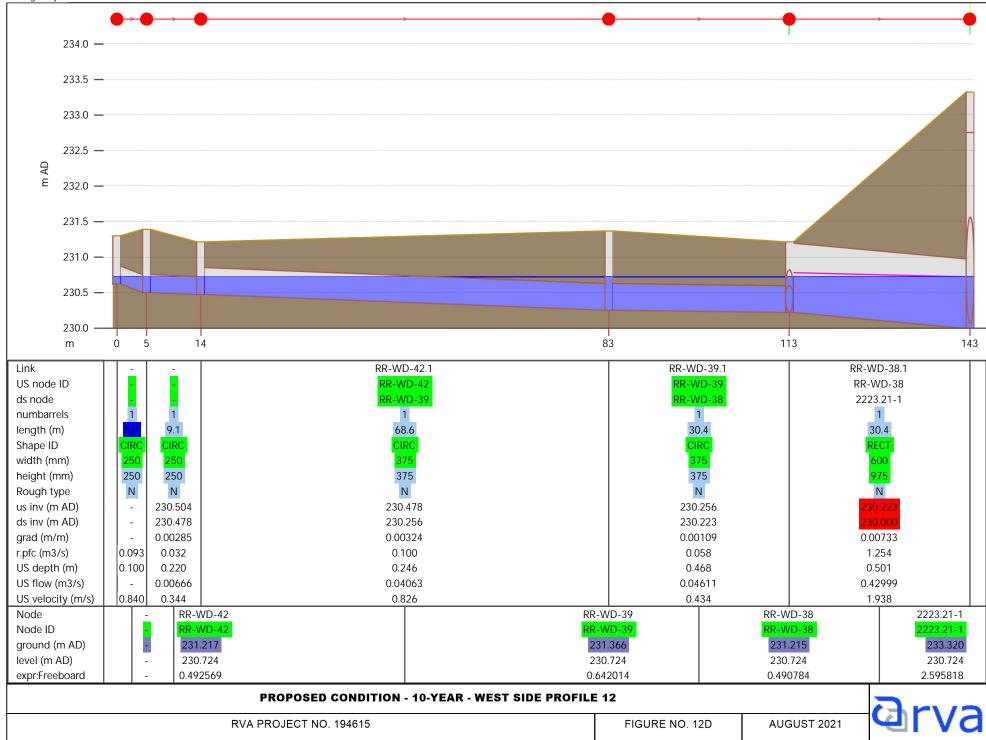




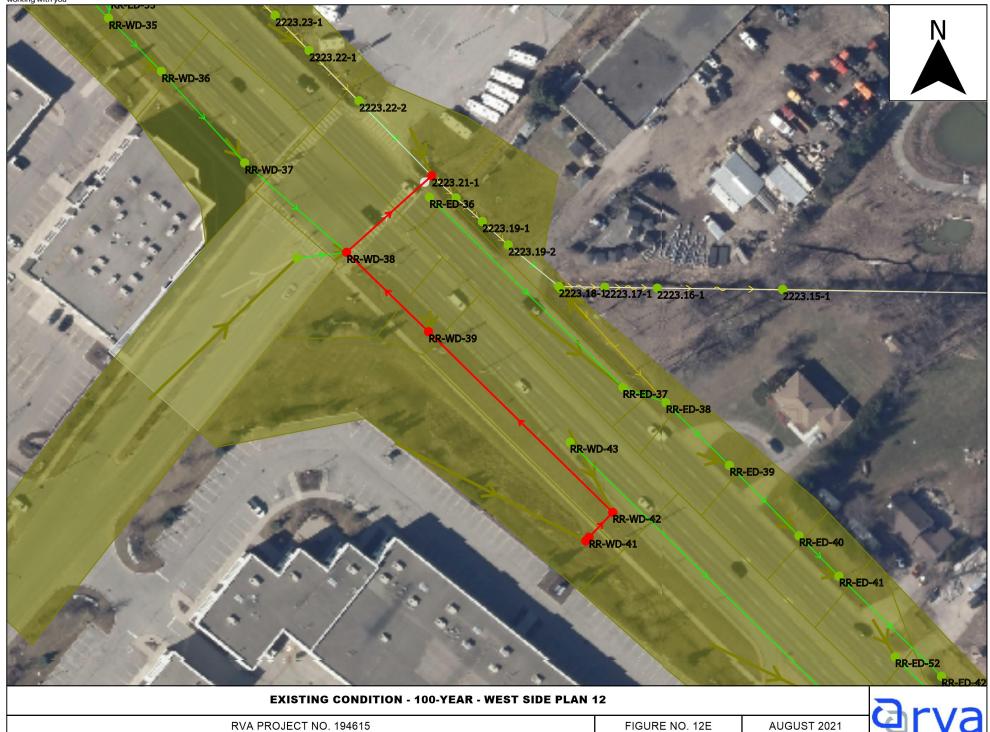




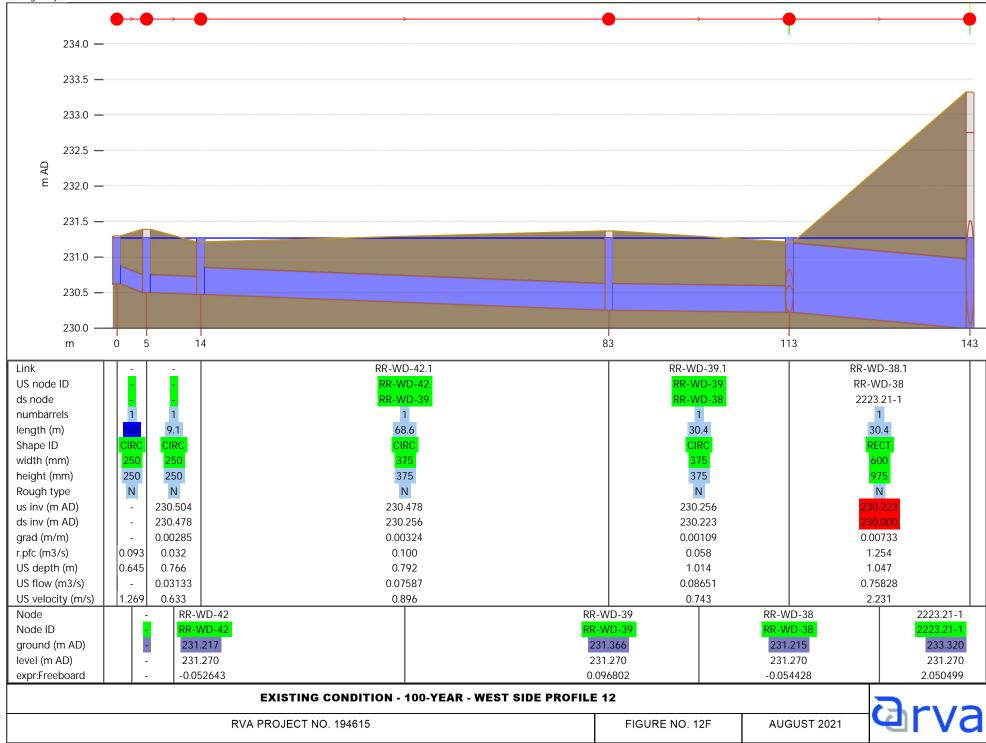




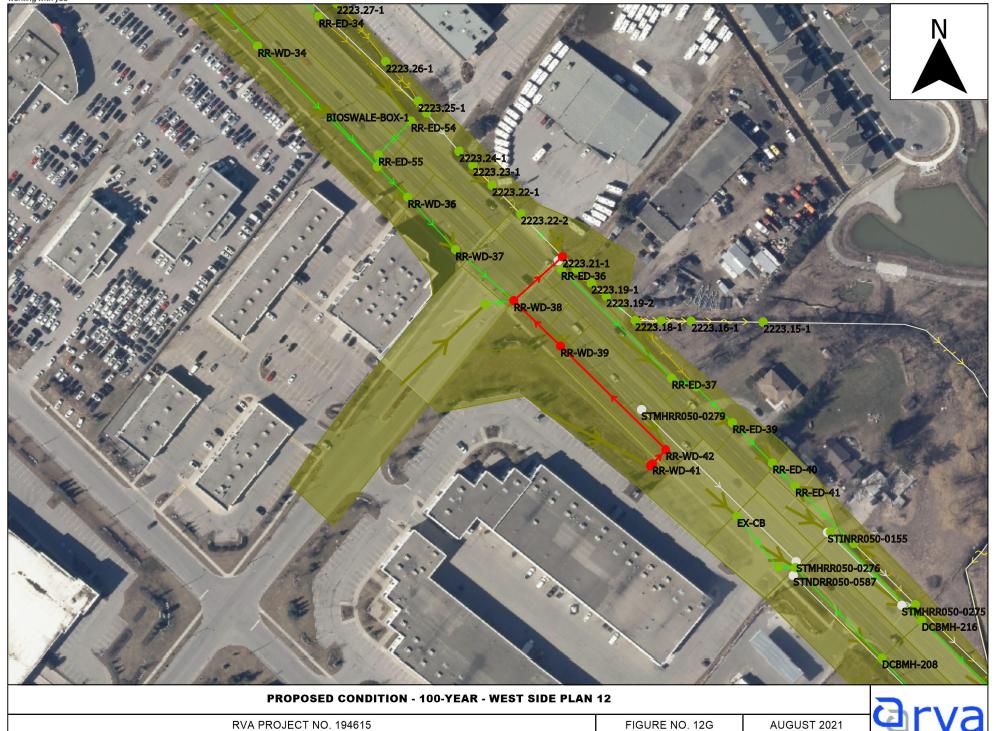




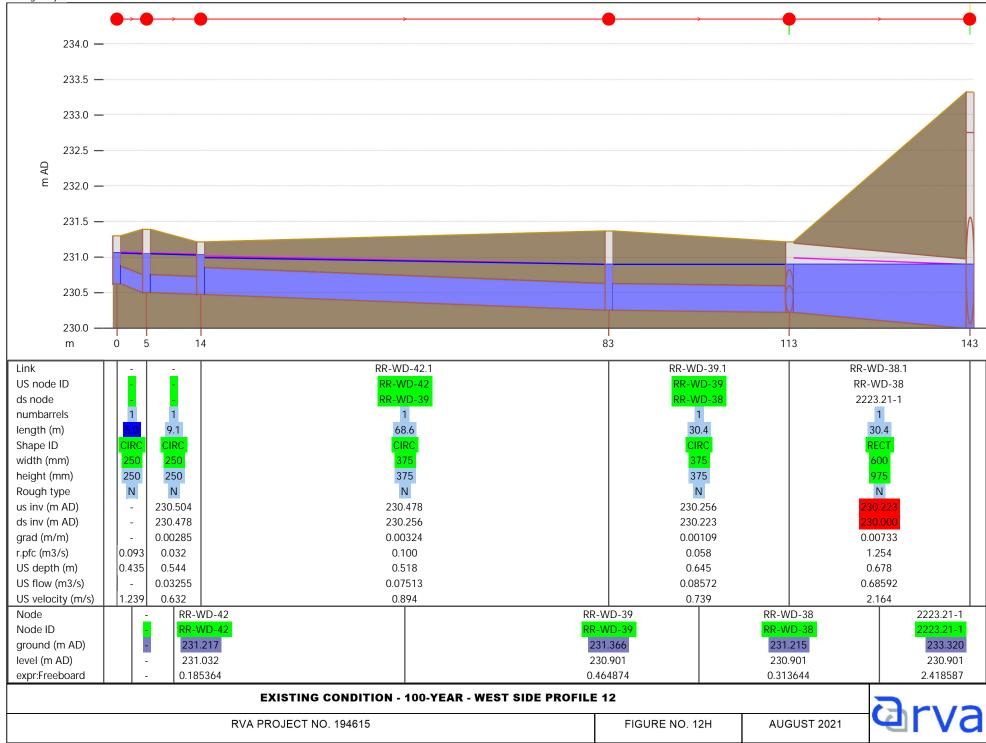














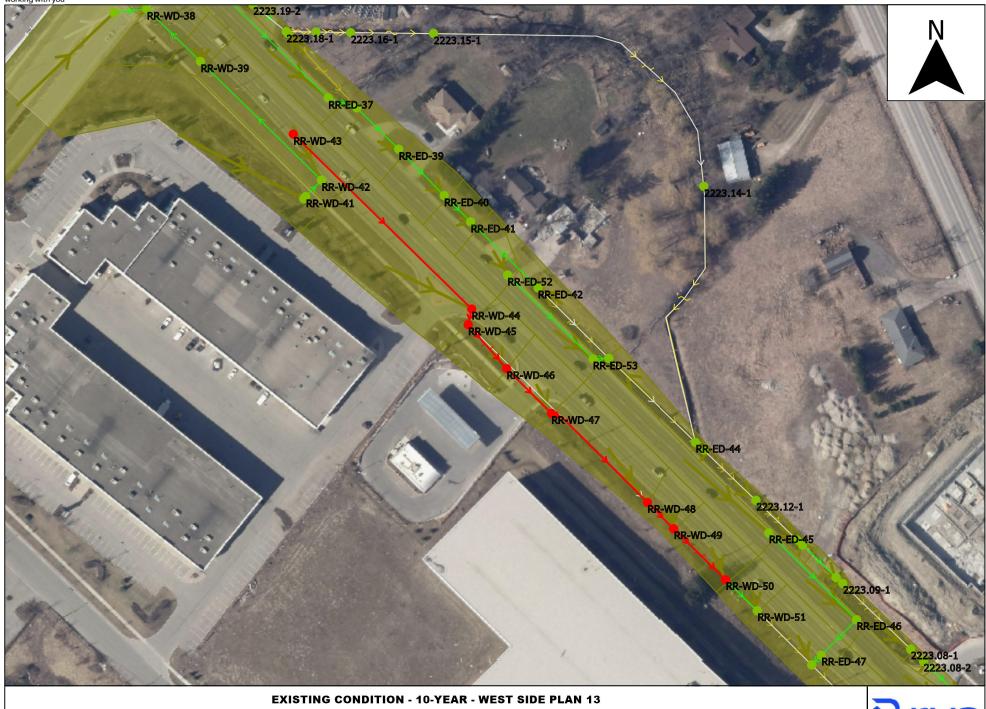
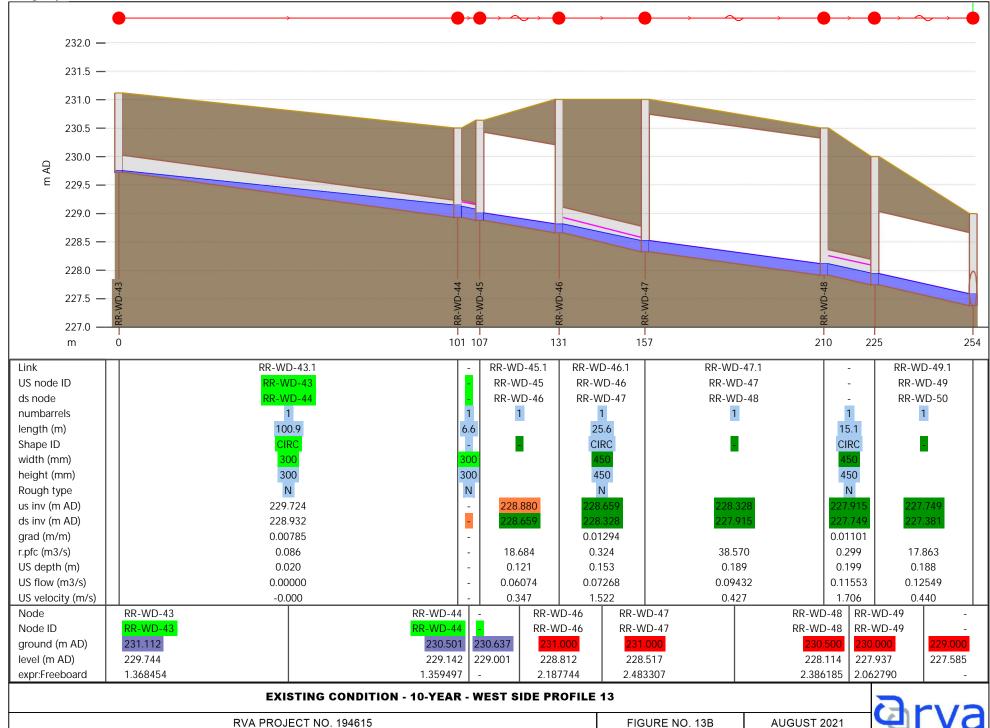
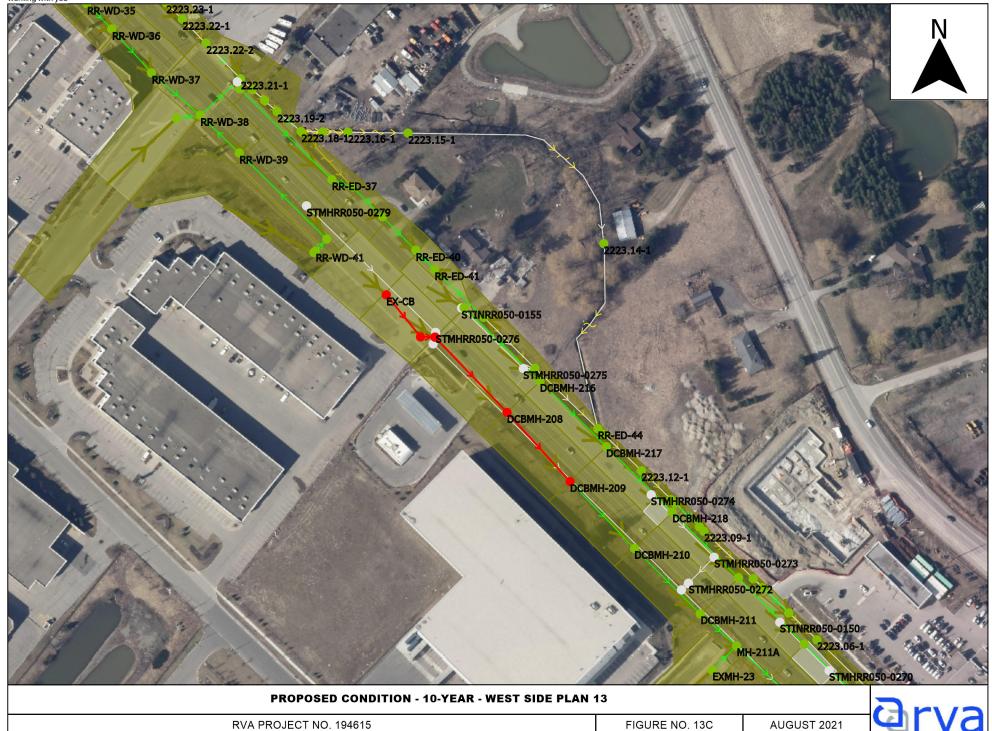


FIGURE NO. 13A

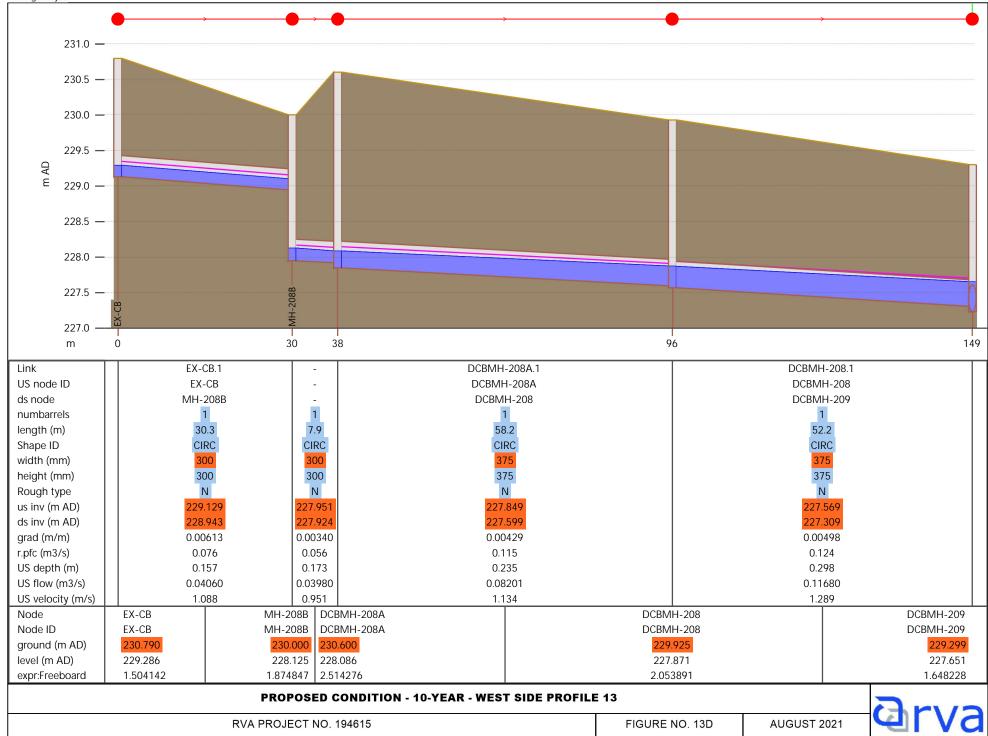




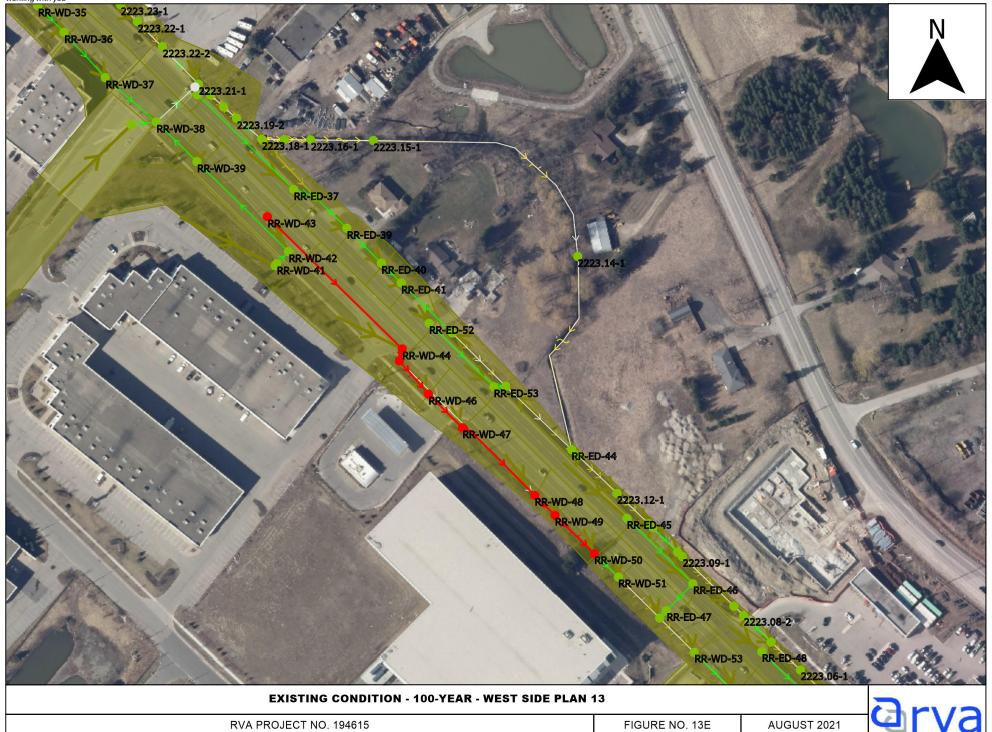




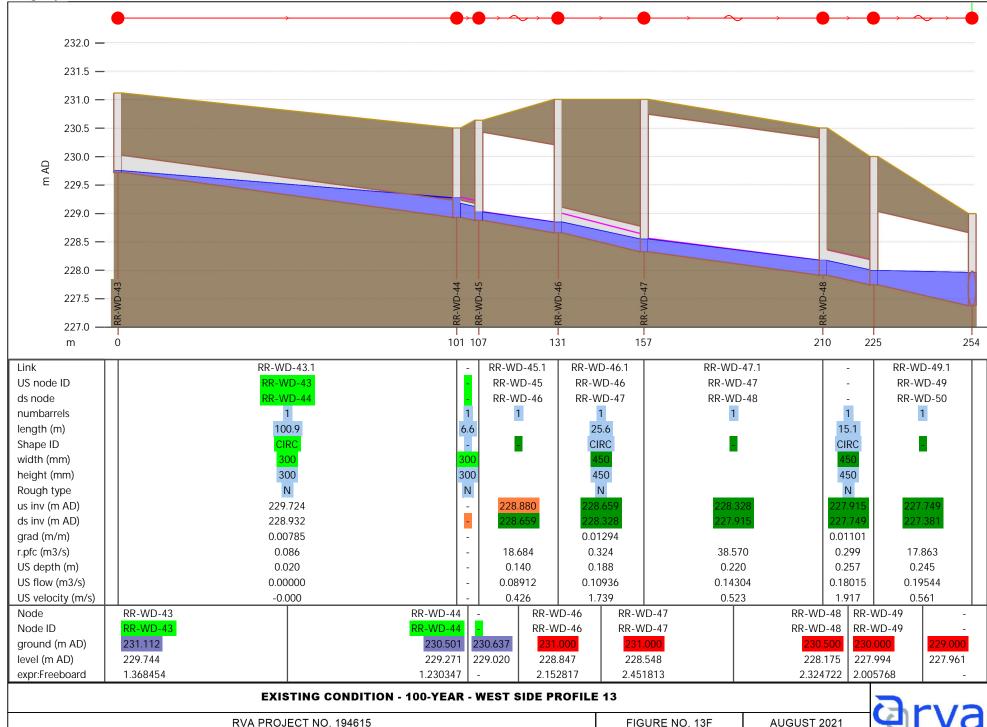














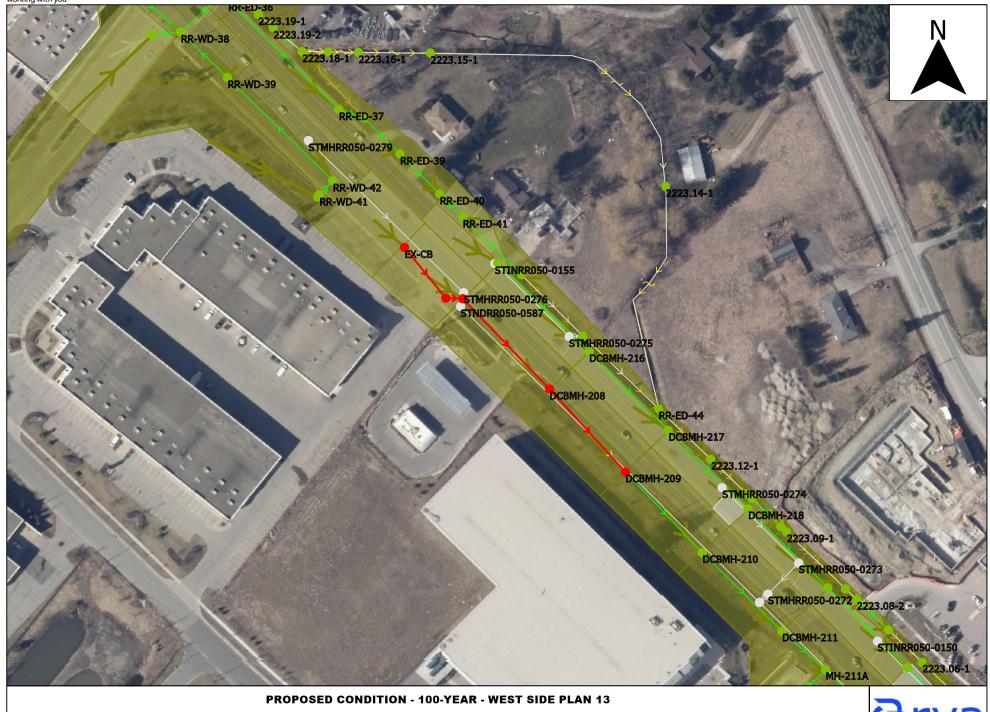
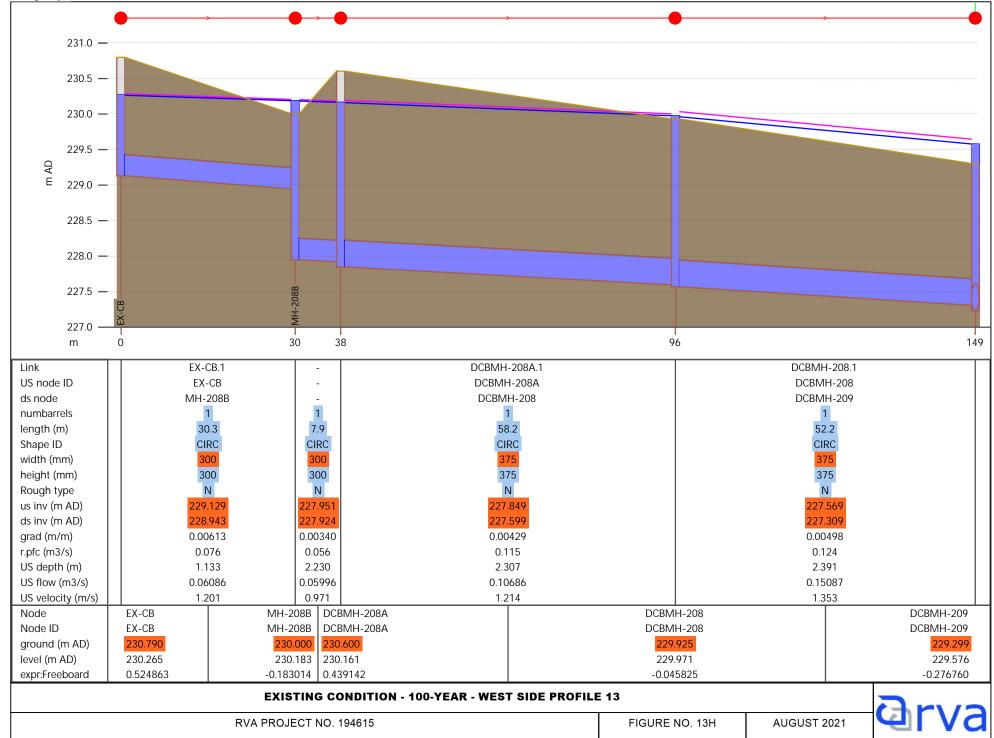
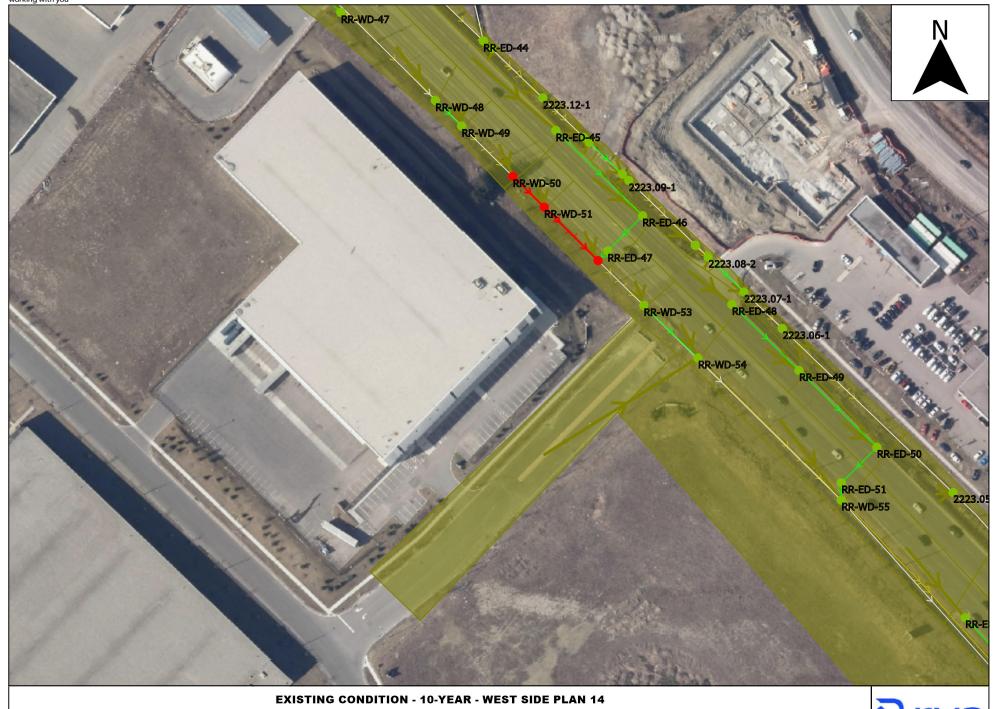


FIGURE NO. 13G





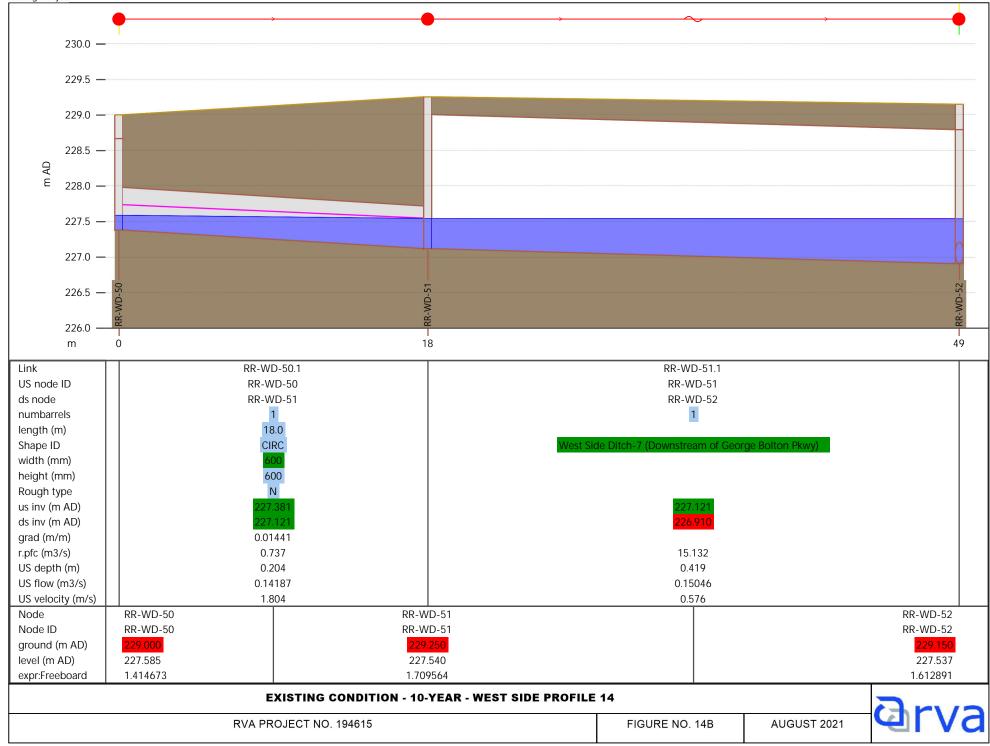




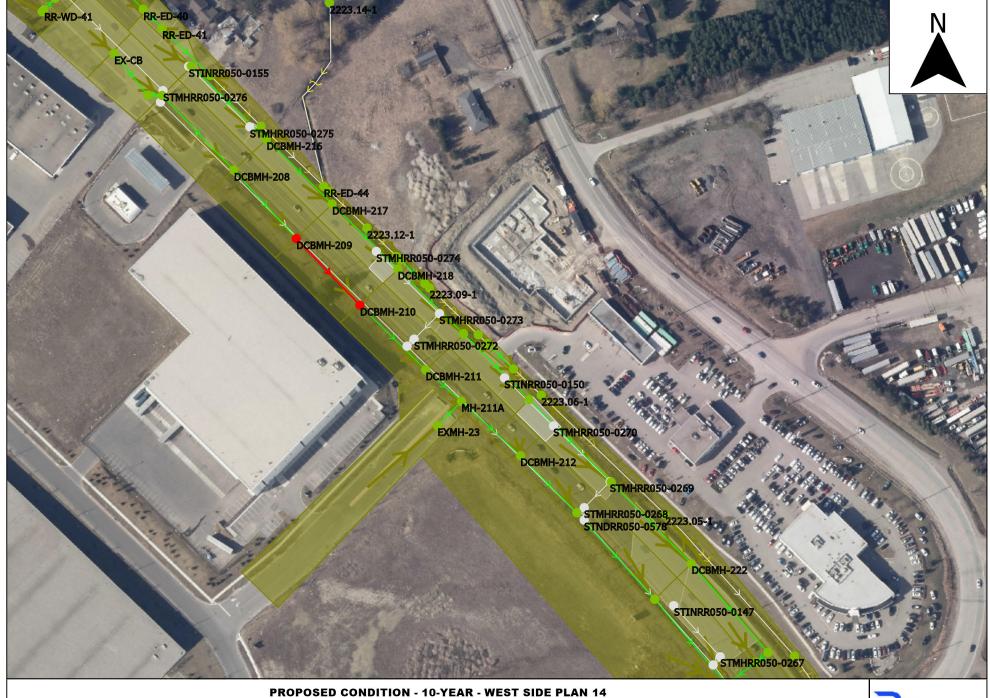
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FIGURE NO. 14A

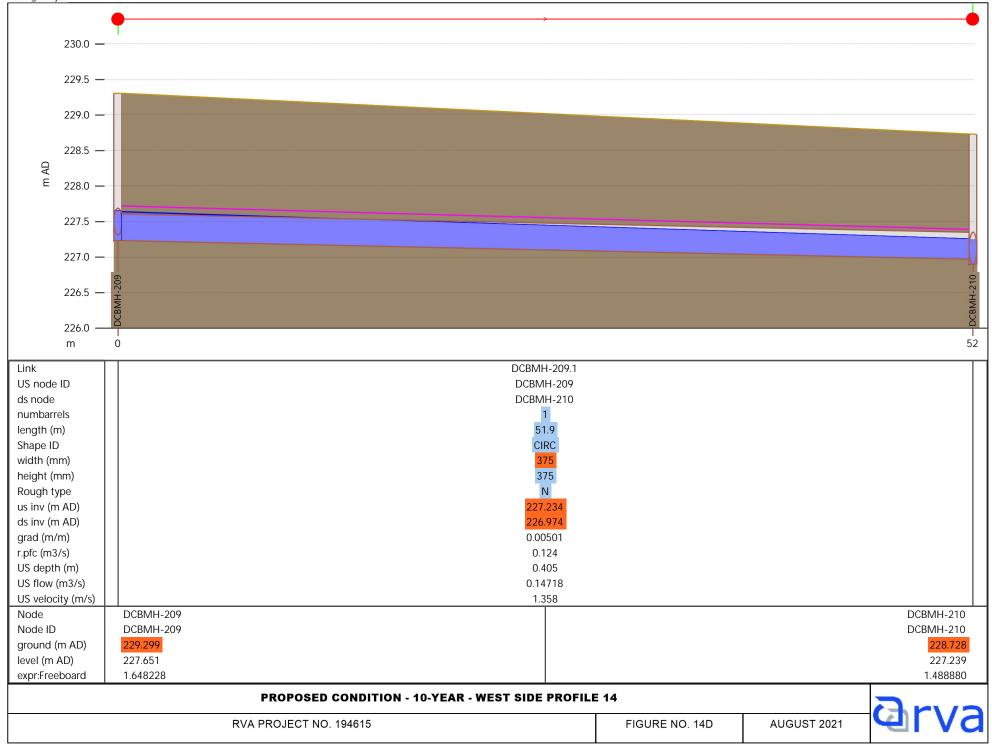














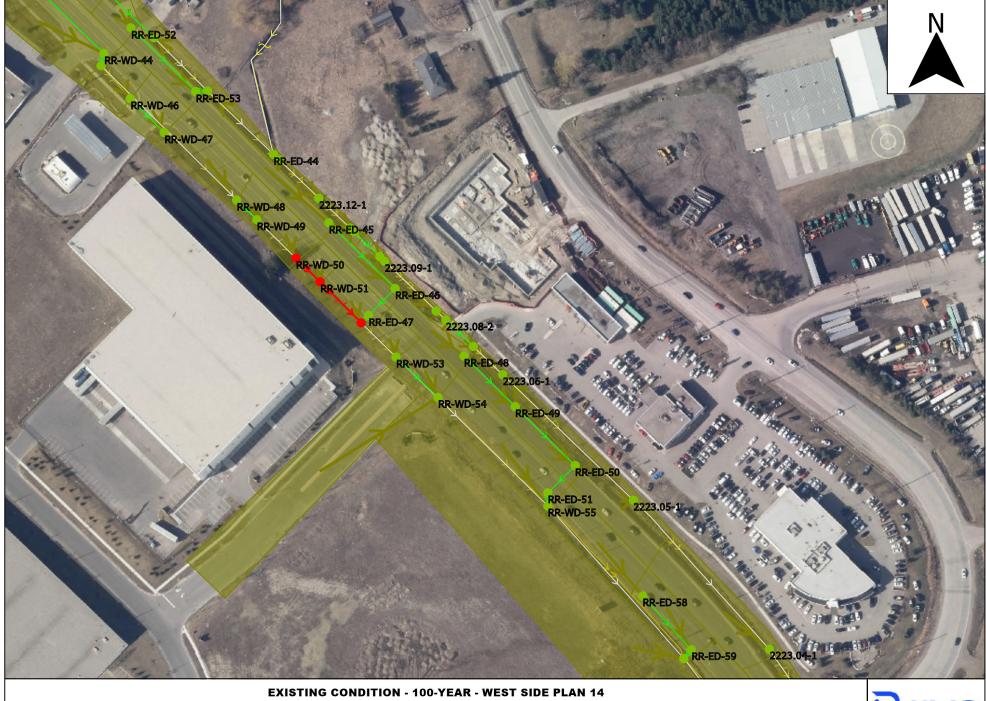
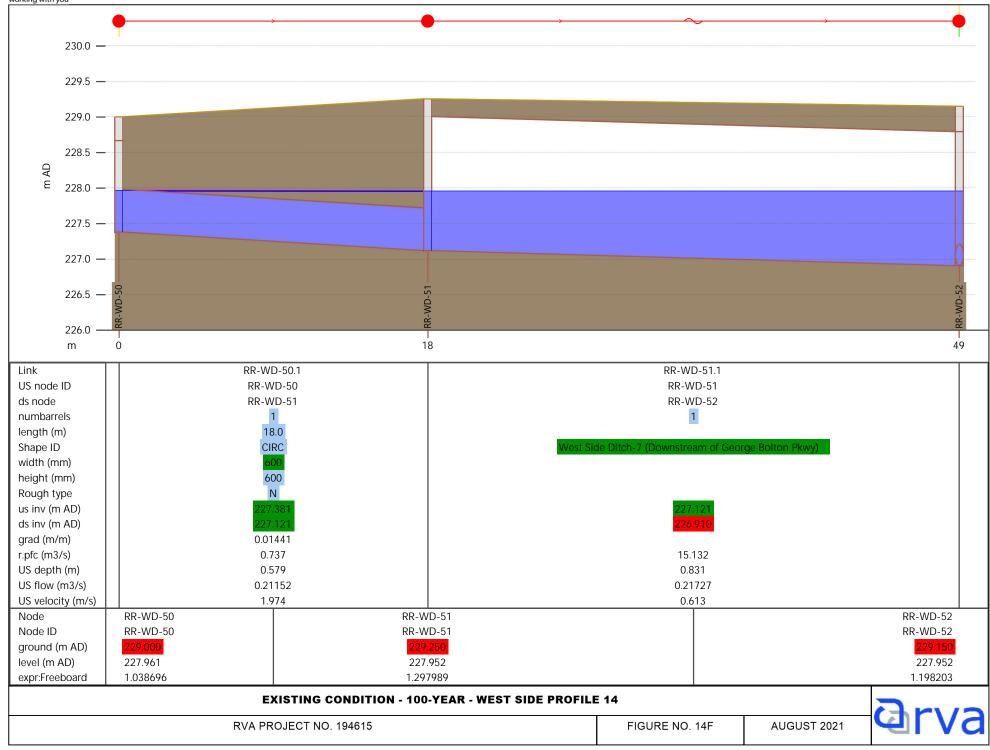


FIGURE NO. 14E AUGUST 2021







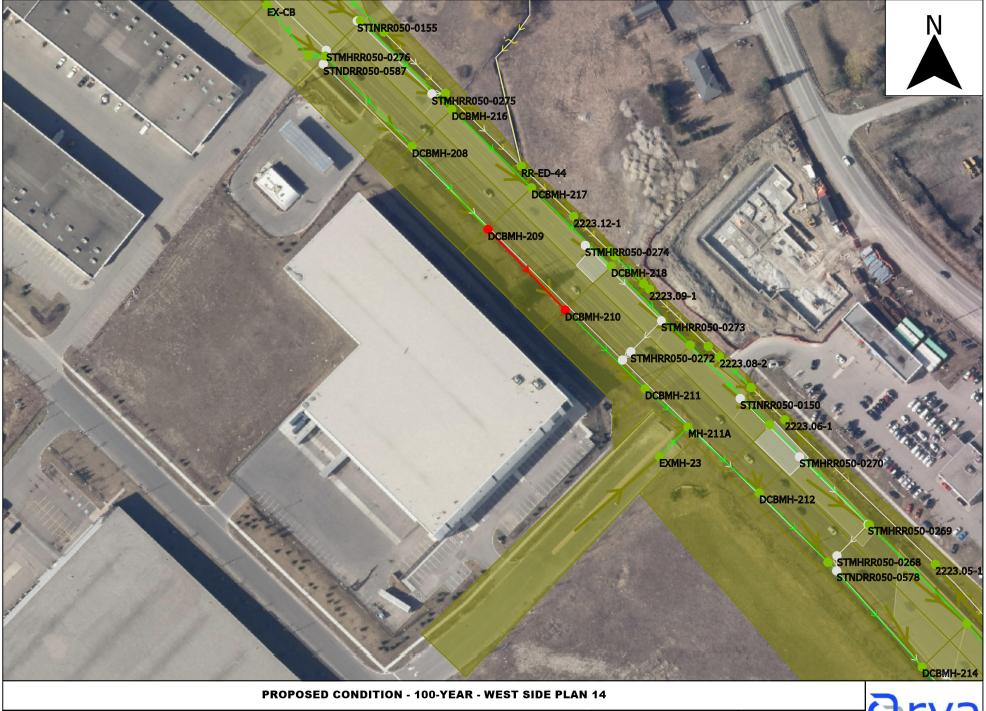
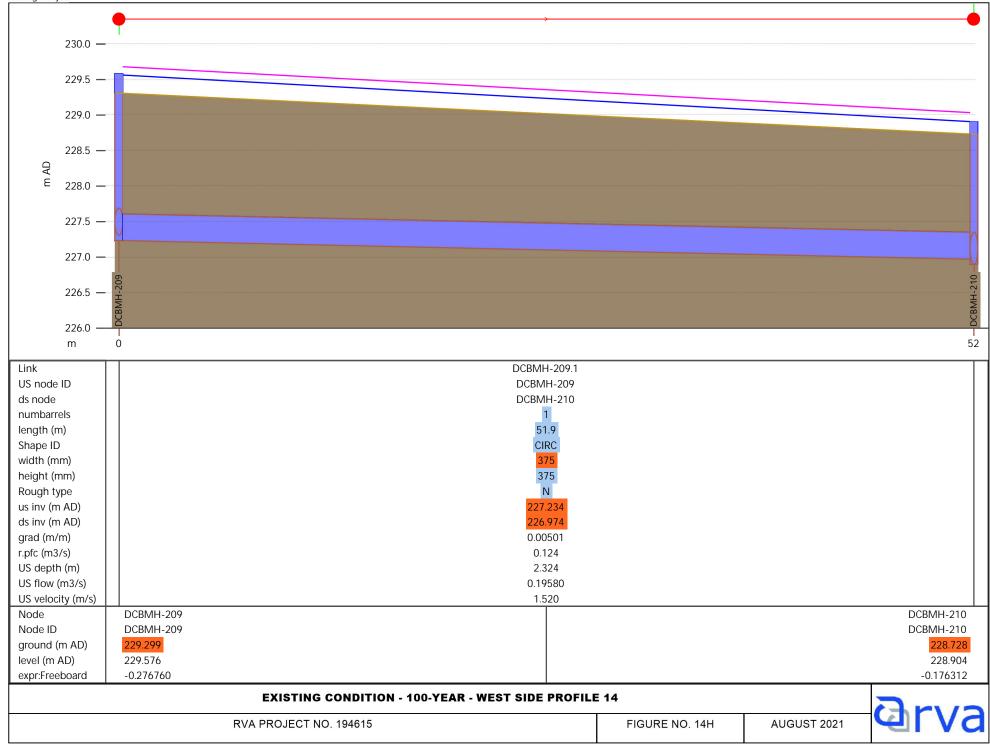
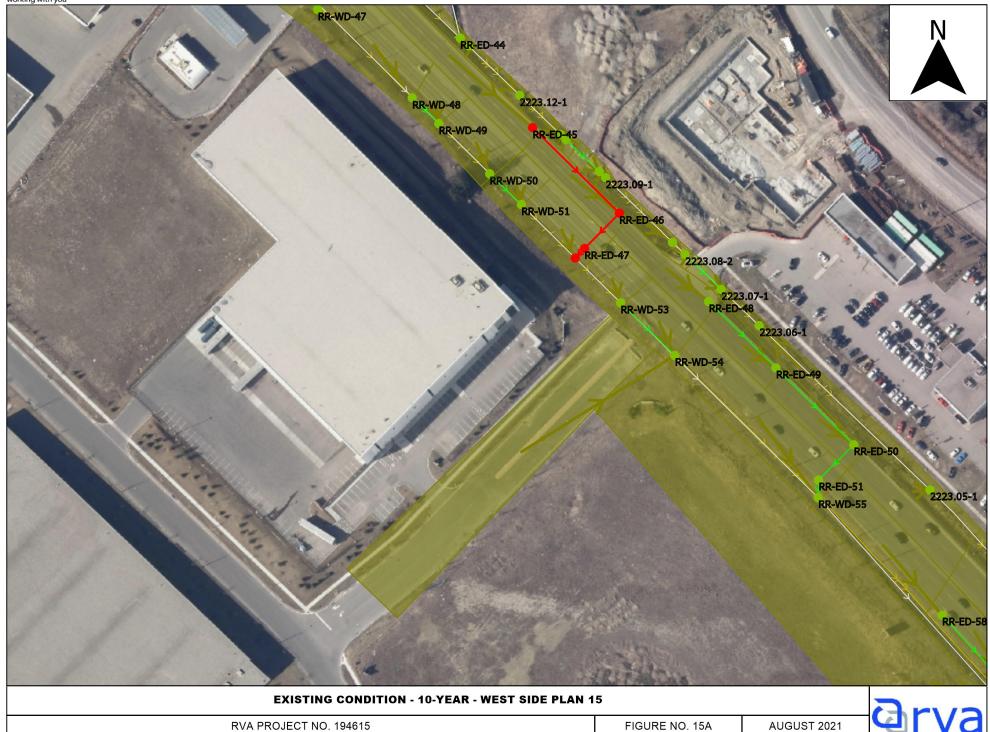


FIGURE NO. 14G

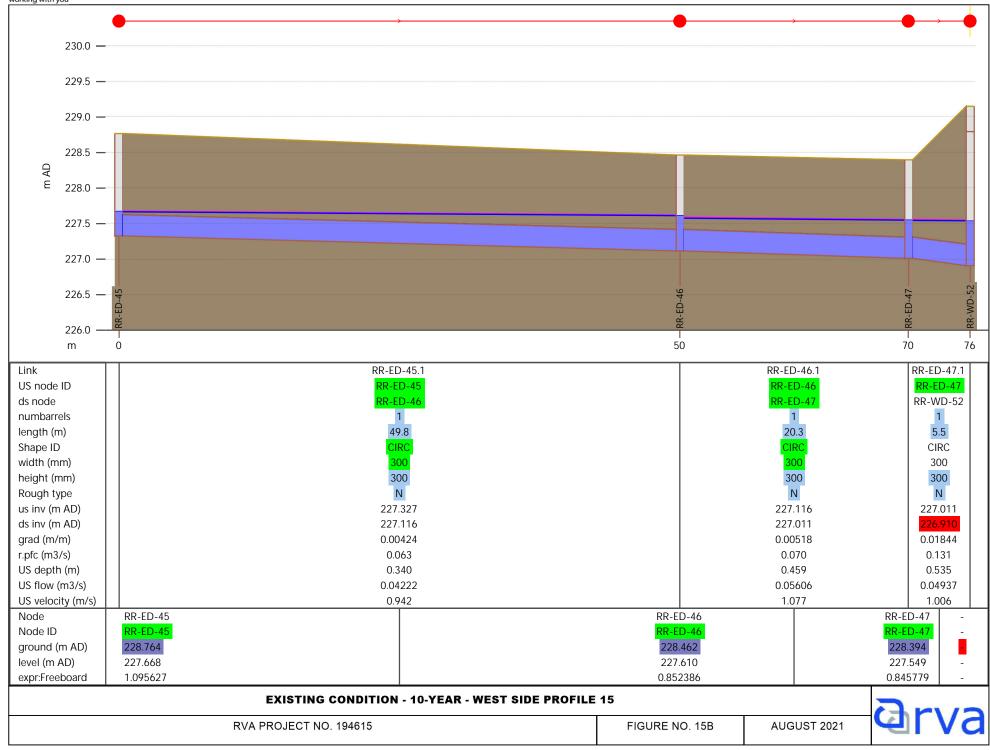














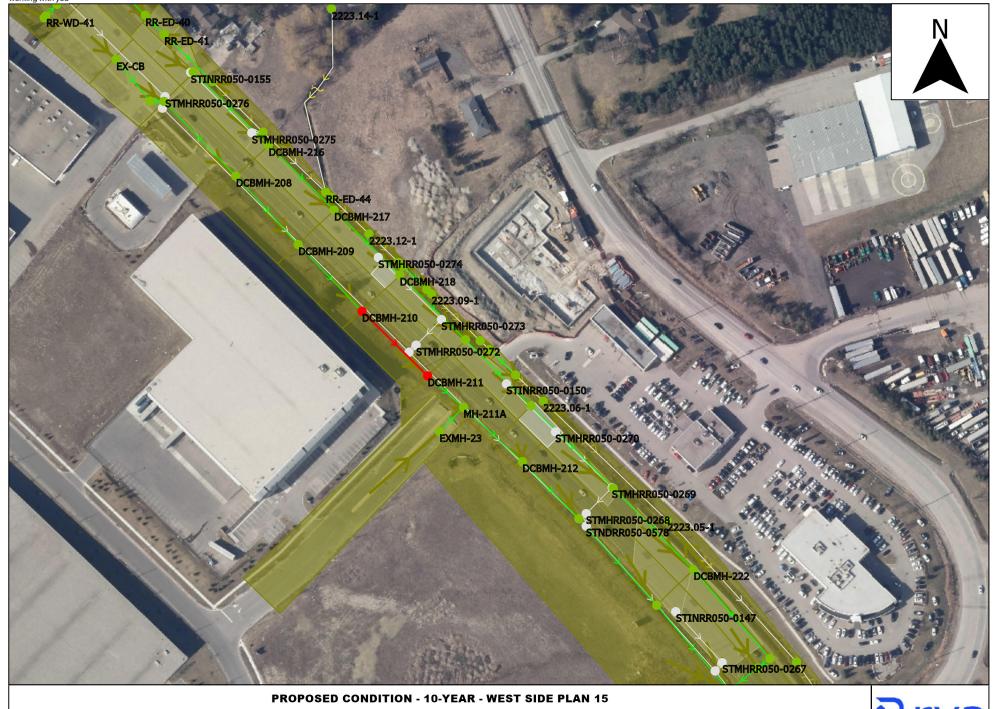
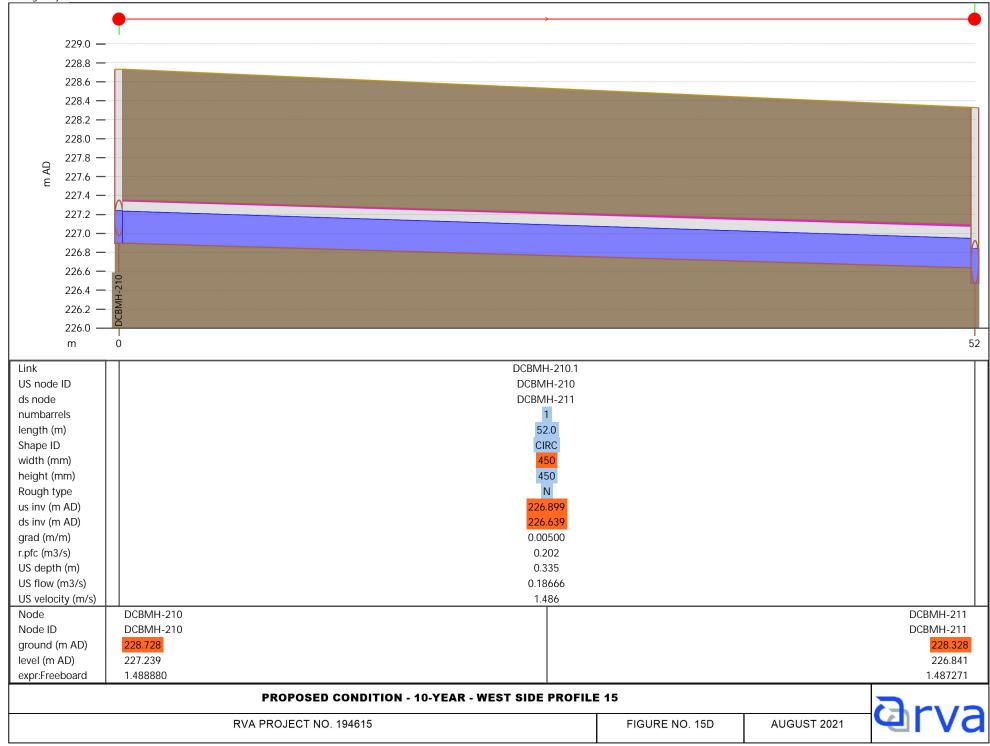


FIGURE NO. 15C AUGUST 2021







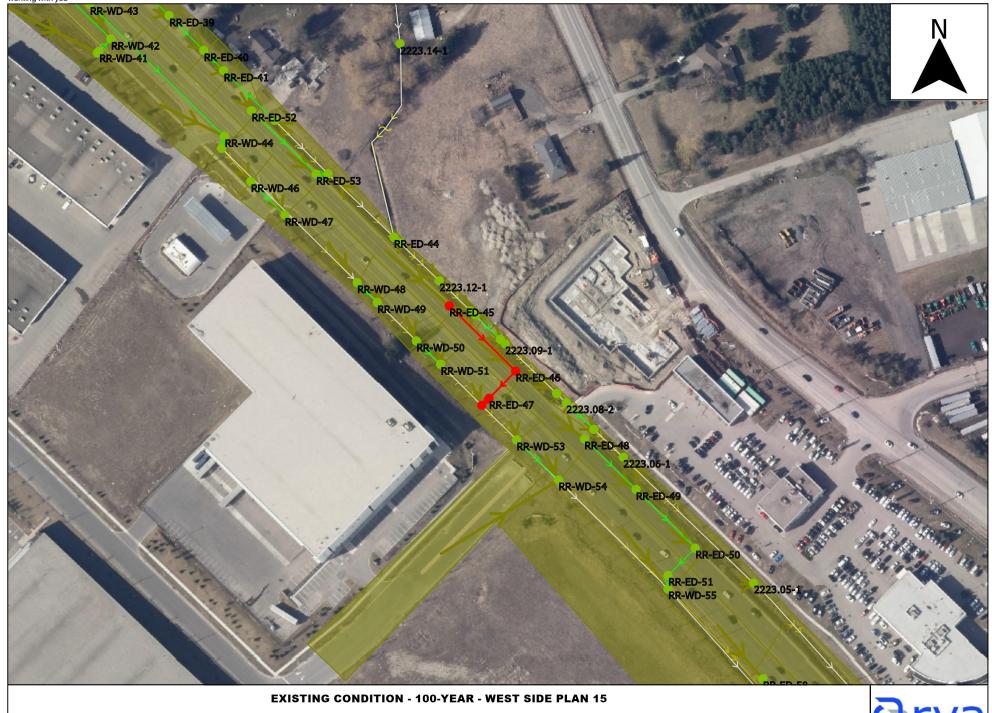
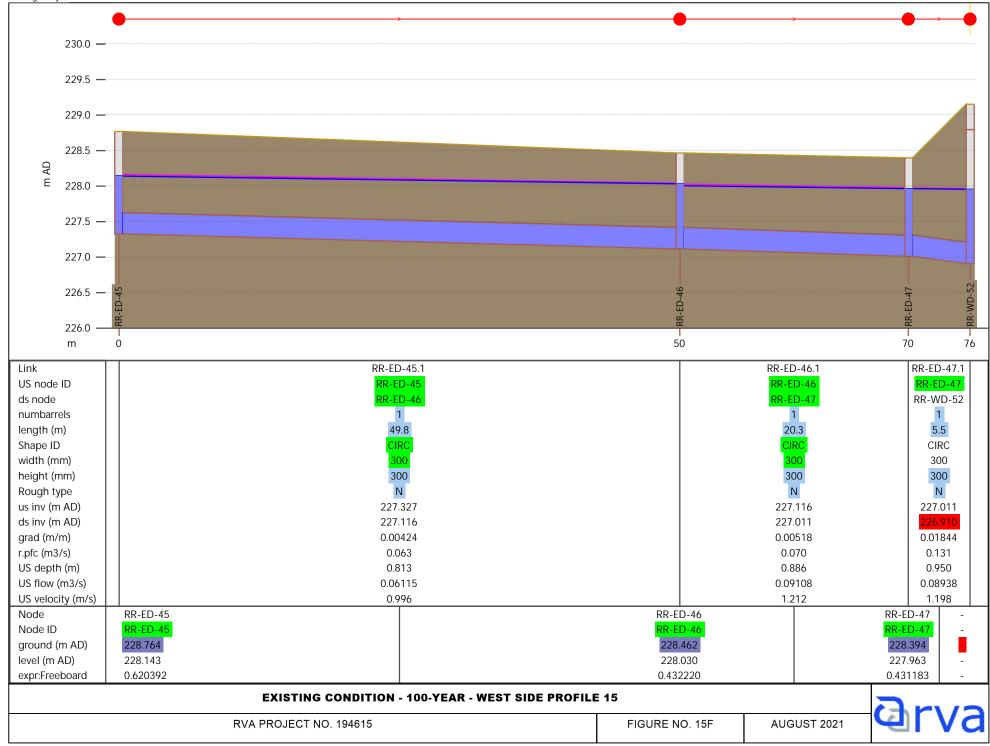


FIGURE NO. 15E







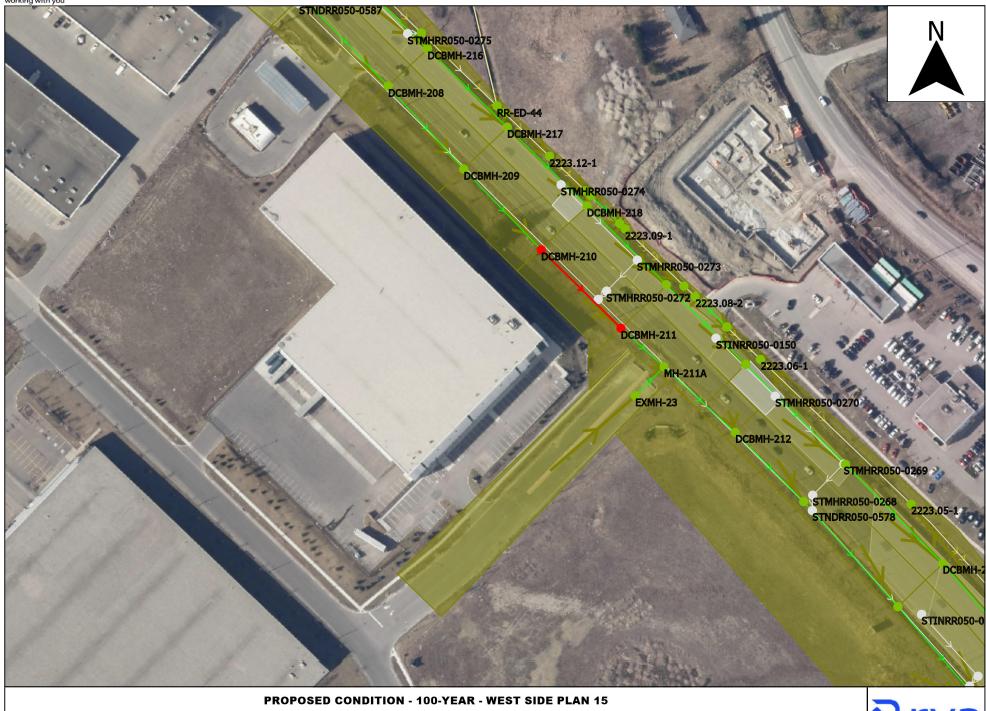
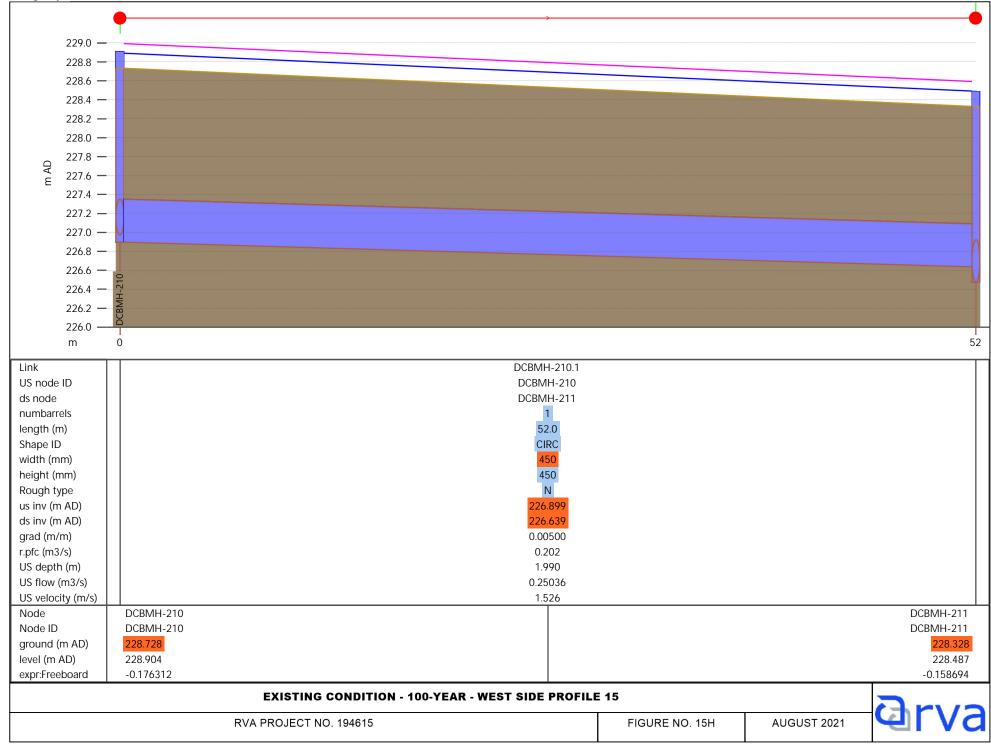


FIGURE NO. 15G







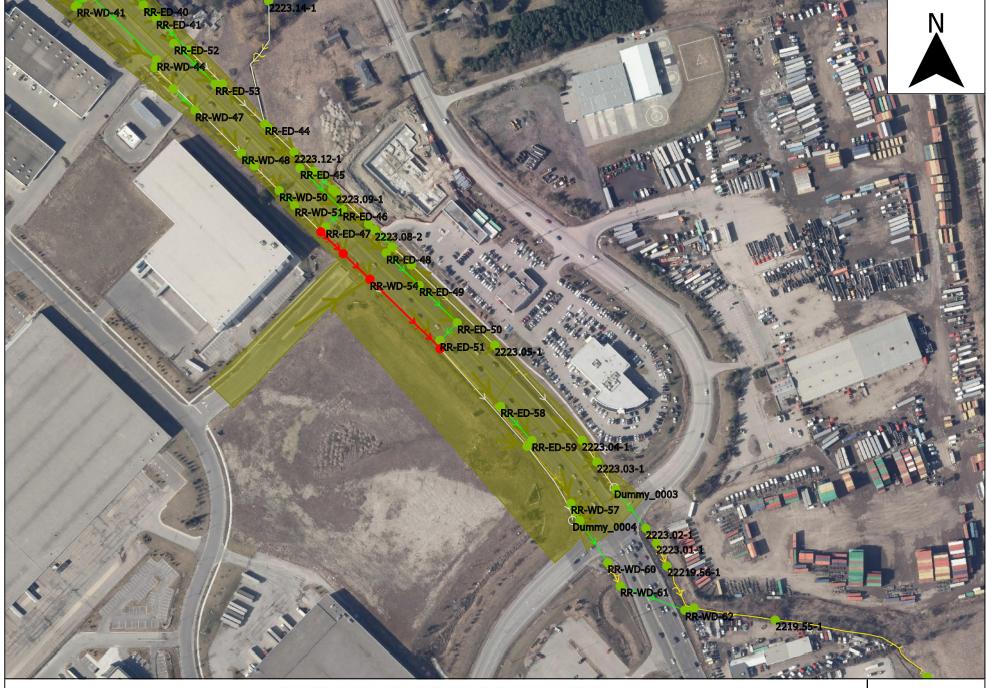
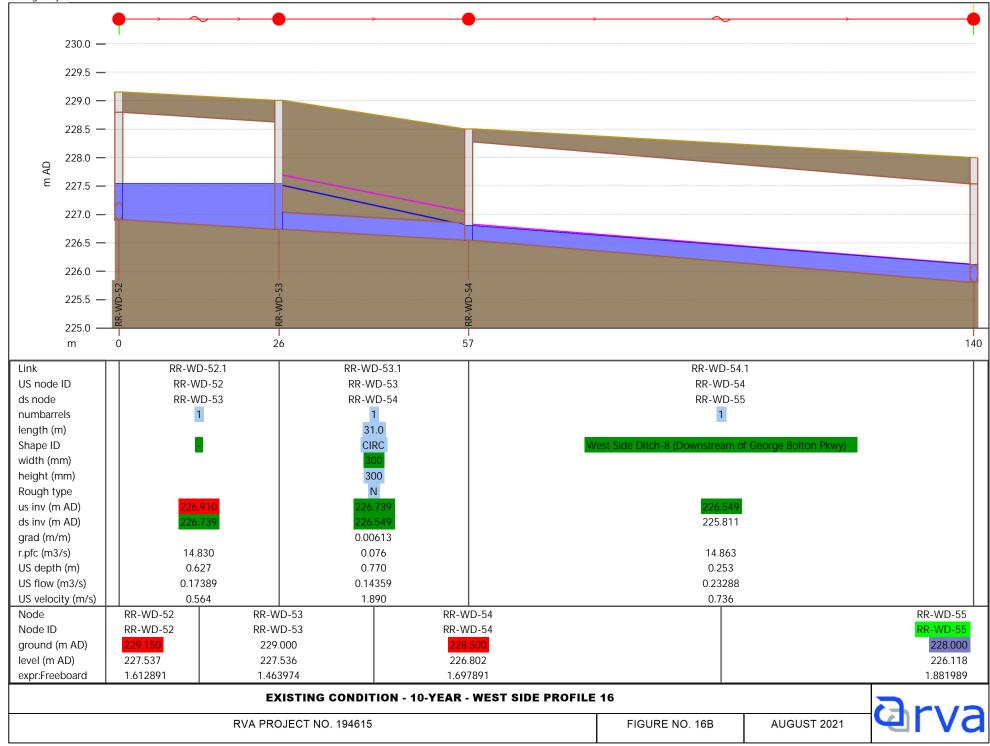




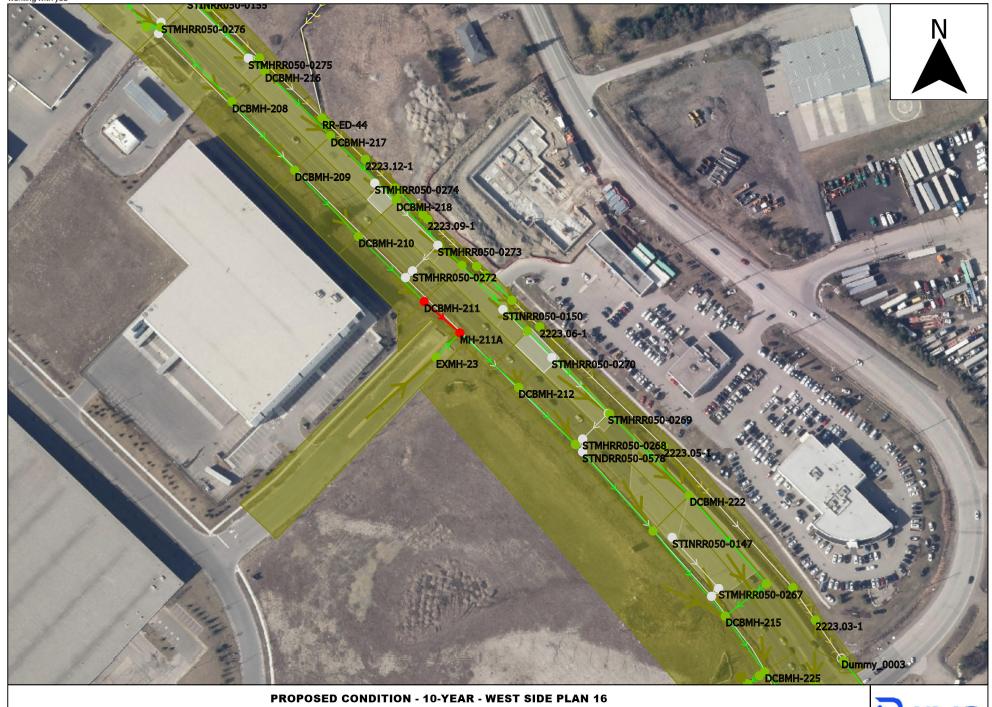
FIGURE NO. 16A







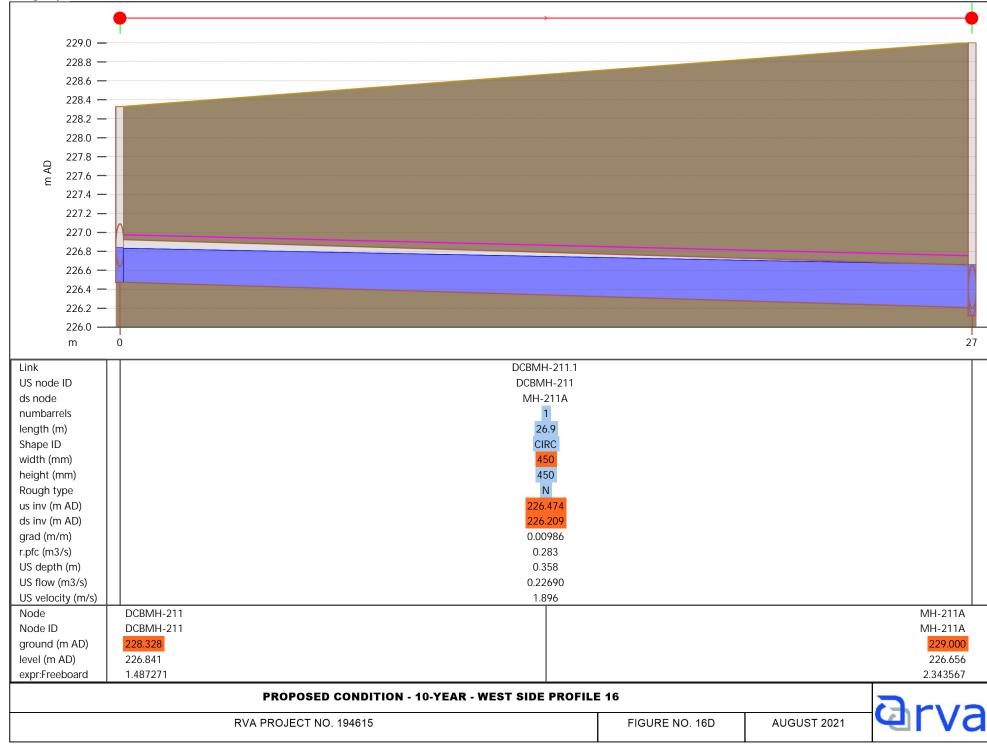




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FIGURE NO. 16C







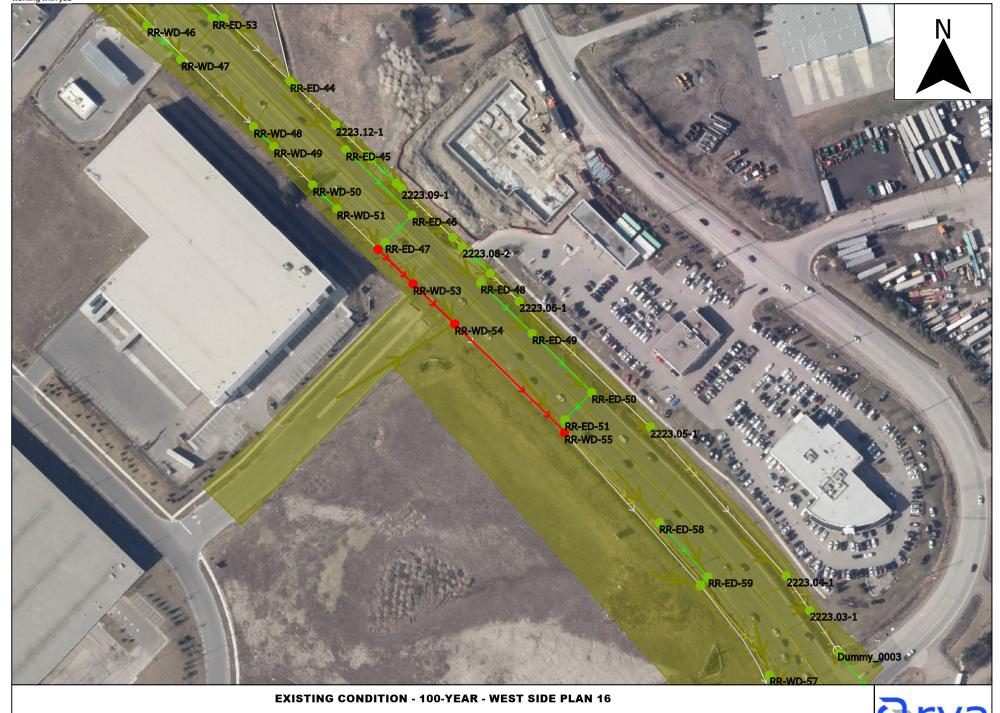
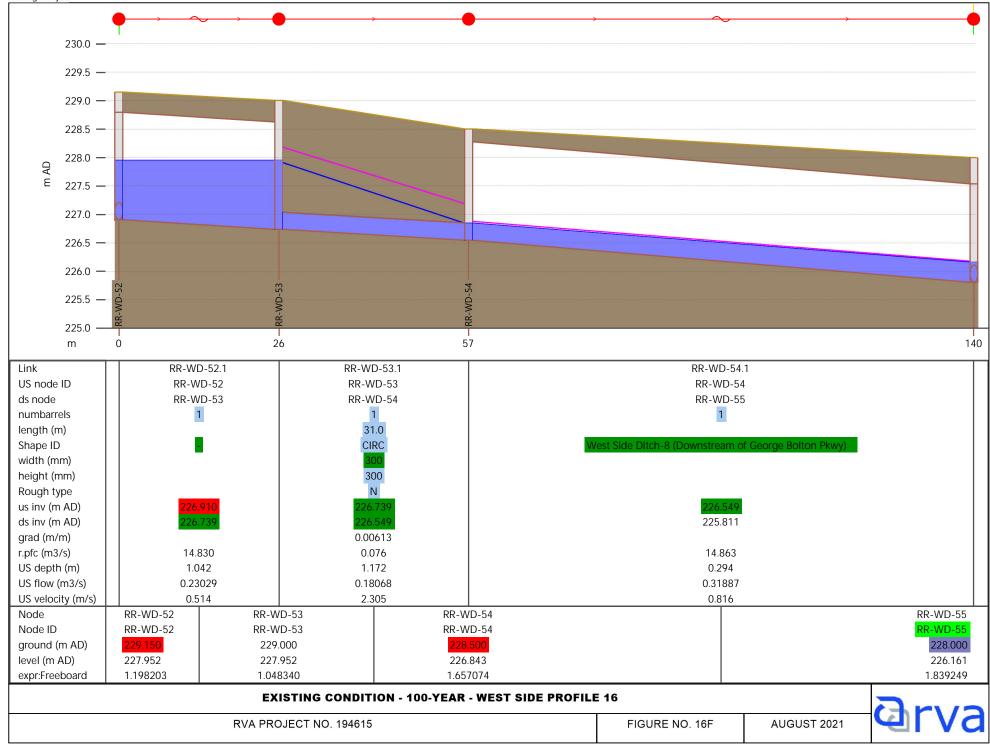
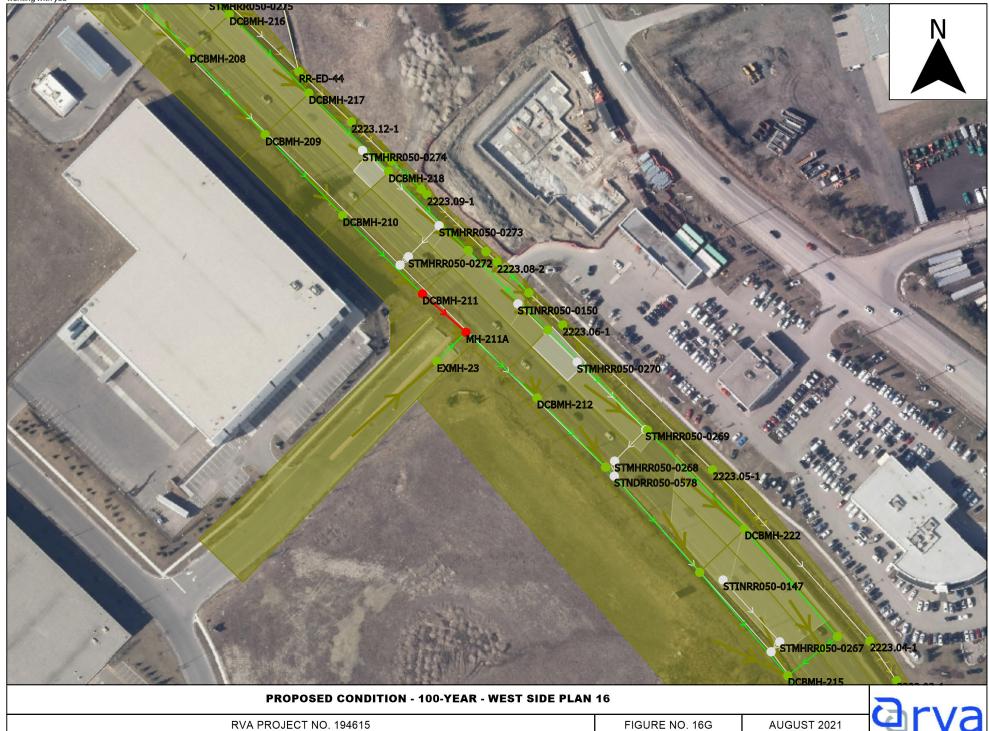


FIGURE NO. 16E











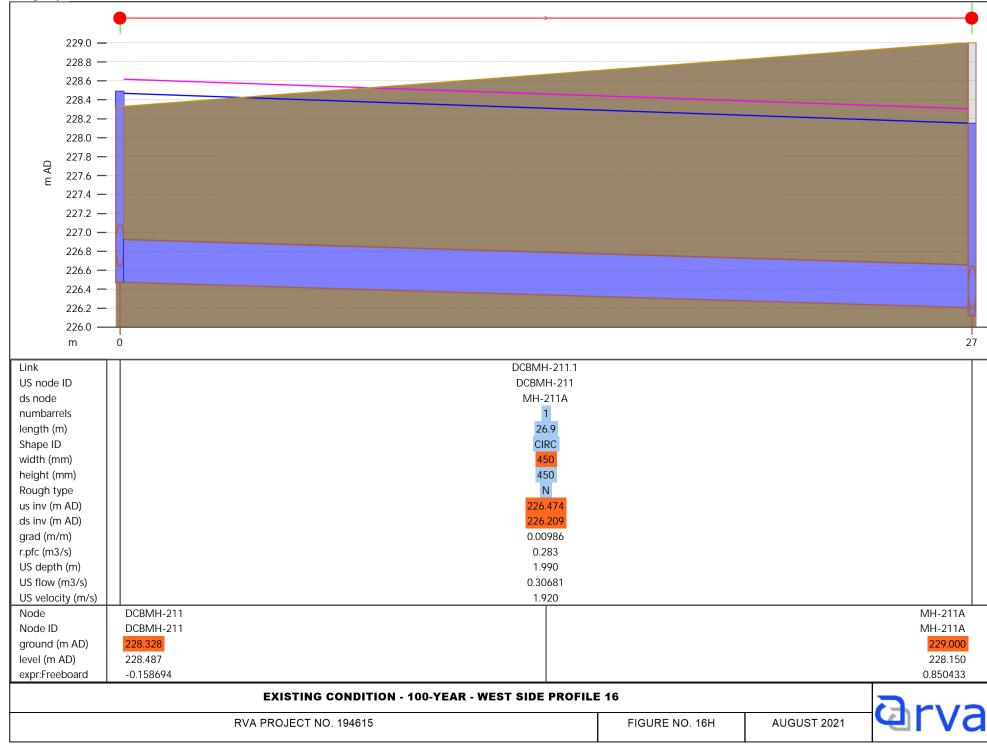




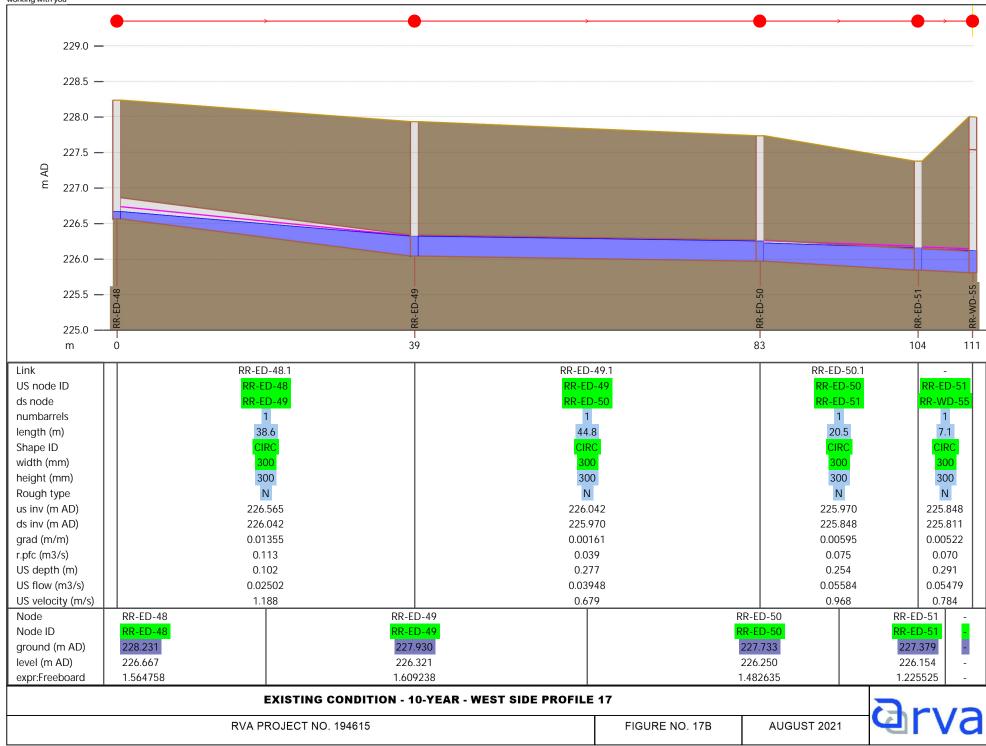




FIGURE NO. 17A









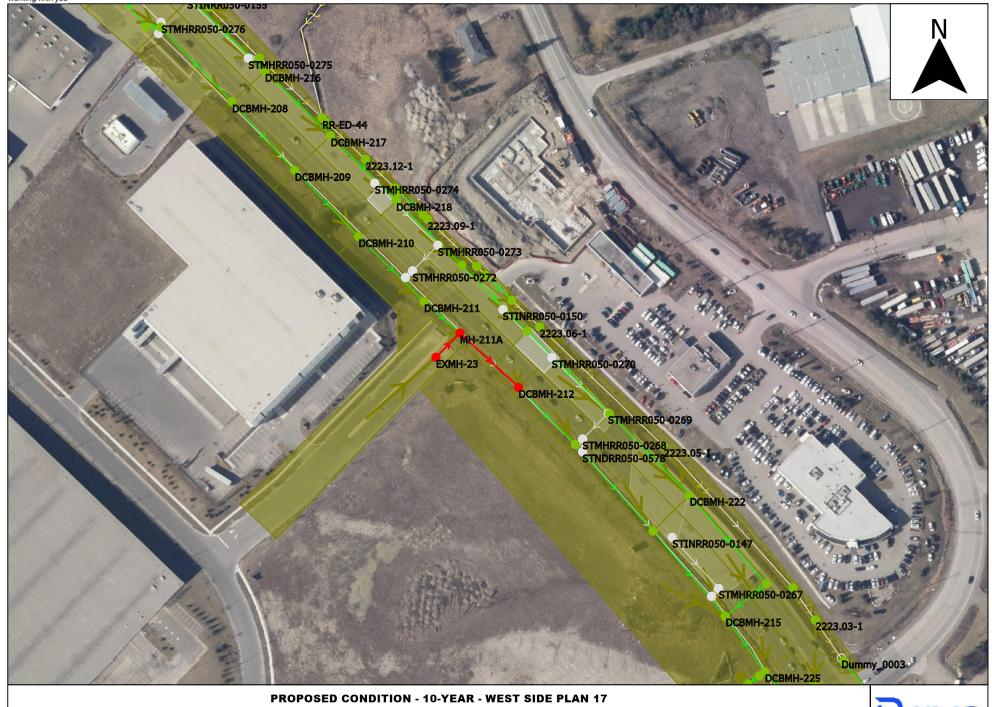


FIGURE NO. 17C



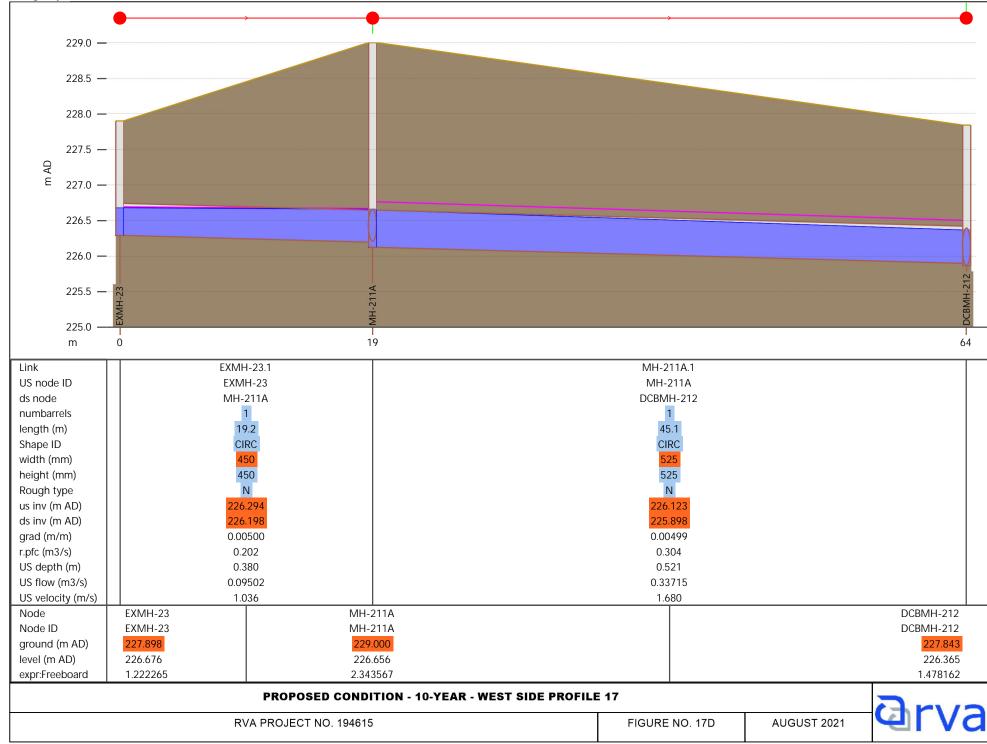
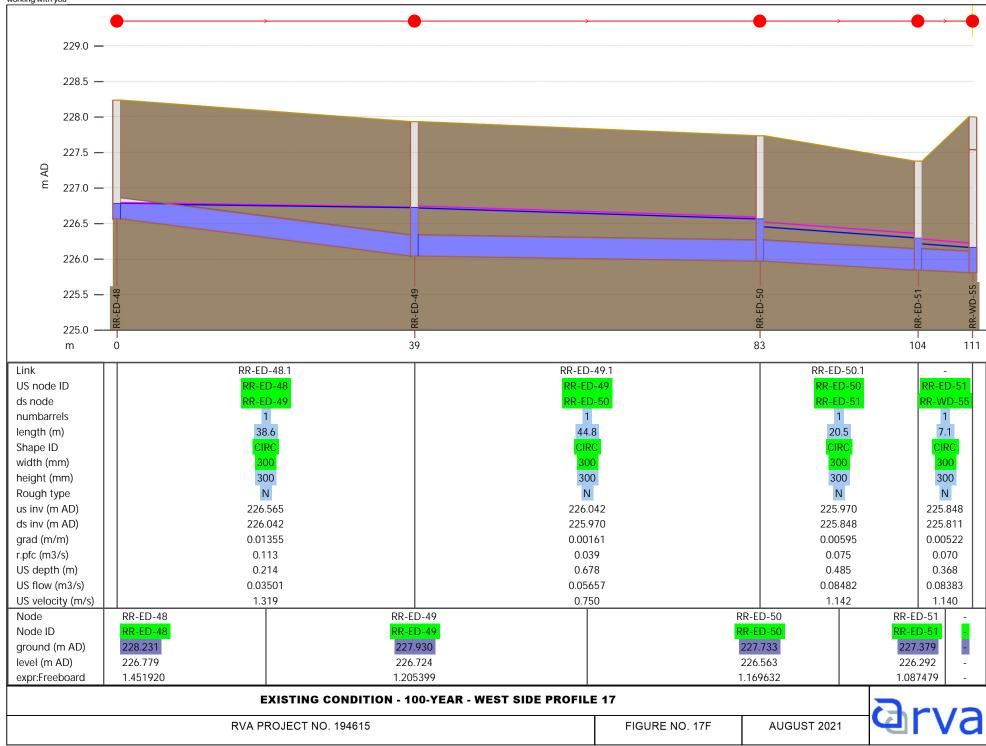




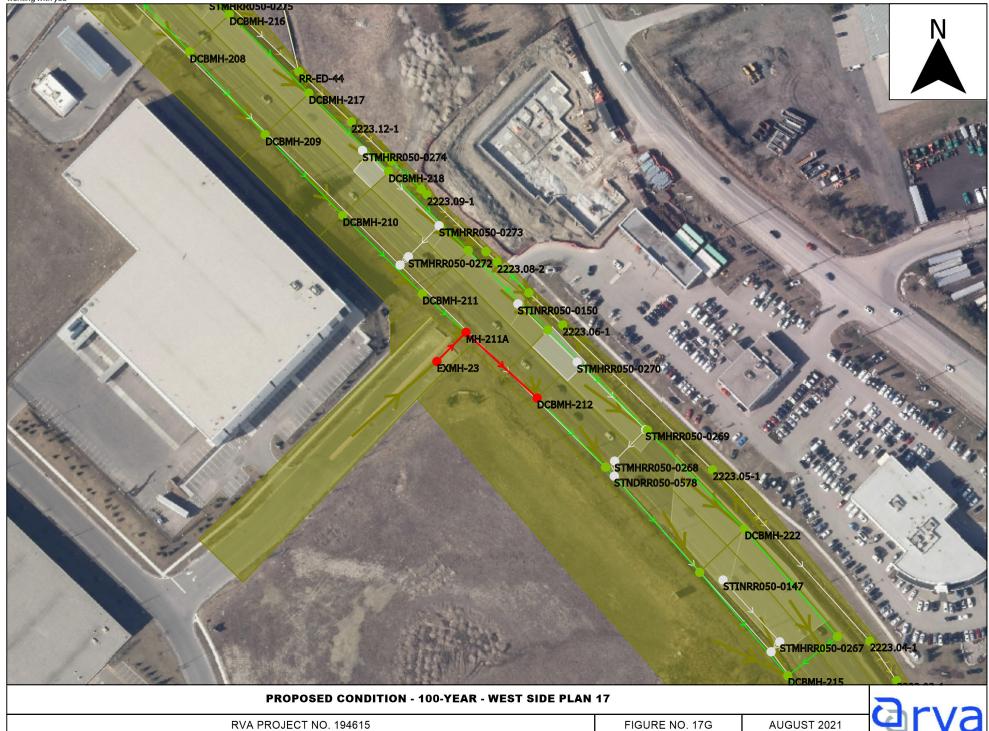


FIGURE NO. 17E AUGUST 2021











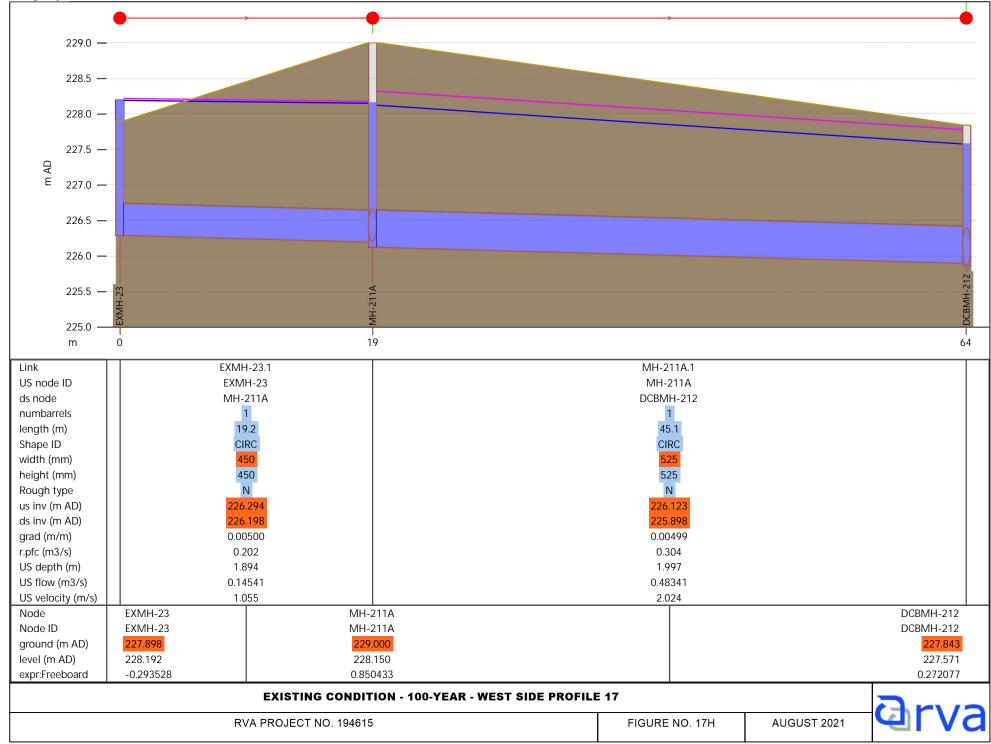




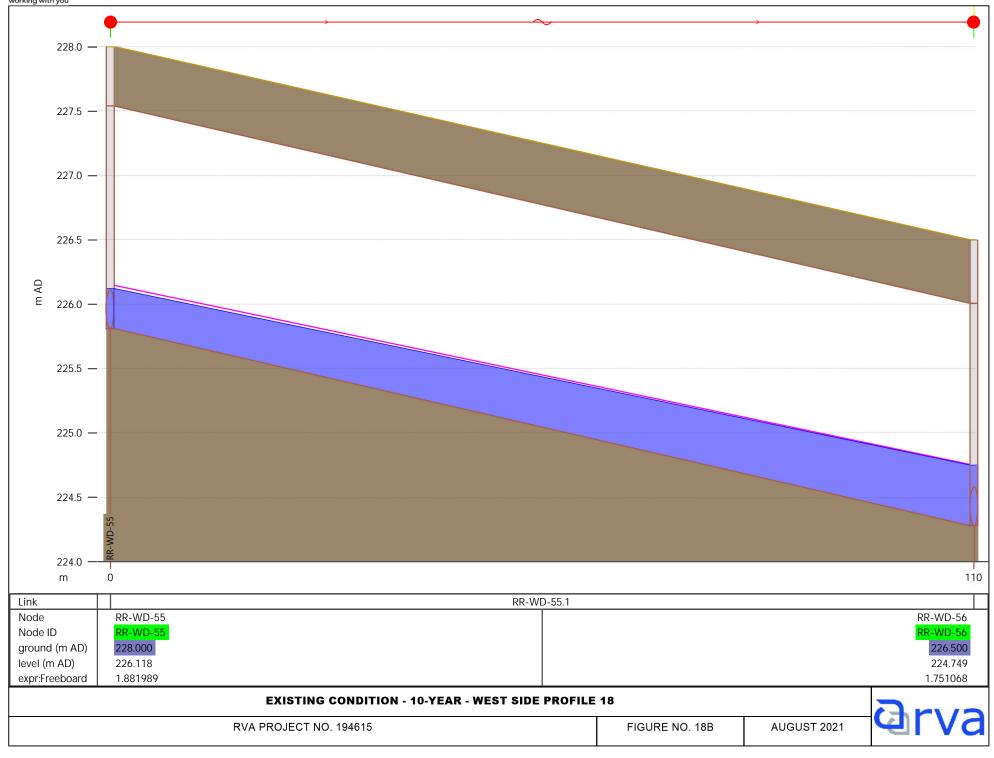




FIGURE NO. 18A











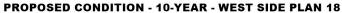
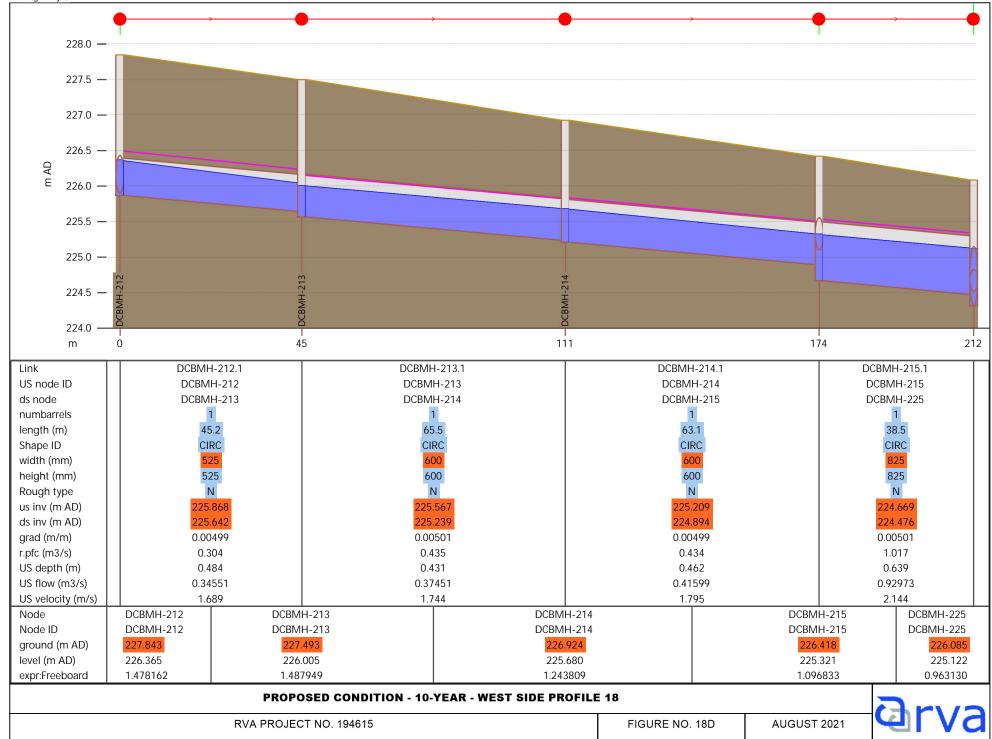


FIGURE NO. 18C







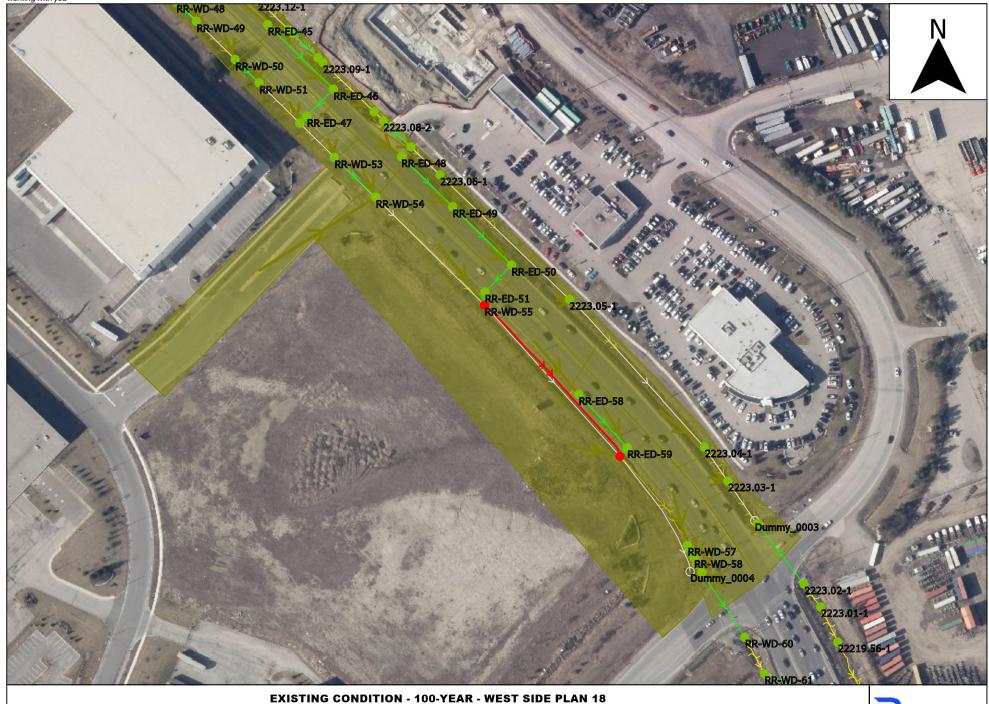


FIGURE NO. 18E



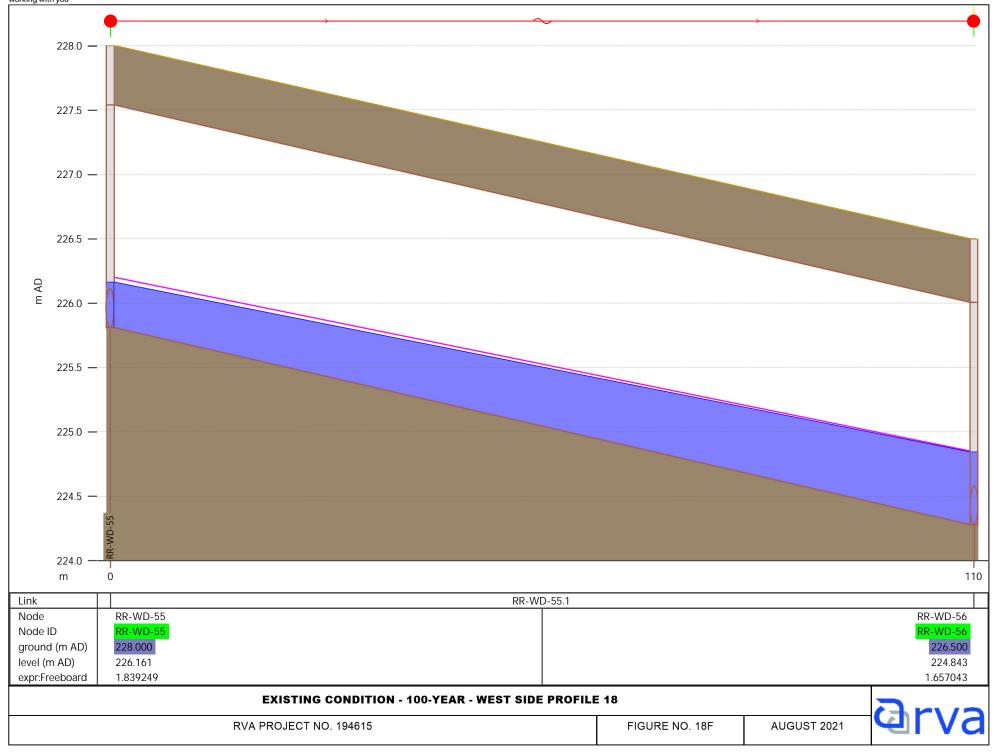


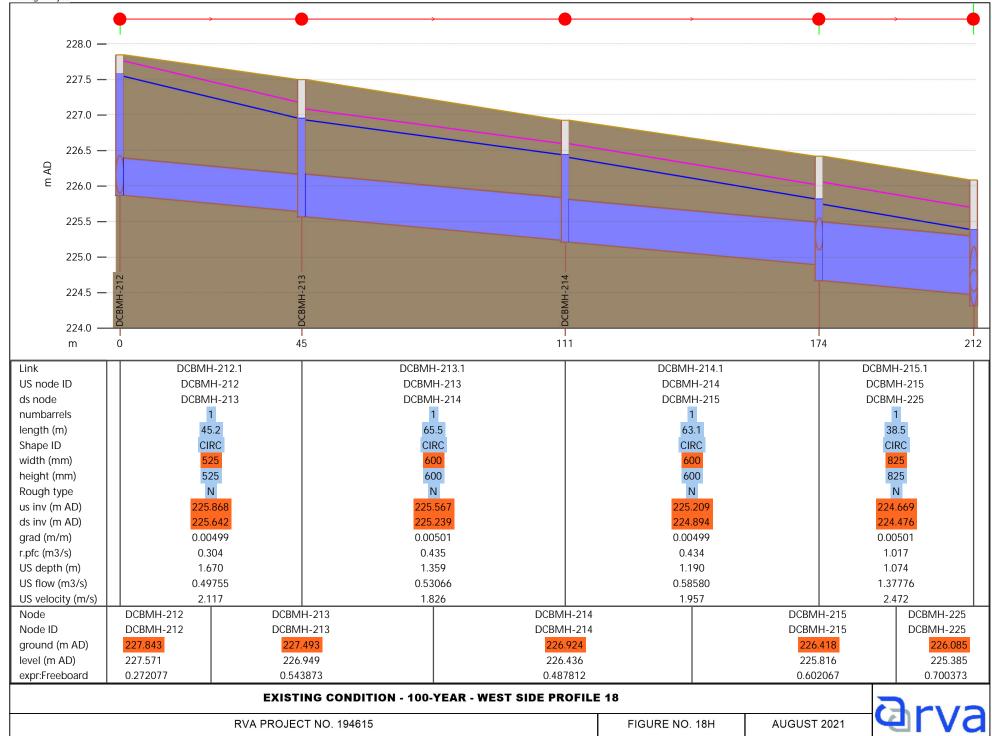




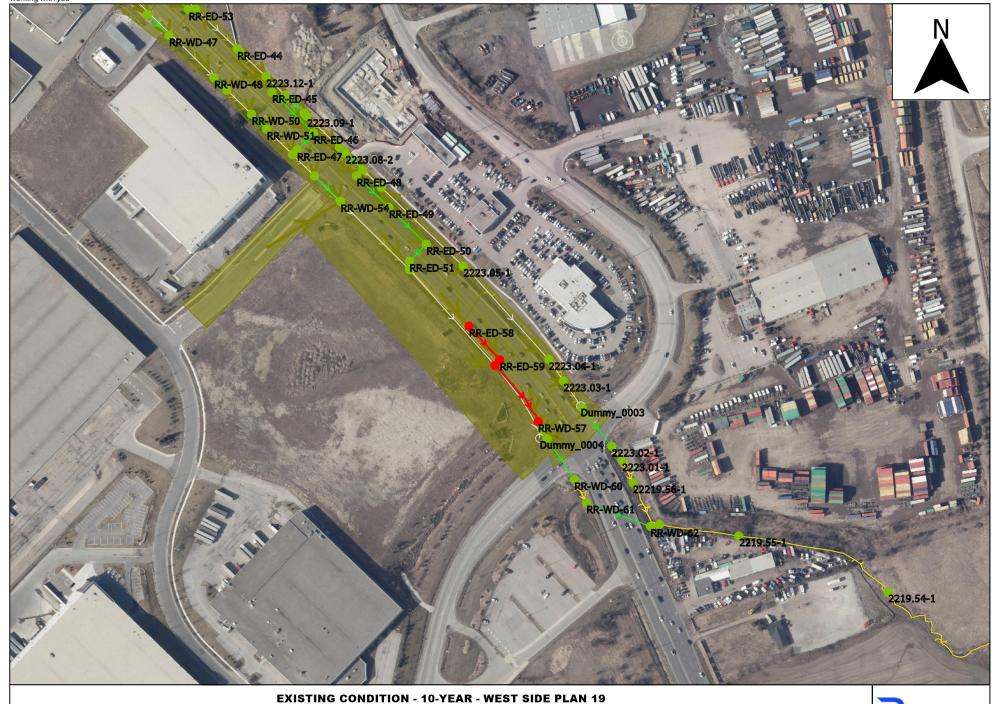


FIGURE NO. 18G



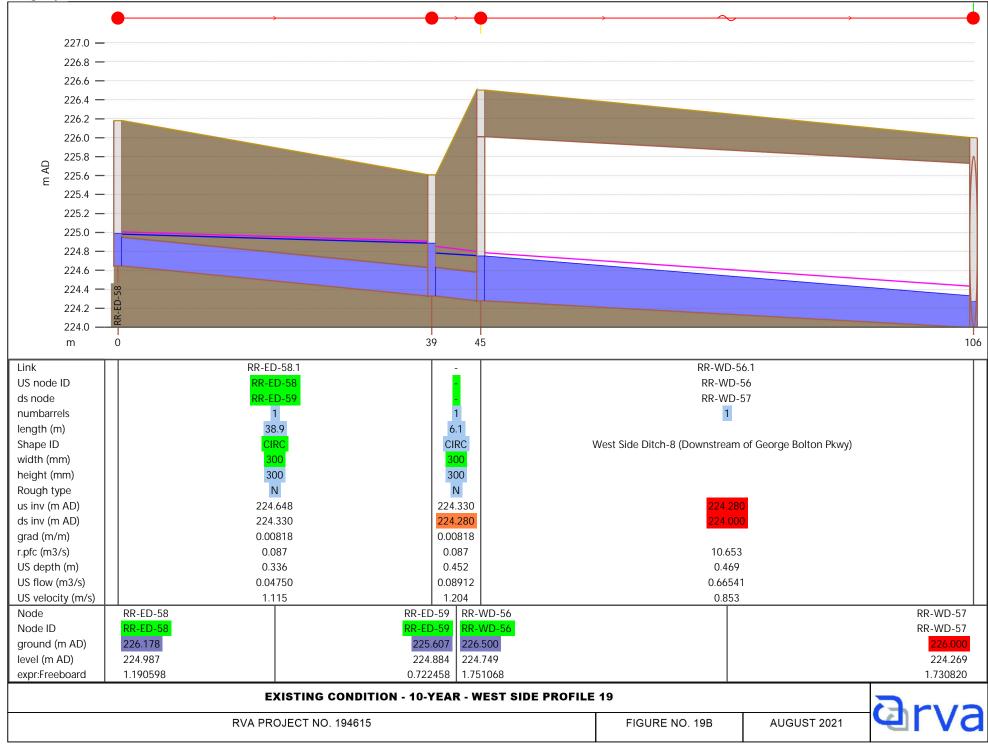




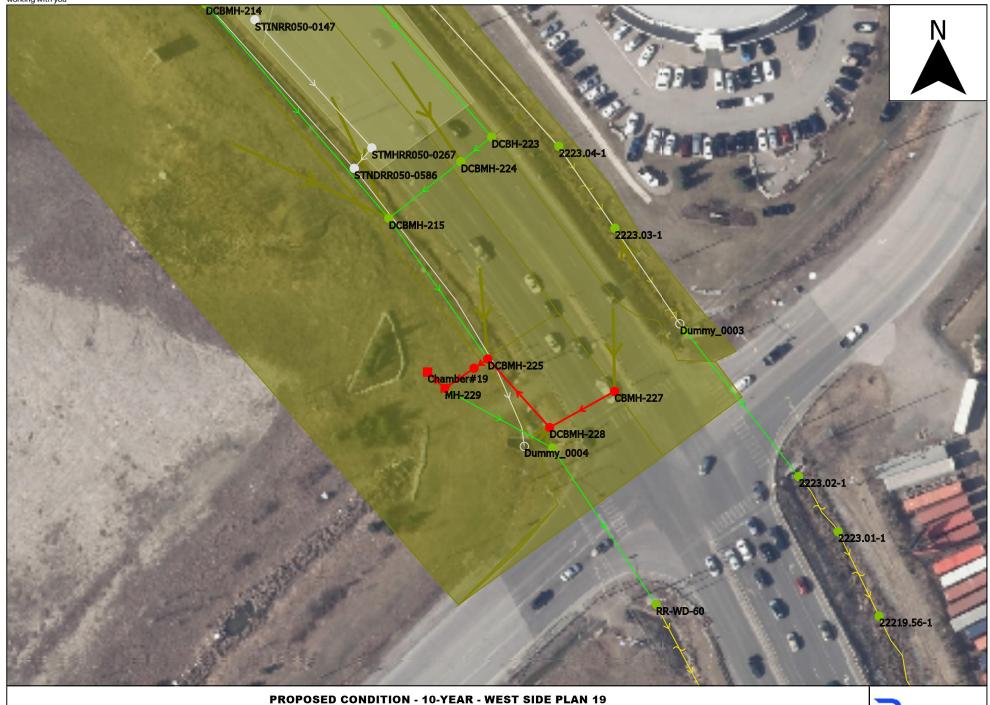








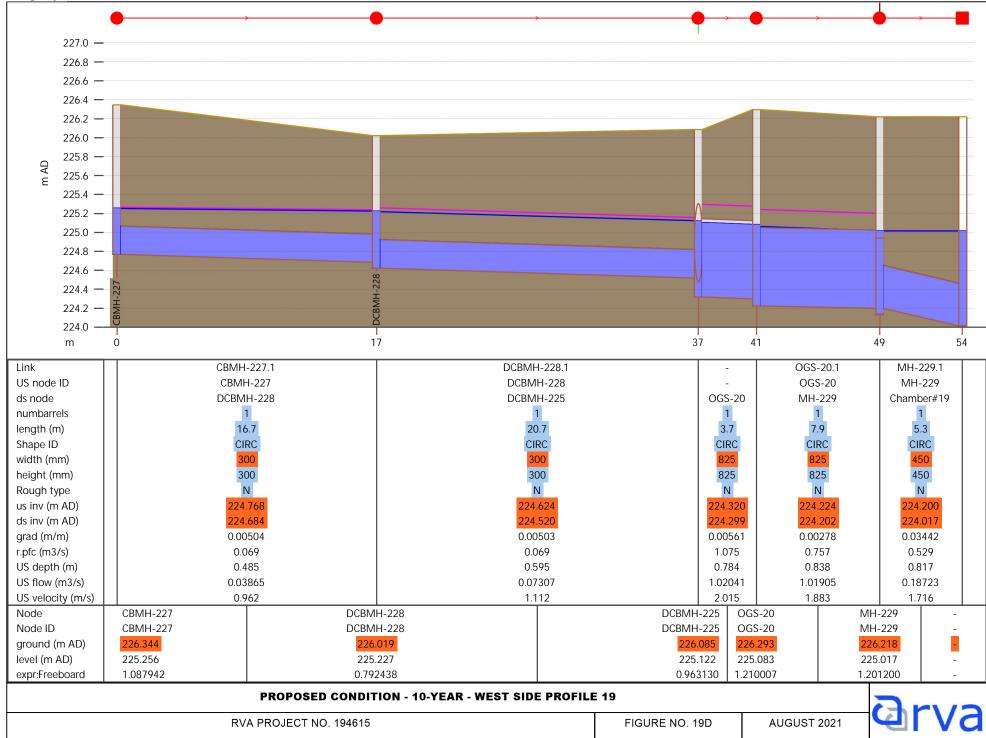




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FIGURE NO. 19C









EXISTING CONDITION - 100-YEAR - WEST SIDE PLAN 19

RVA PROJECT NO. 194615

FIGURE NO. 19E





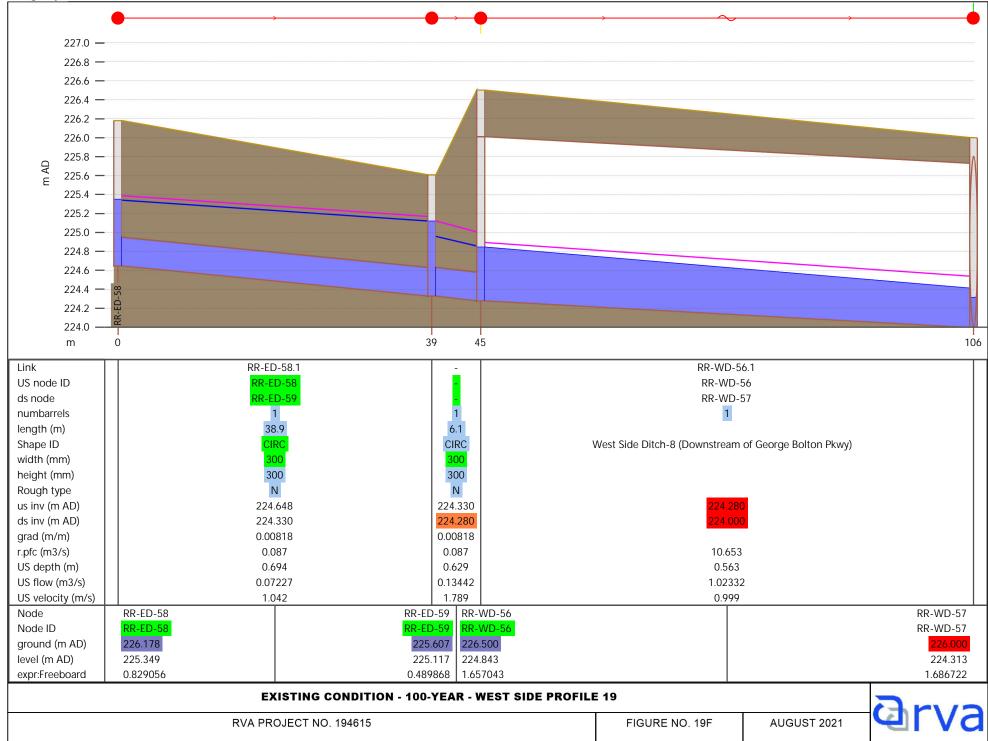








FIGURE NO. 19G





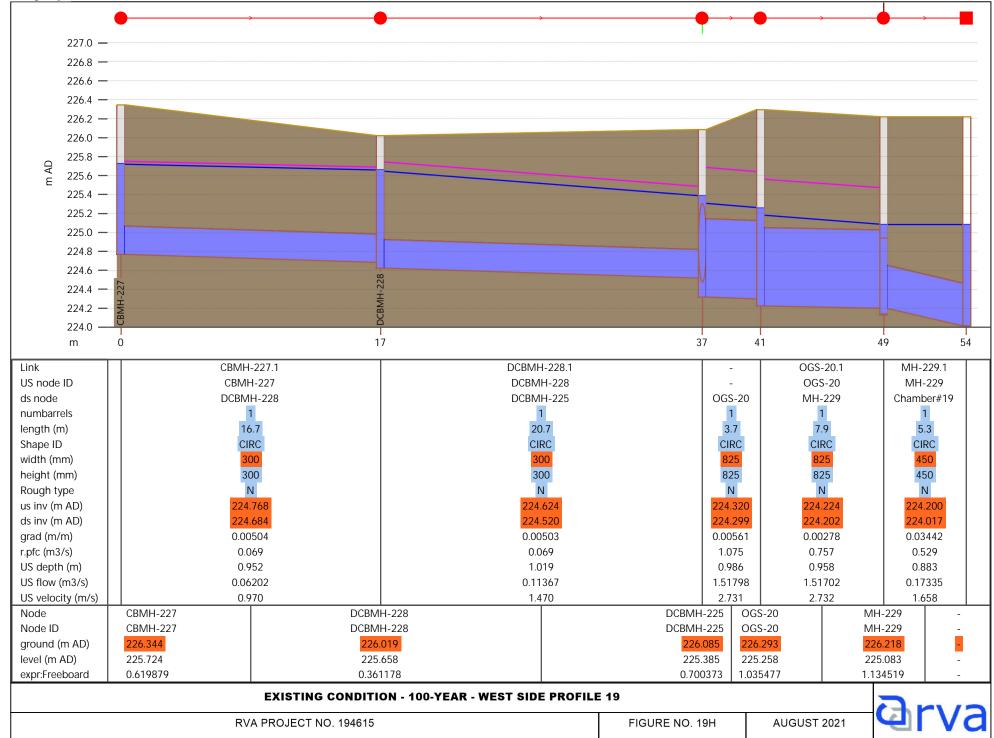




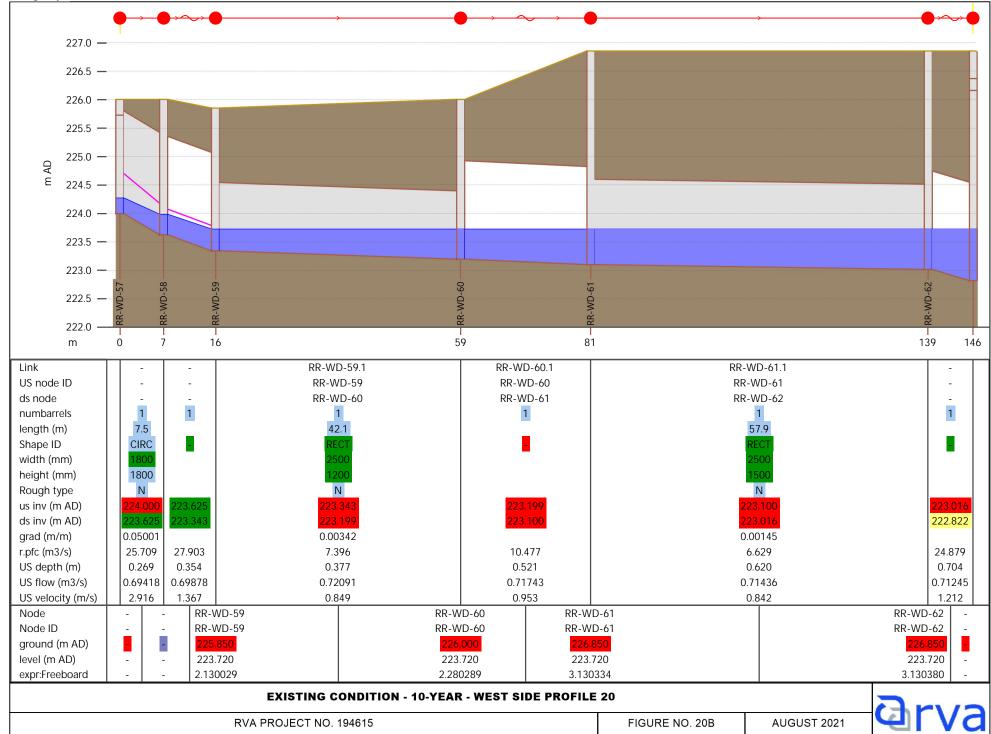




FIGURE NO. 20A











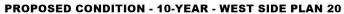
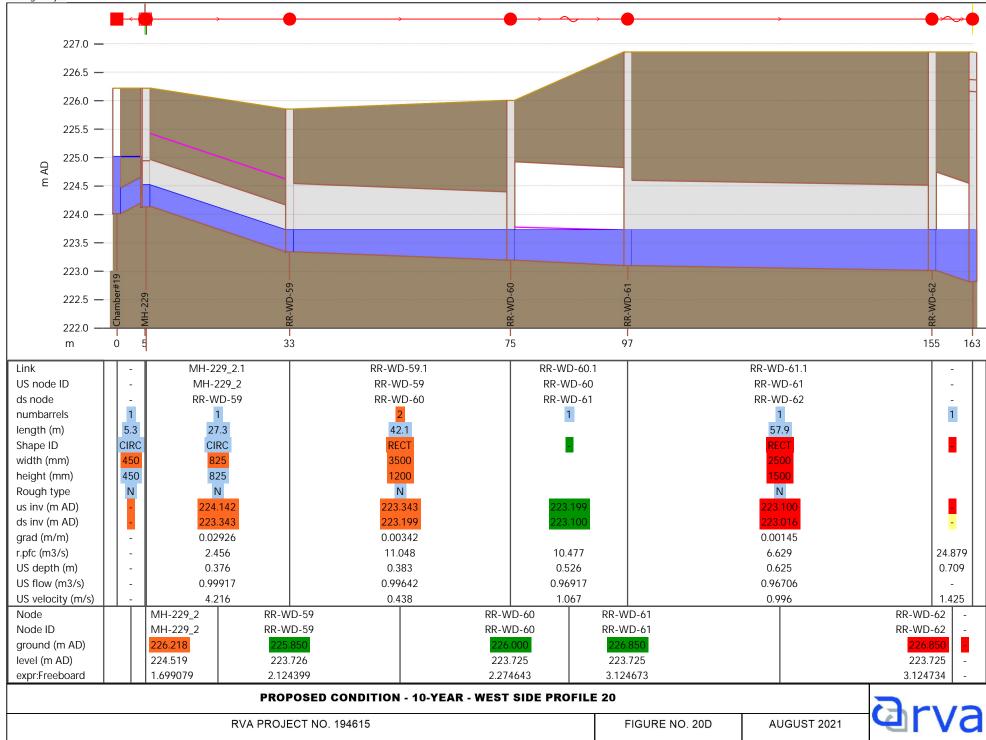


FIGURE NO. 20C











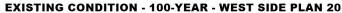
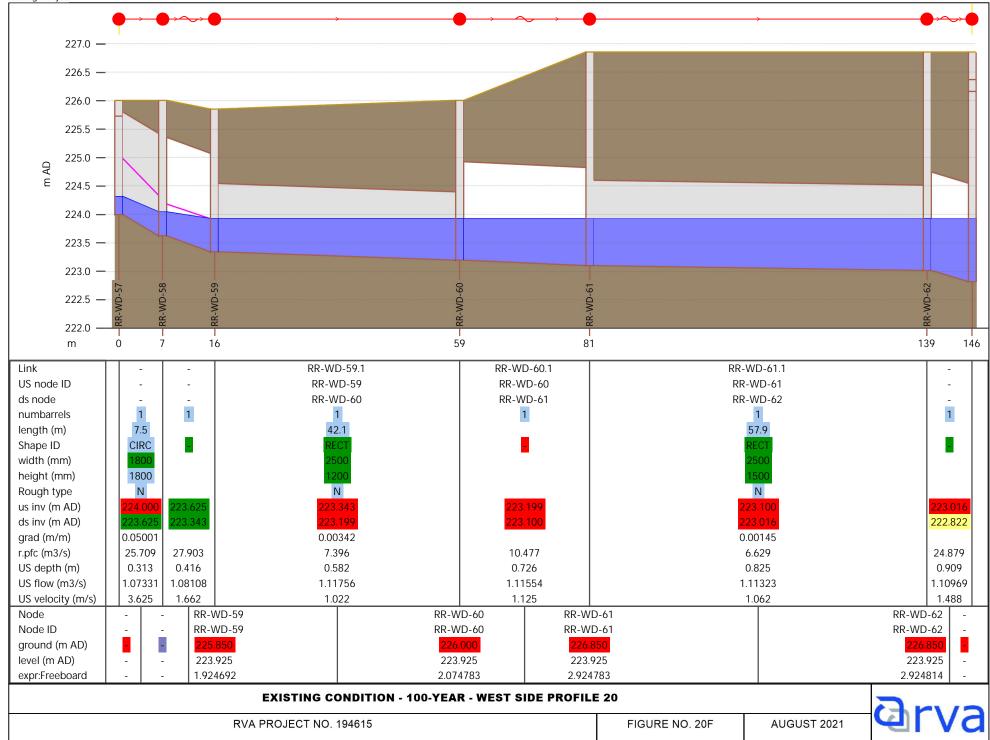


FIGURE NO. 20E











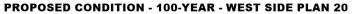
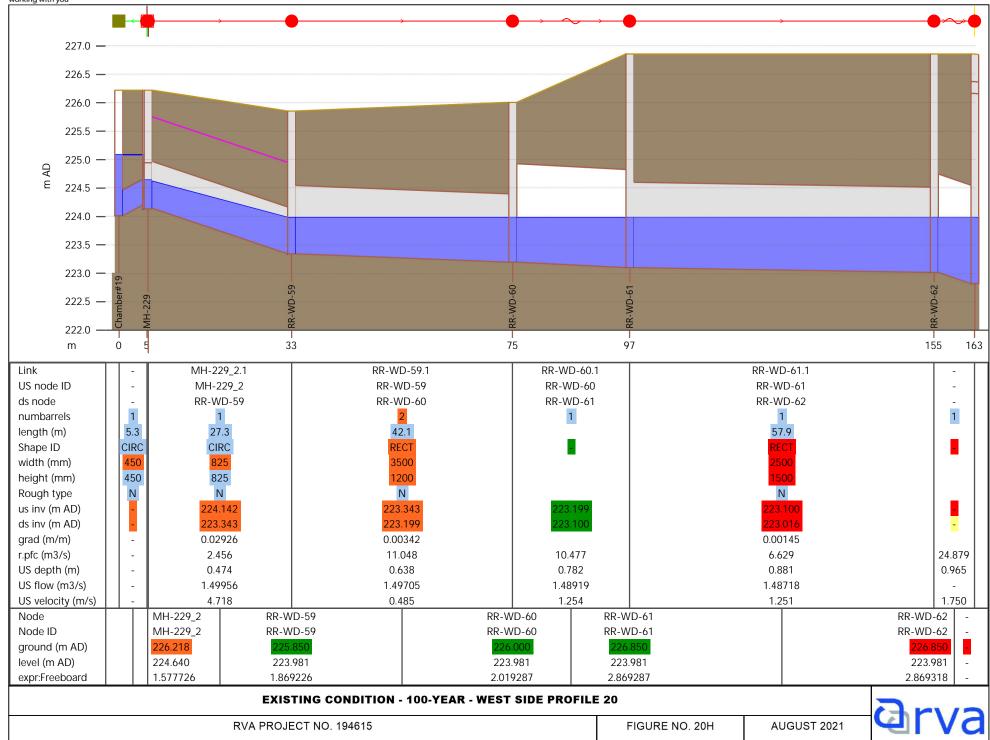


FIGURE NO. 20G











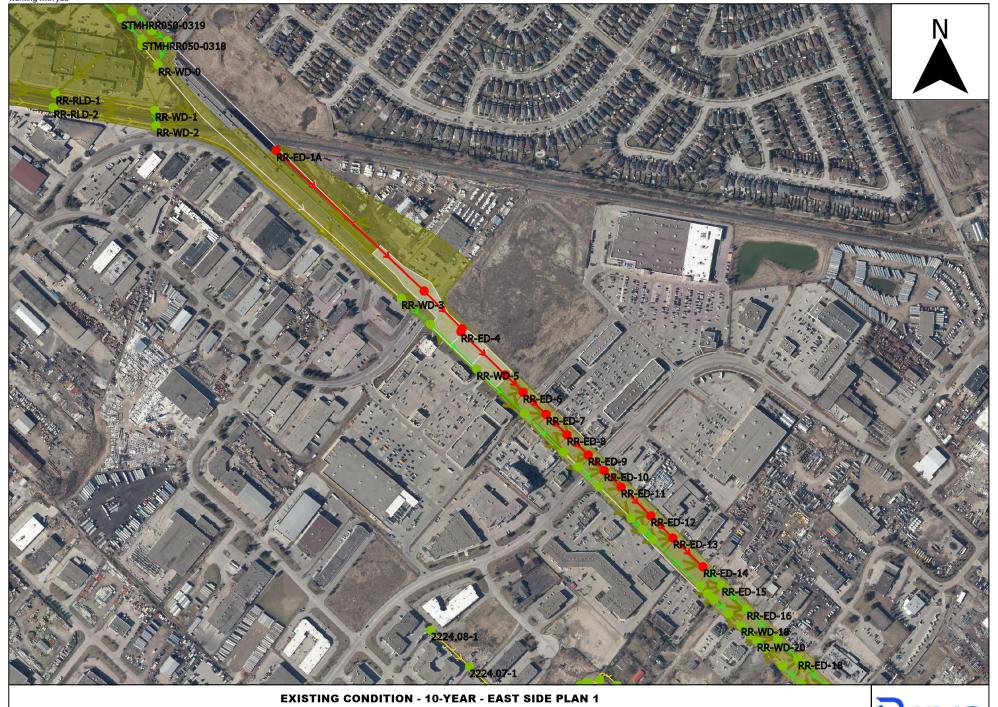
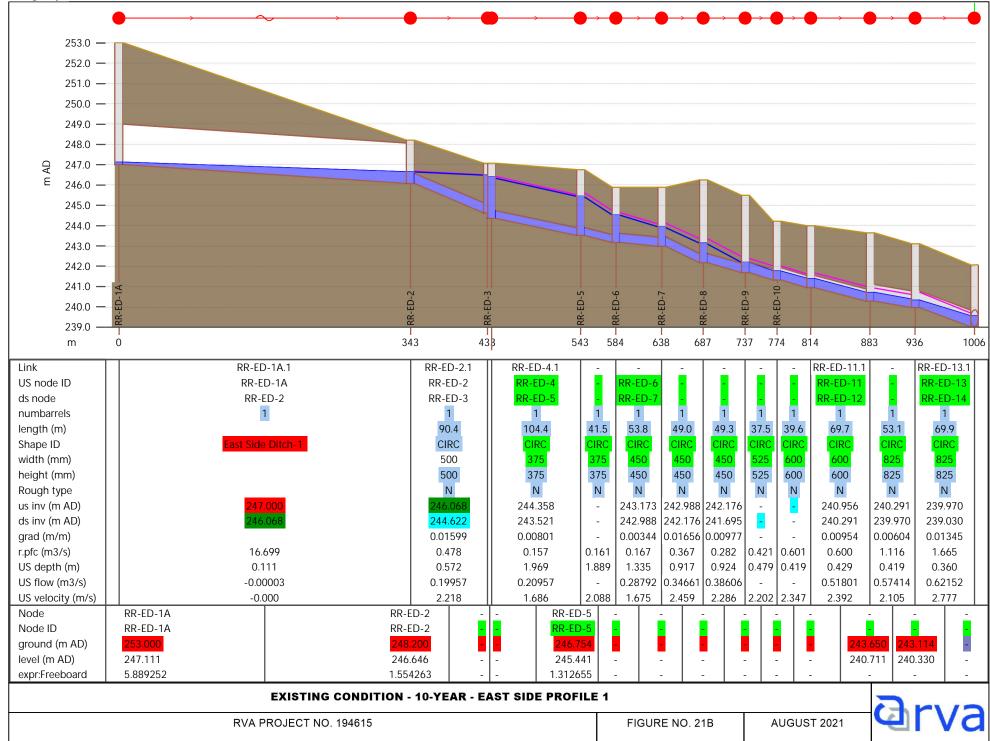
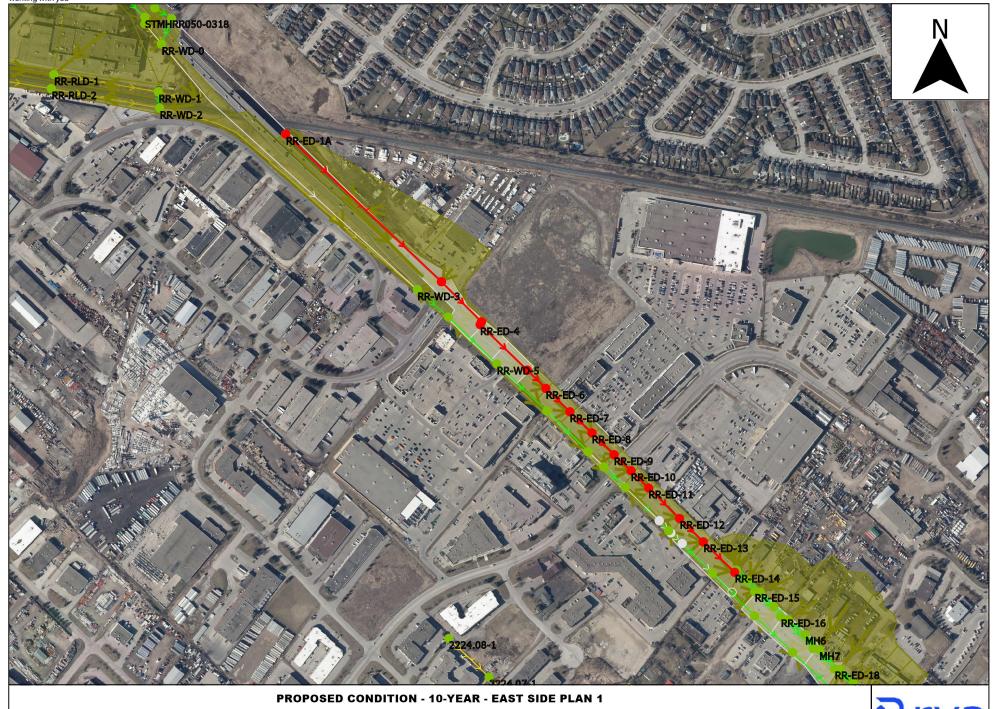


FIGURE NO. 21A





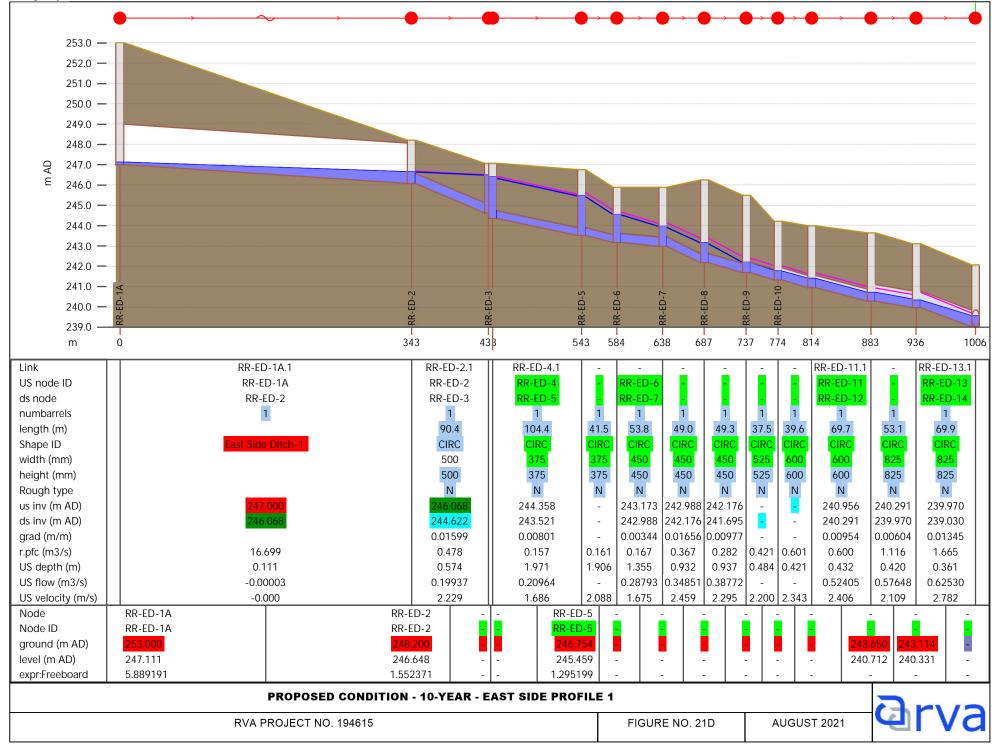




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FIGURE NO. 21C







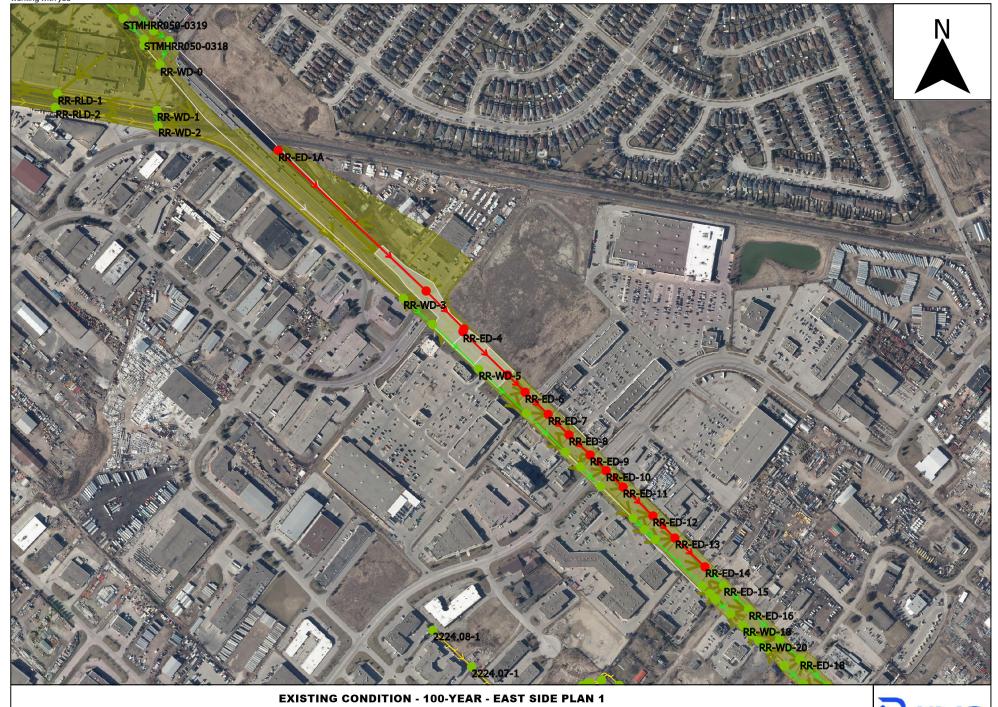
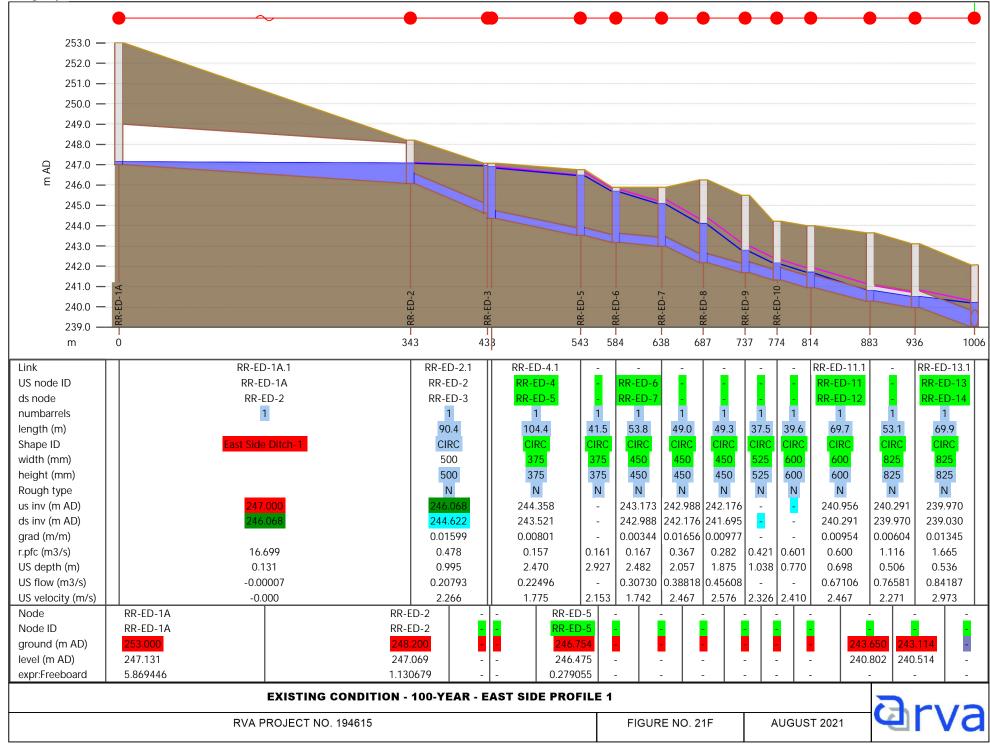
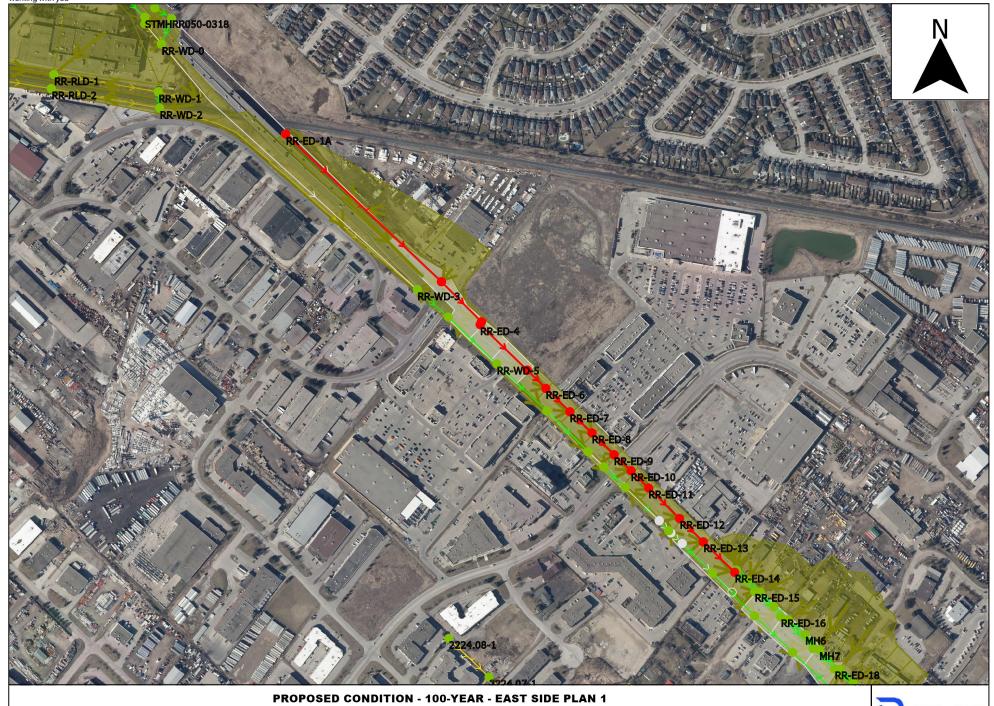


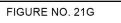
FIGURE NO. 21E



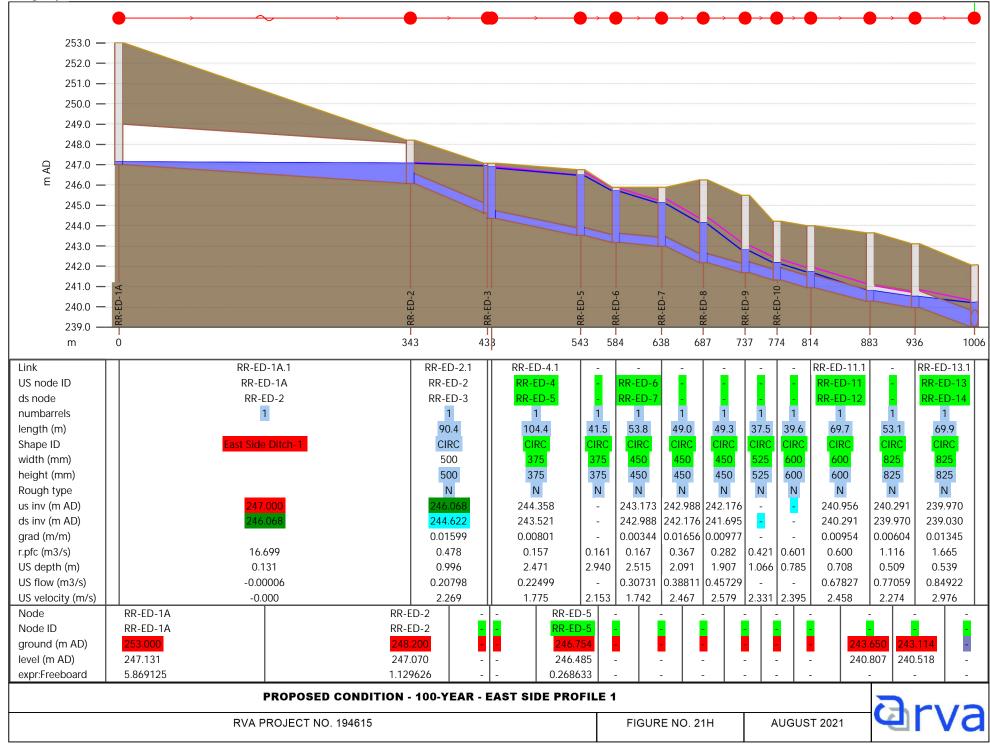














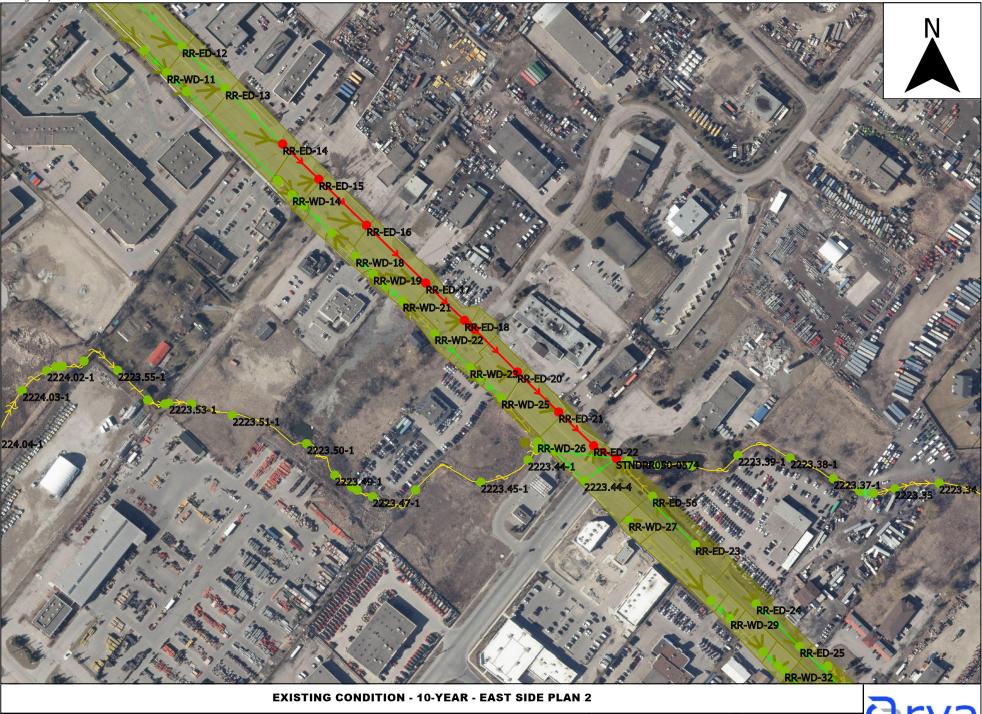
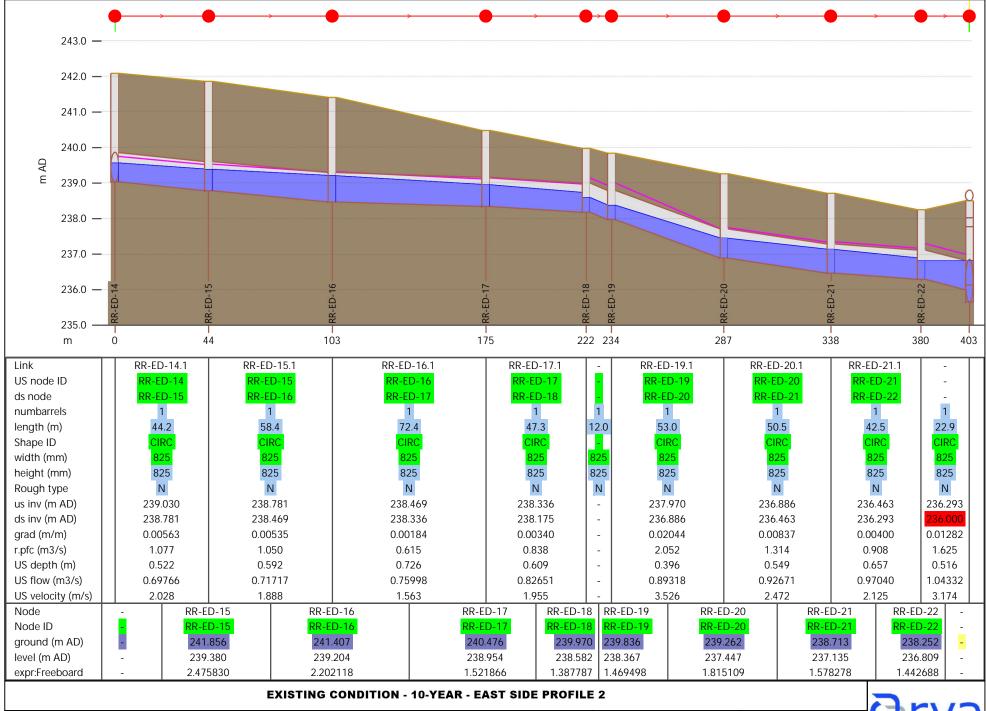


FIGURE NO. 22A AUGUST 2021







RVA PROJECT NO. 194615 FIGURE NO. 22B





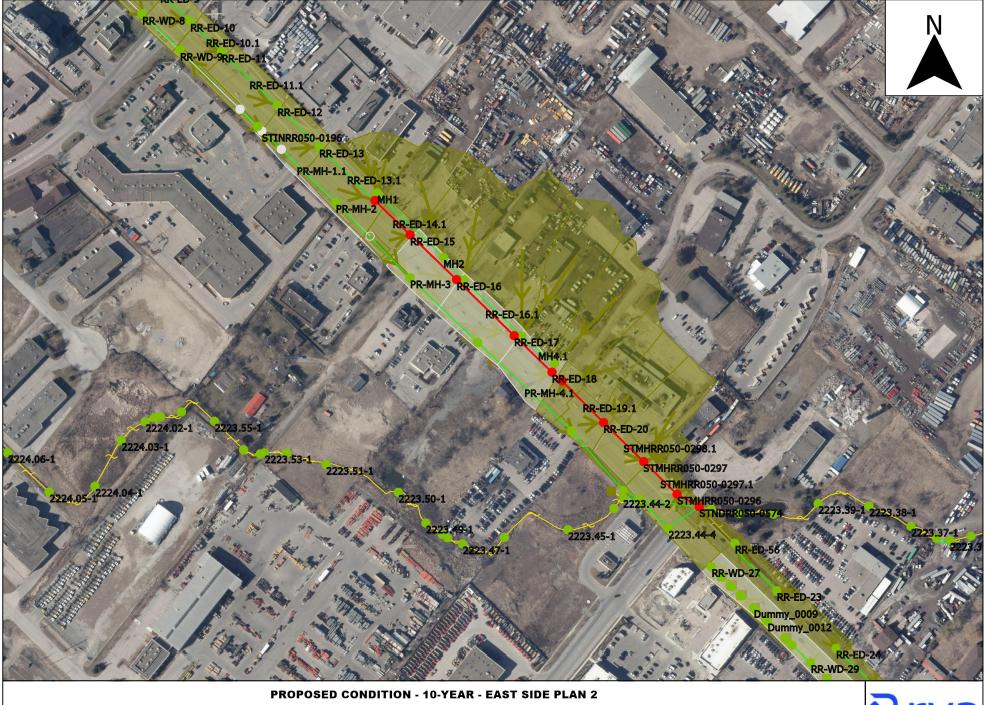


FIGURE NO. 22C AUGUST 2021





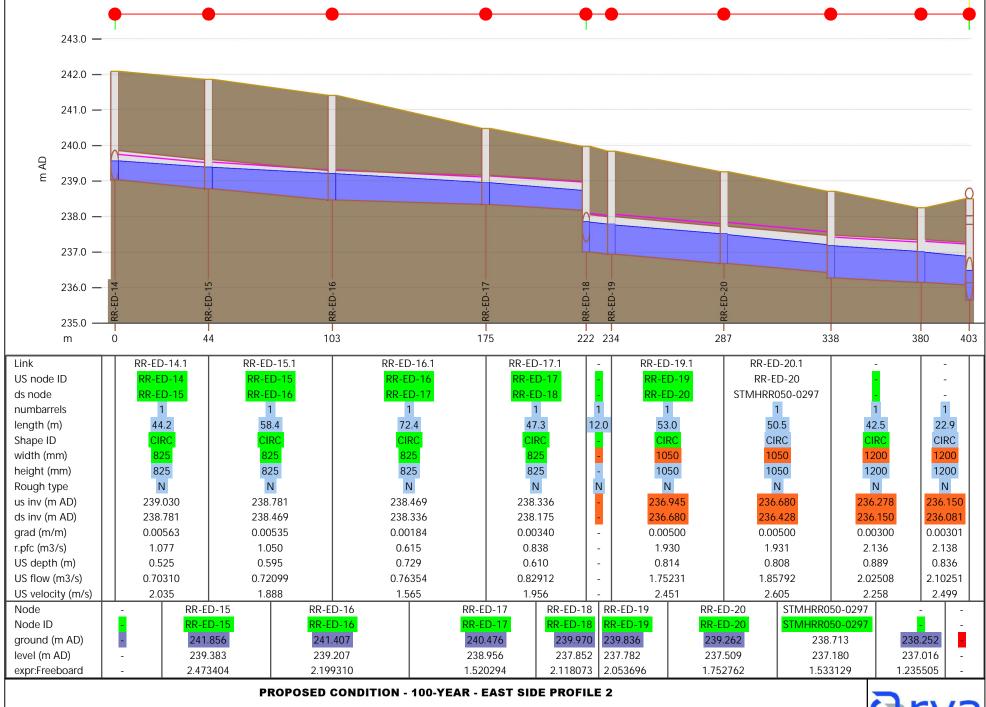
PROPOSED CONDITION - 10-YEAR - EAST SIDE PLAN 2-1 (PROPOSED STORM SEWERS UNDERNEATH THE PROPOSED SIDEWALK)

RVA PROJECT NO. 194615

FIGURE NO. 22C-1



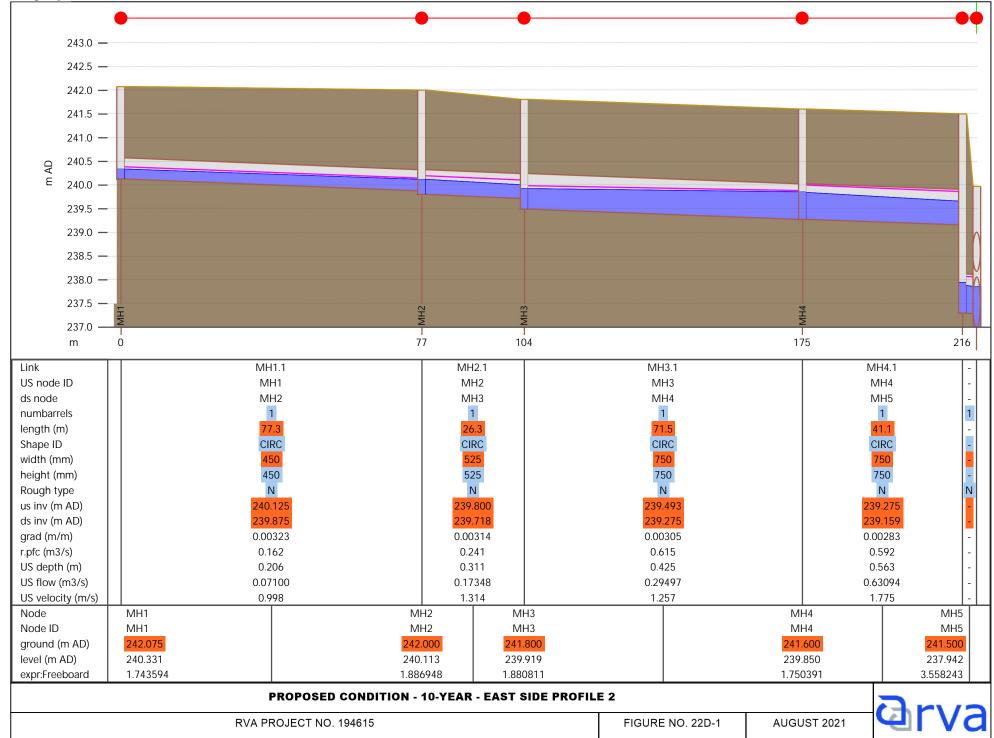




RVA PROJECT NO. 194615 FIGURE NO. 22D



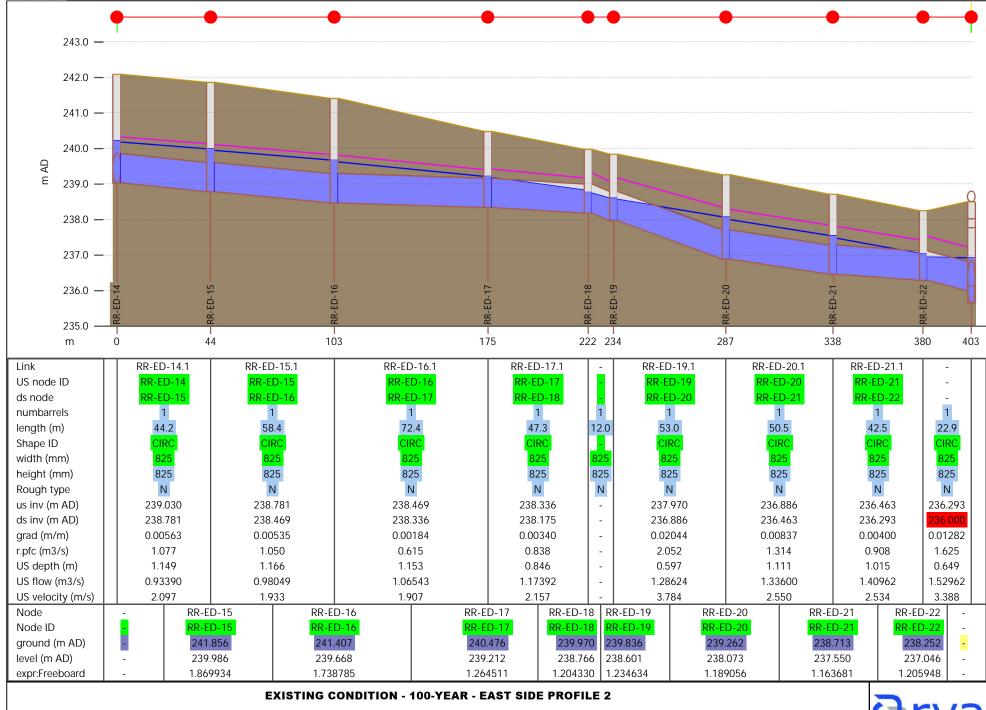












RVA PROJECT NO. 194615 FIGURE NO. 22F

arva





FIGURE NO. 22G



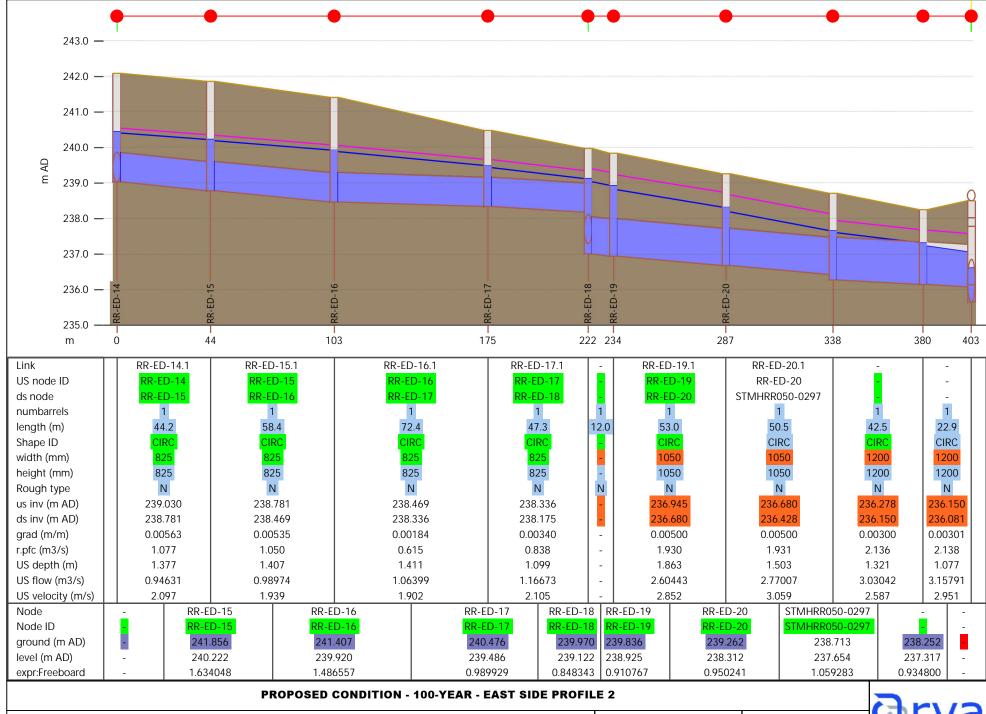




AUGUST 2021 FIGURE NO. 22G-1

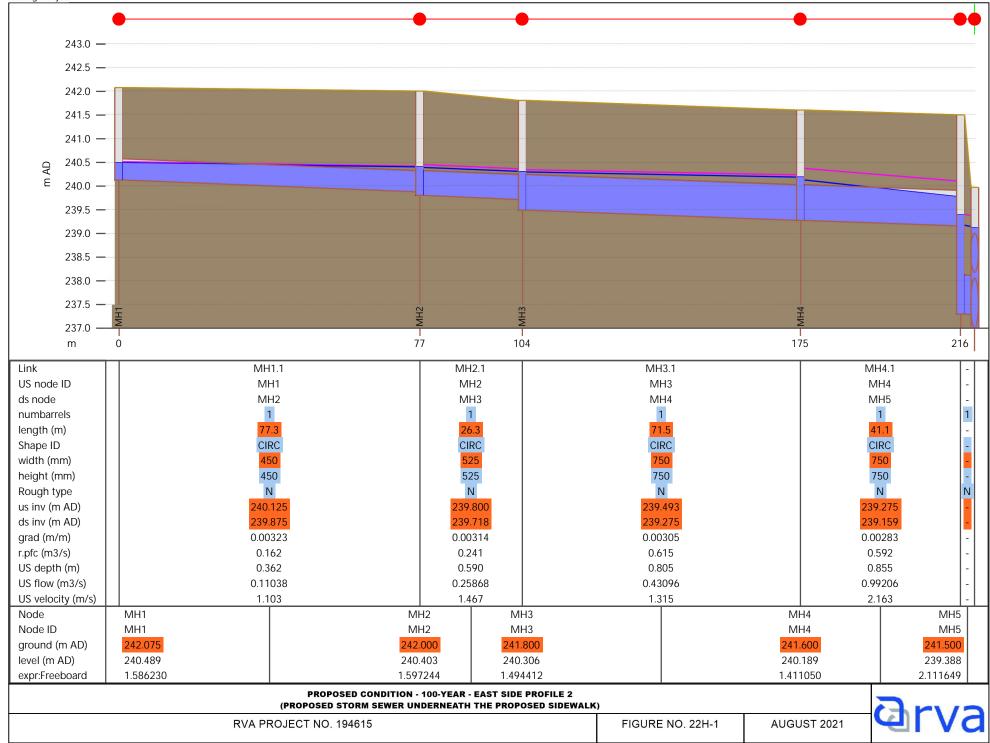






22H AUGUST 2021







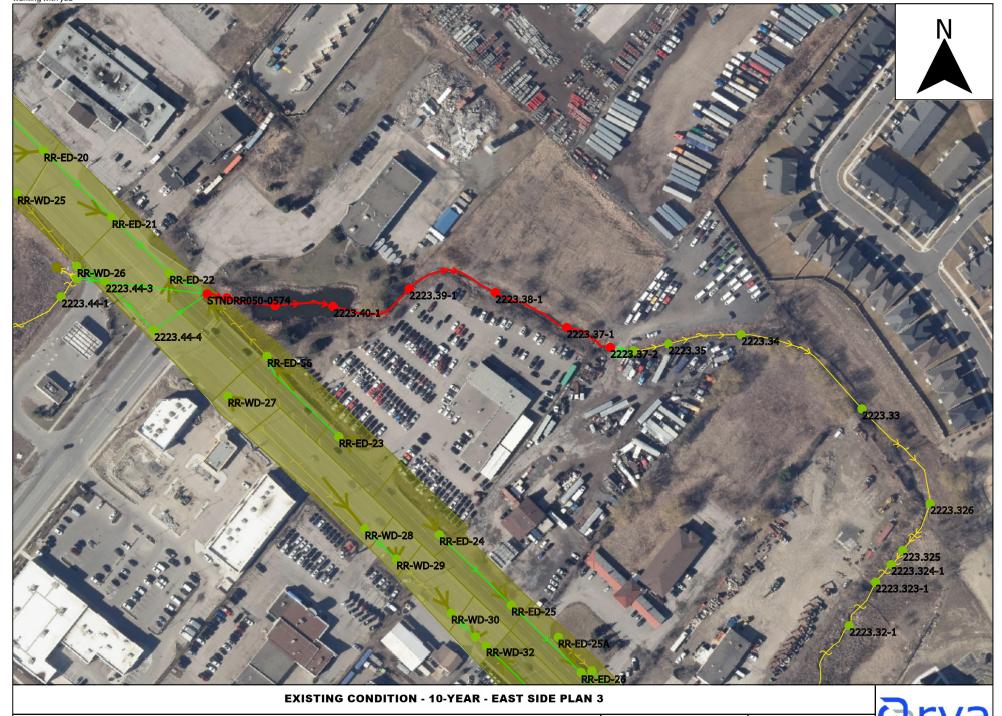


FIGURE NO. 23A



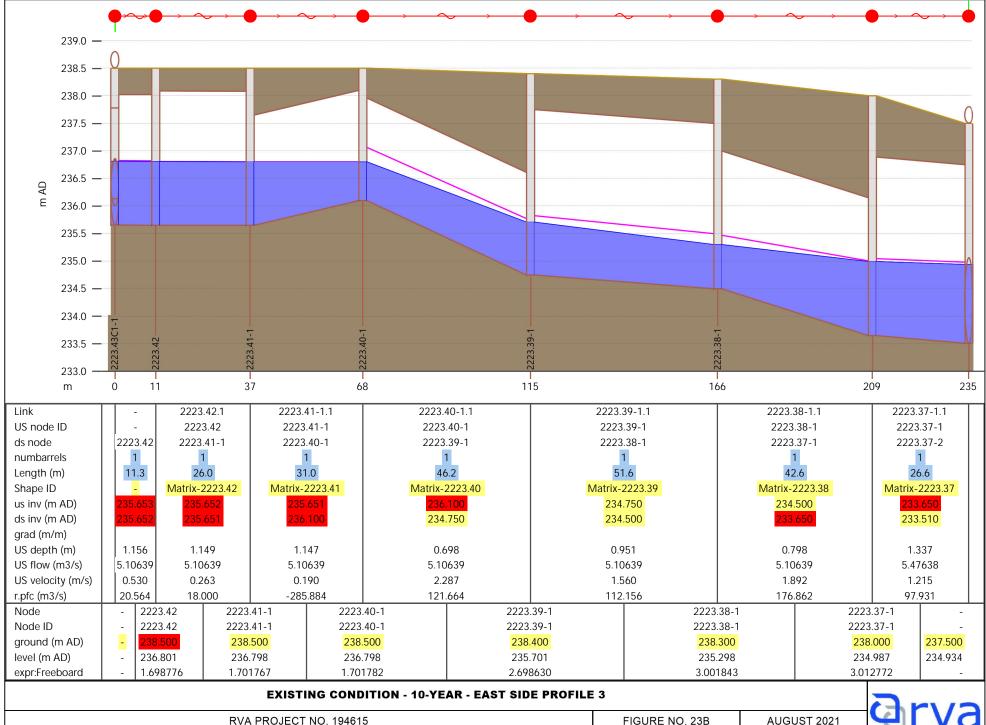


FIGURE NO. 23B AUGUST 2021





FIGURE NO. 23C





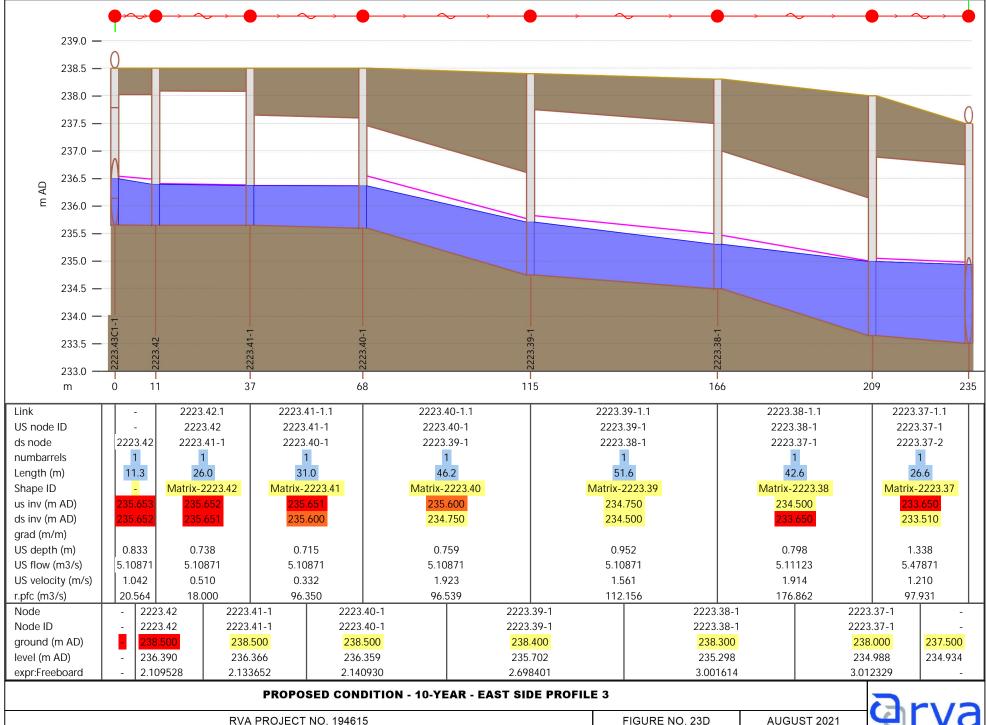
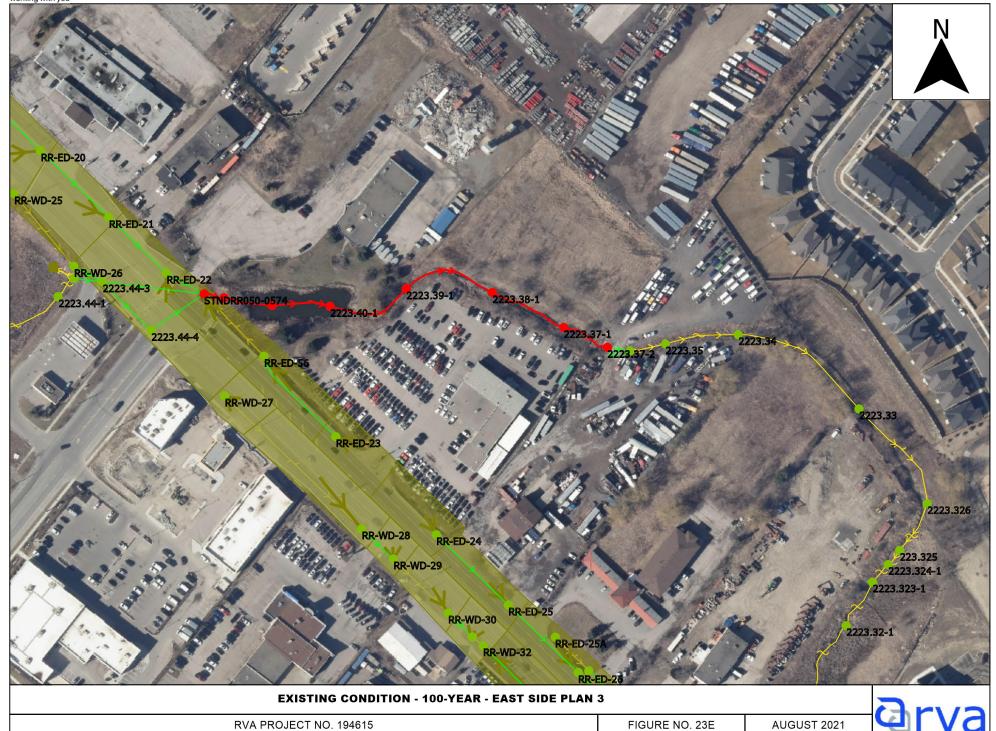
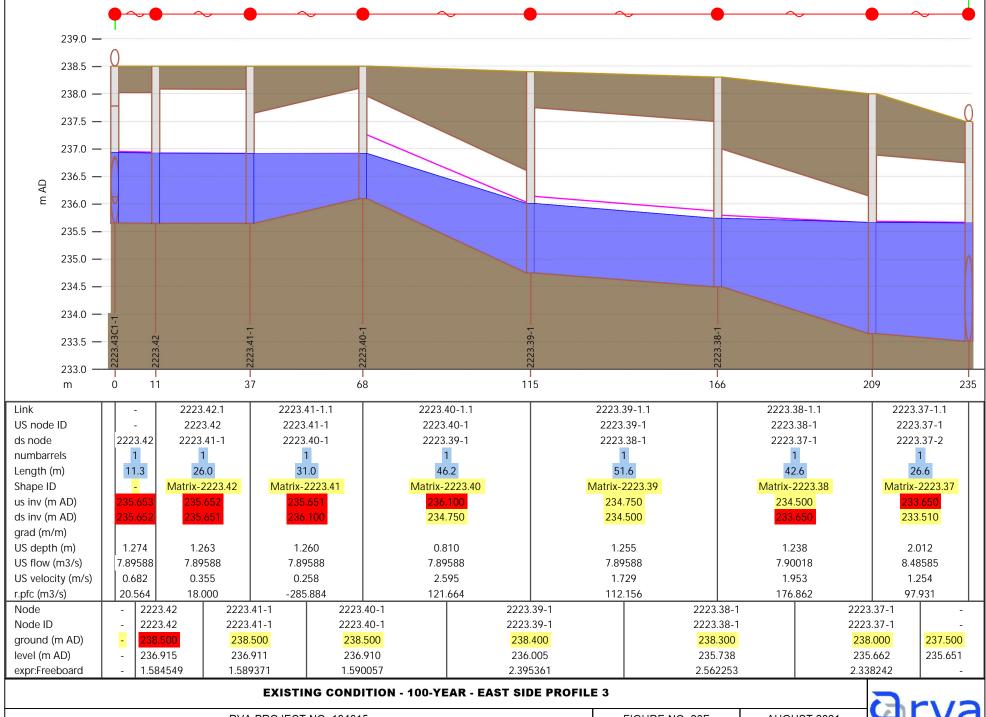


FIGURE NO. 23D AUGUST 2021











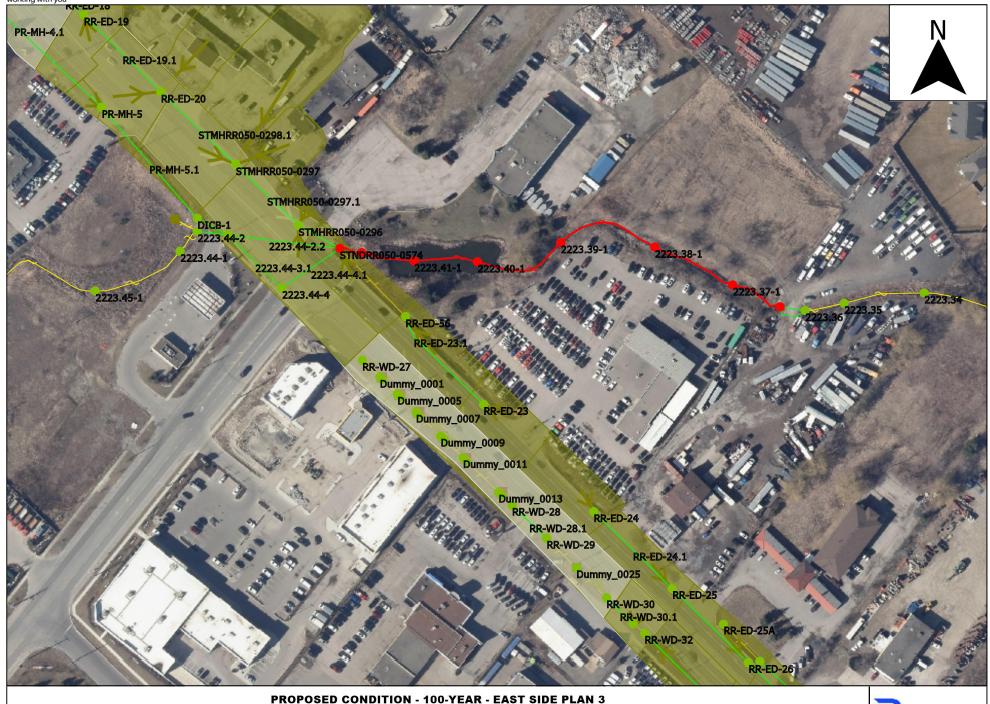
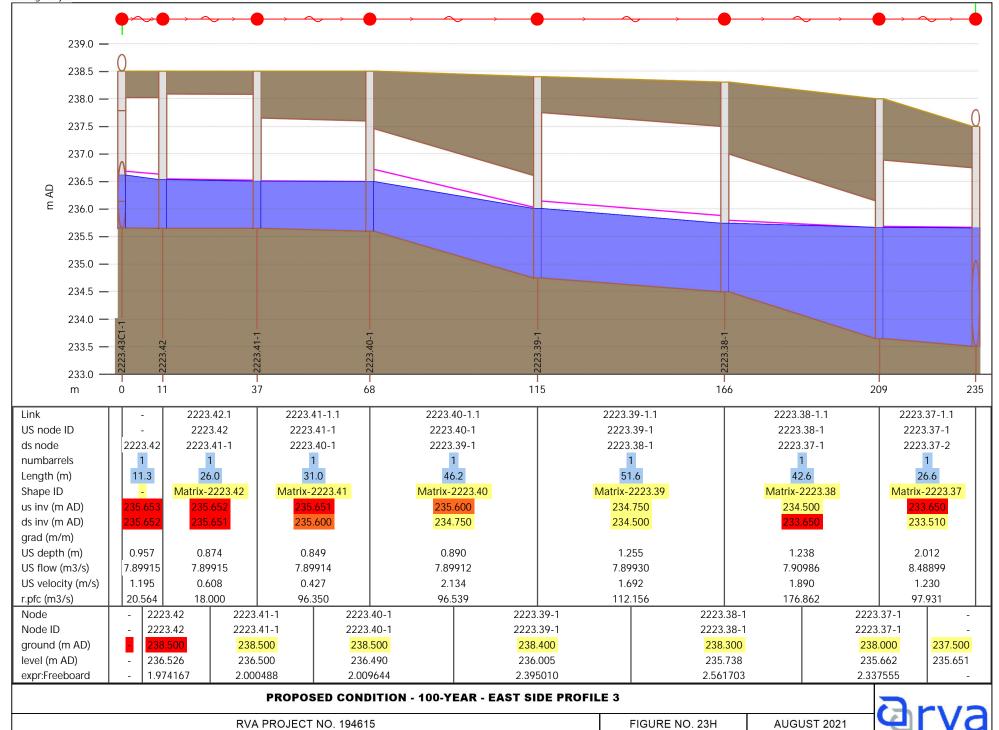
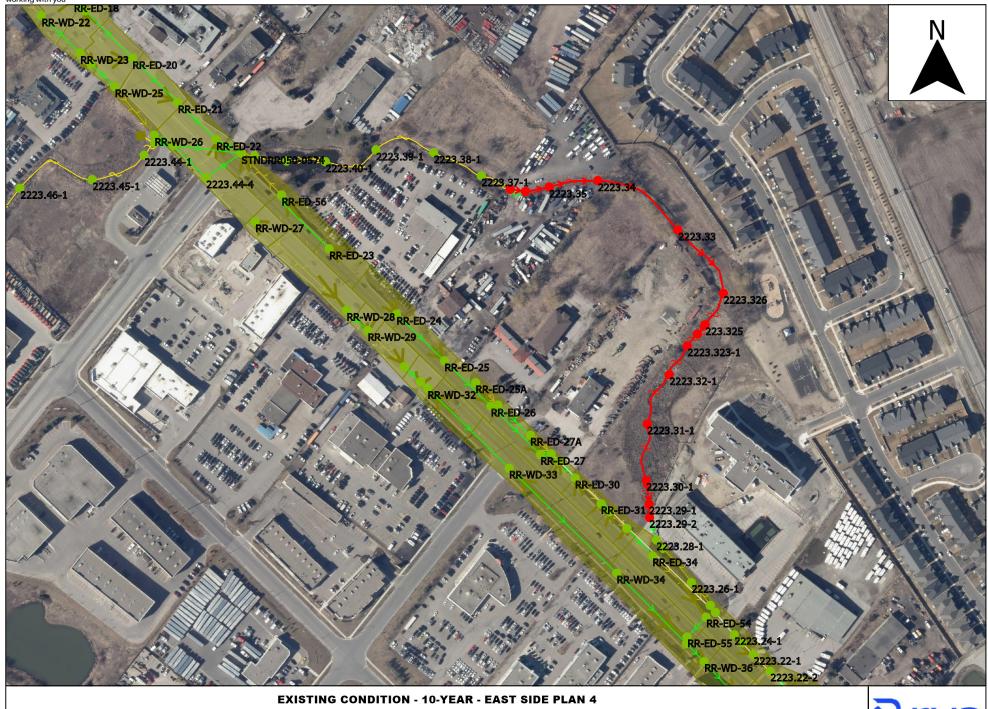


FIGURE NO. 23G AUGUST 2021





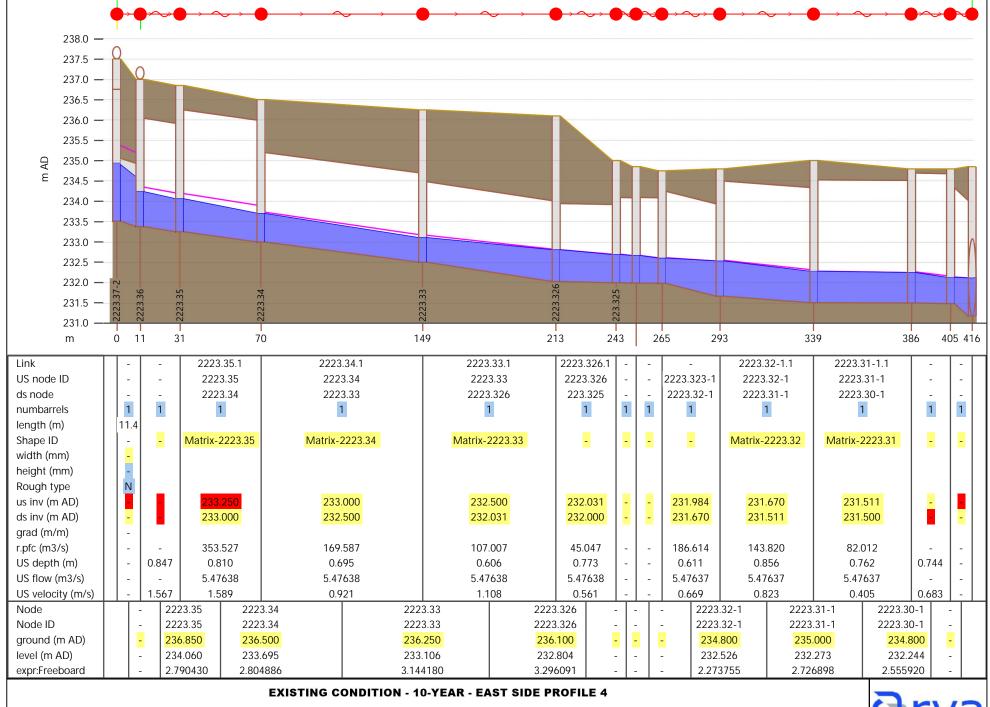




AUGUST 2021

FIGURE NO. 24A

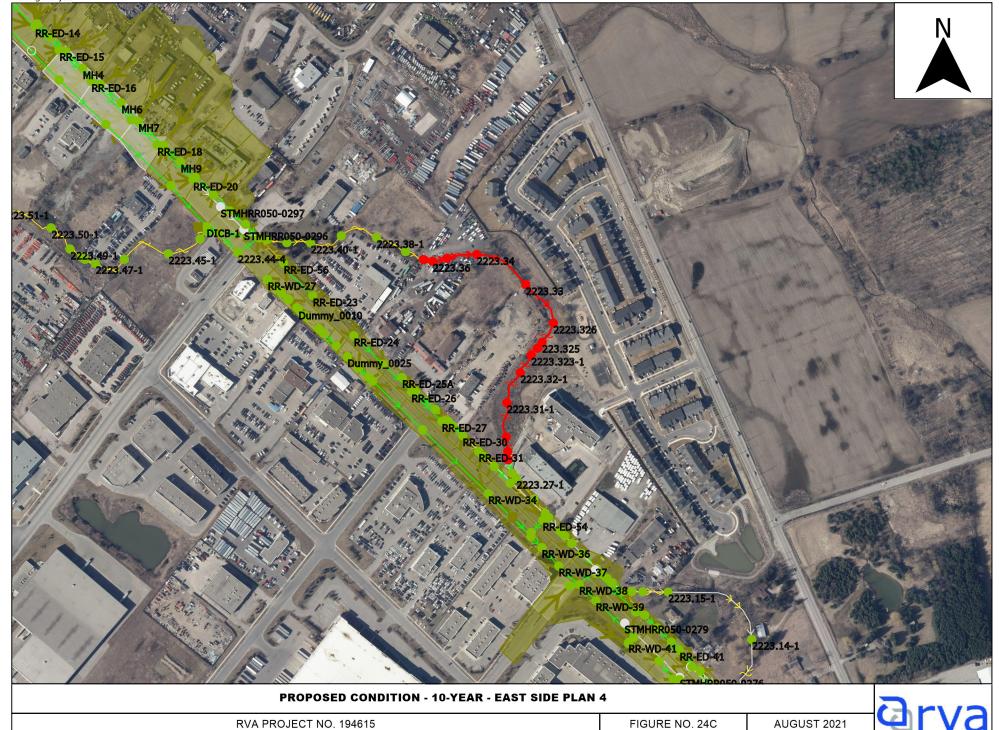




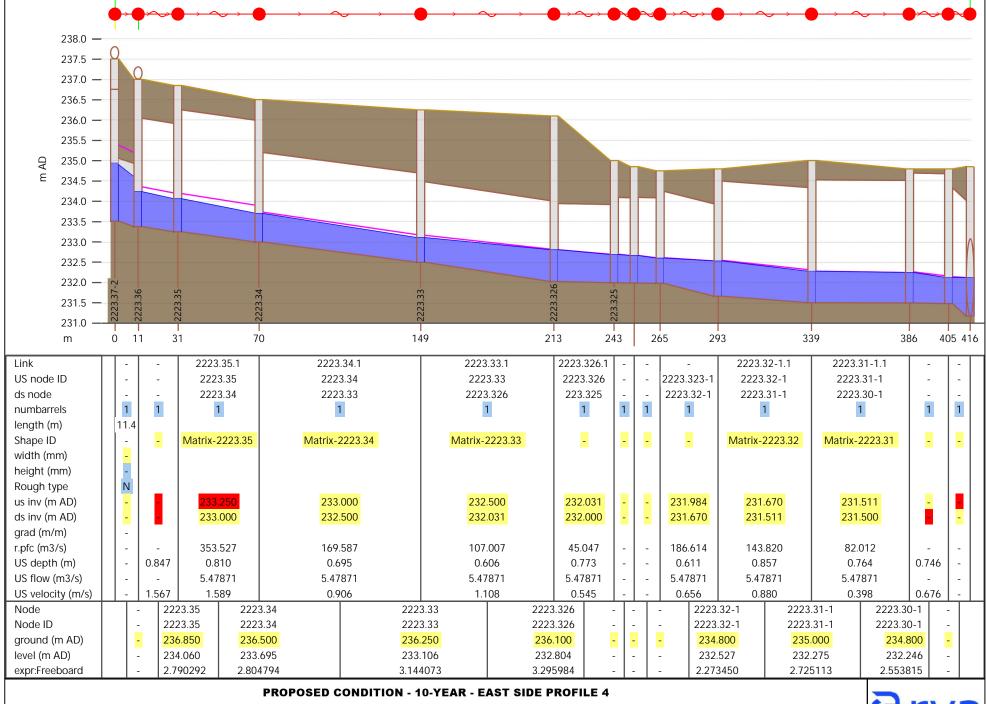
RVA PROJECT NO. 194615 FIGURE NO. 24B







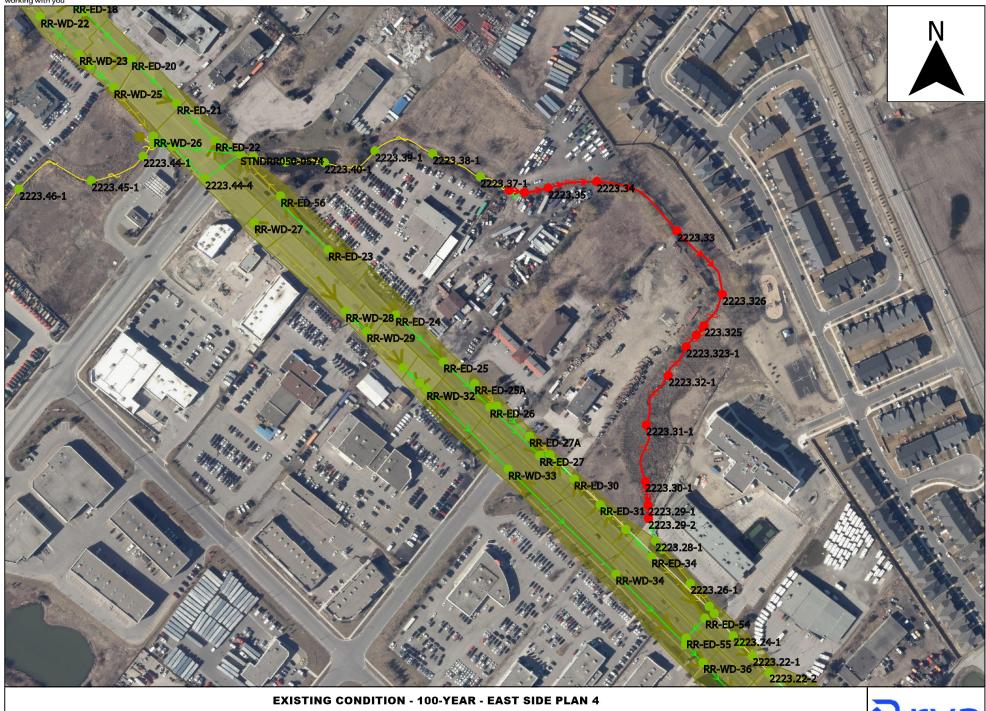




RVA PROJECT NO. 194615 FIGURE NO. 24D







AUGUST 2021

FIGURE NO. 24E



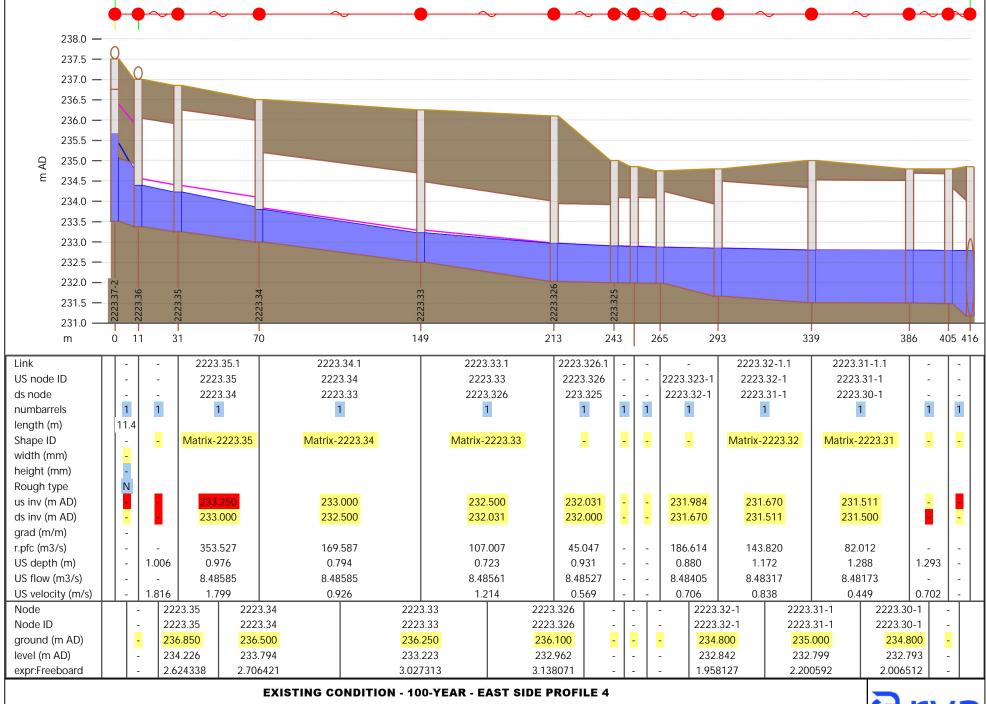
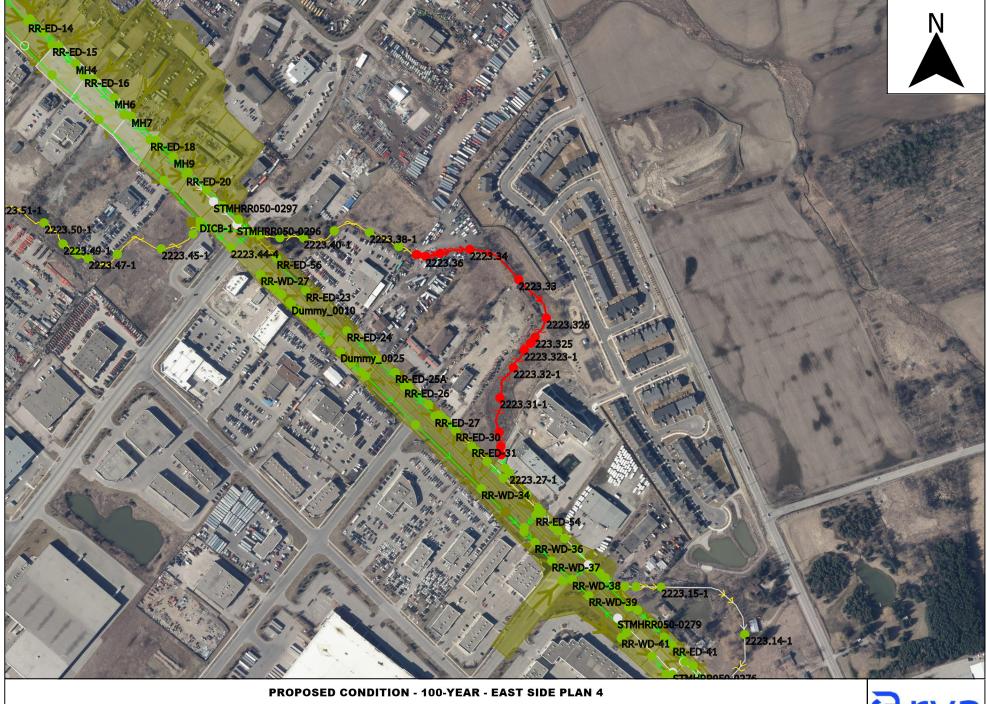


FIGURE NO. 24F

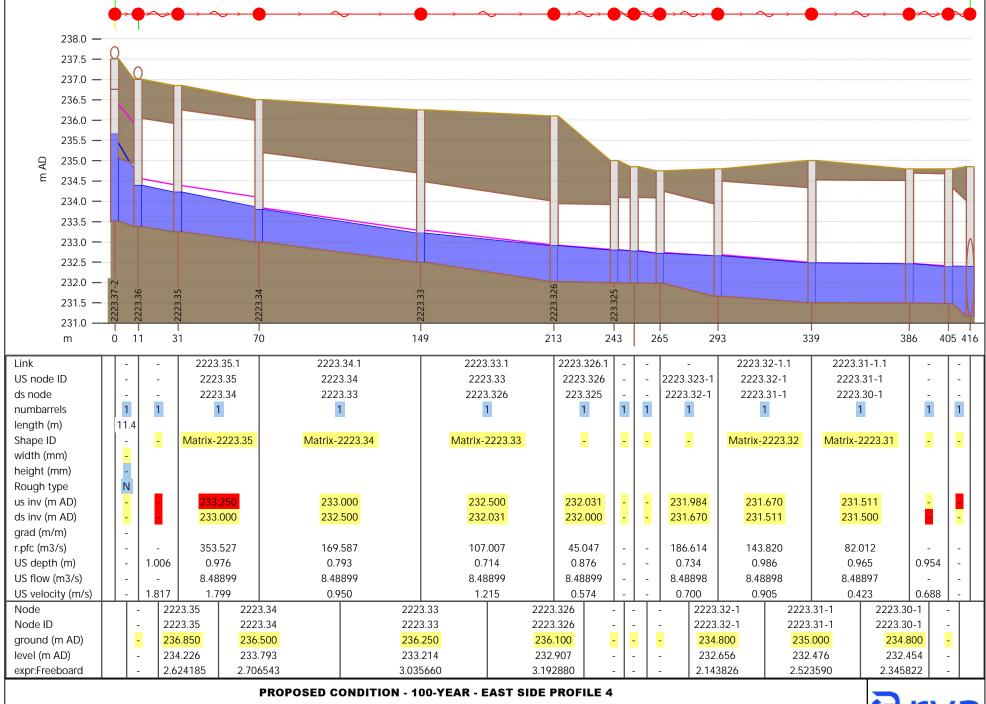












RVA PROJECT NO. 194615 FIGURE NO. 24H





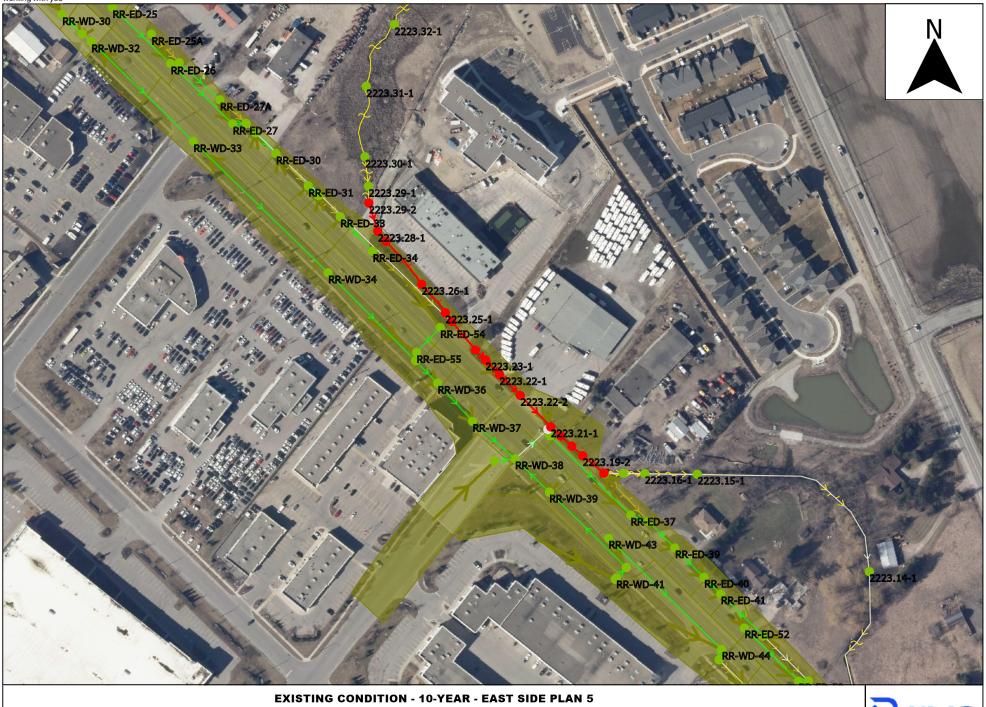
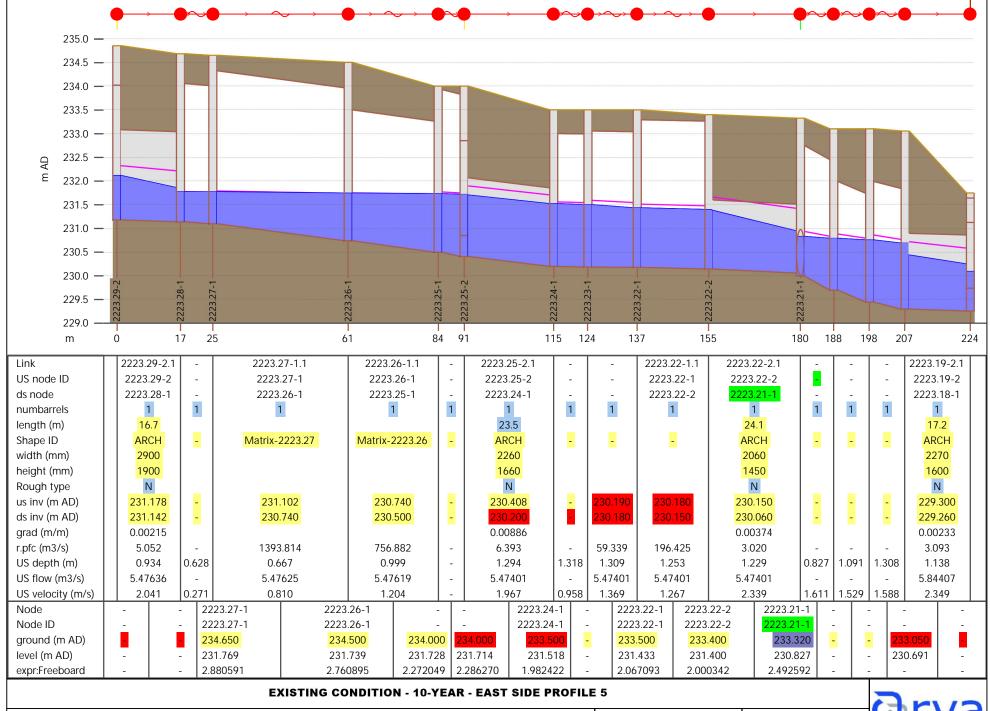
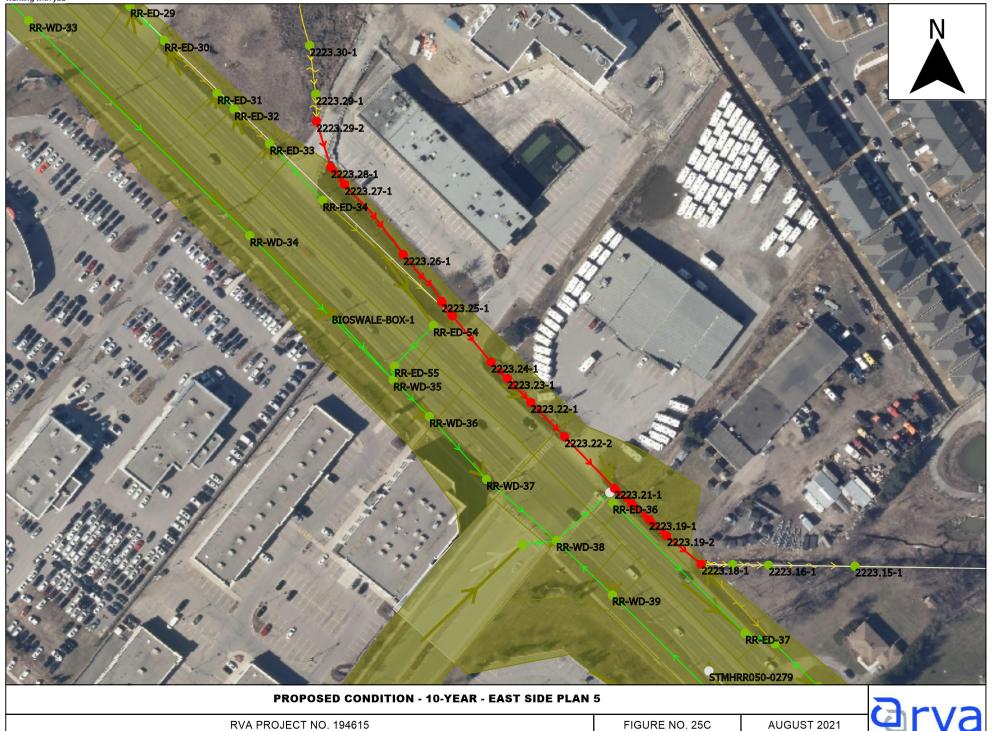


FIGURE NO. 25A

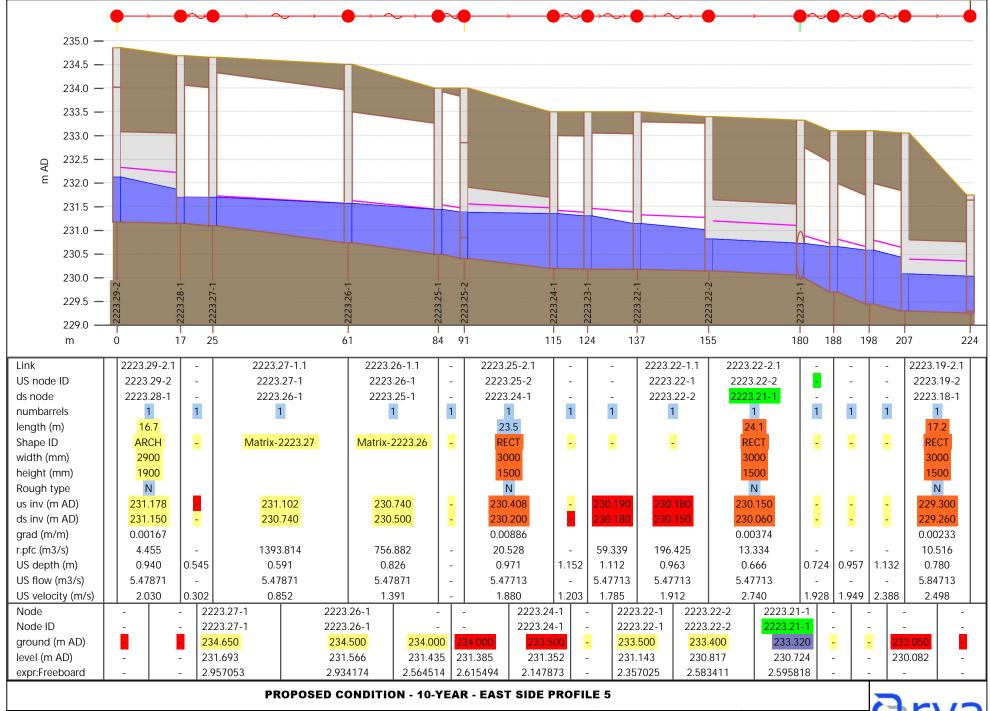














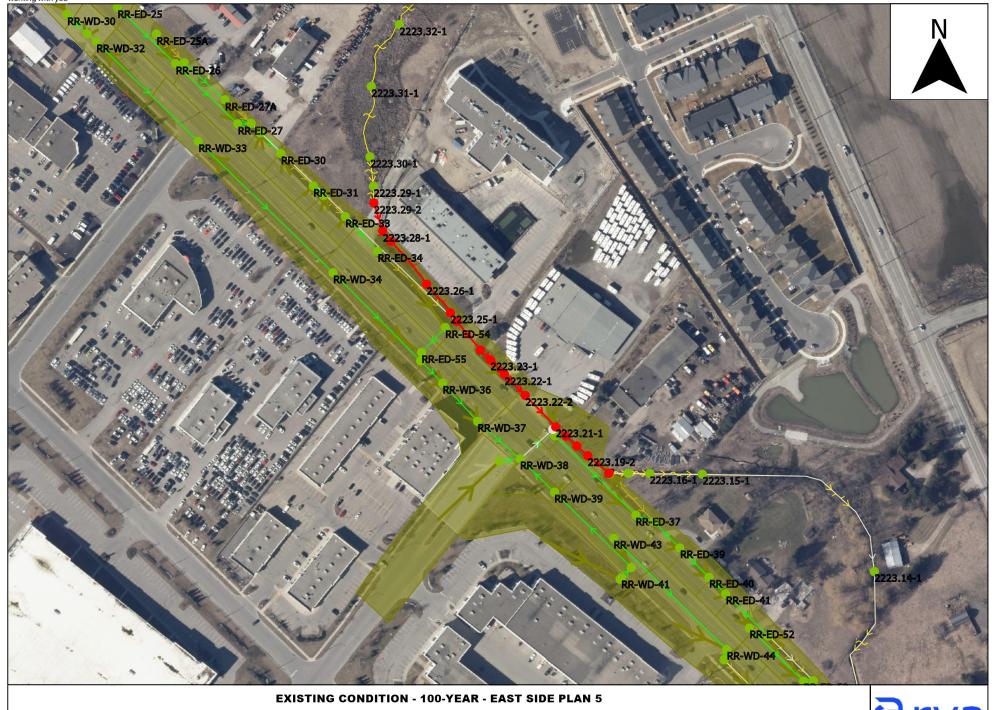
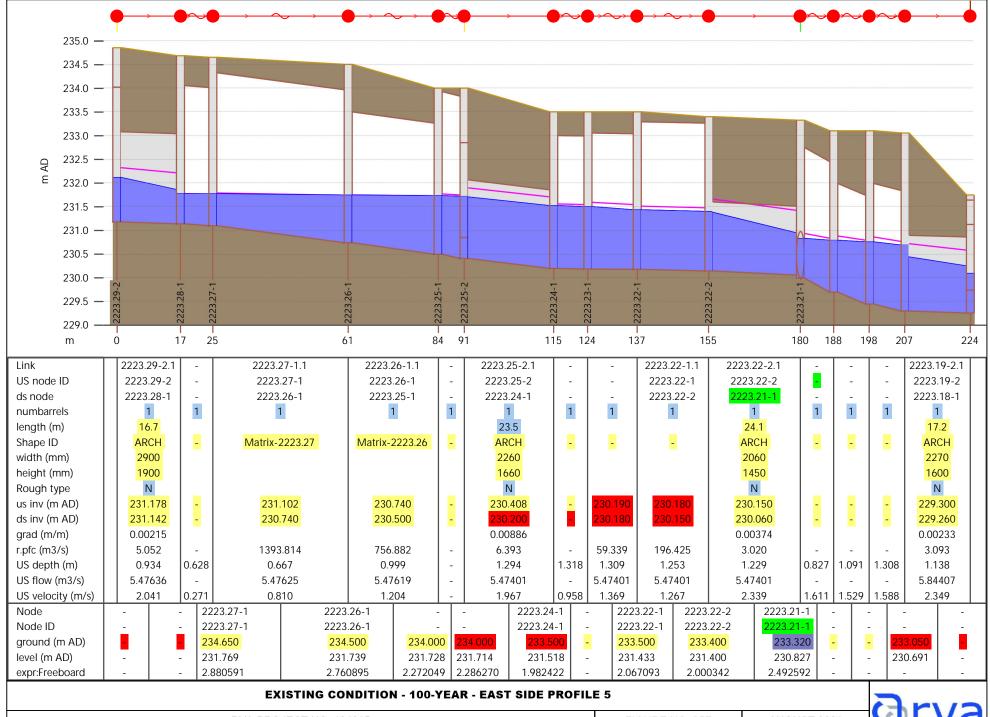


FIGURE NO. 25E AUGUST 2021





RVA PROJECT NO. 194615 FIGURE NO. 25F





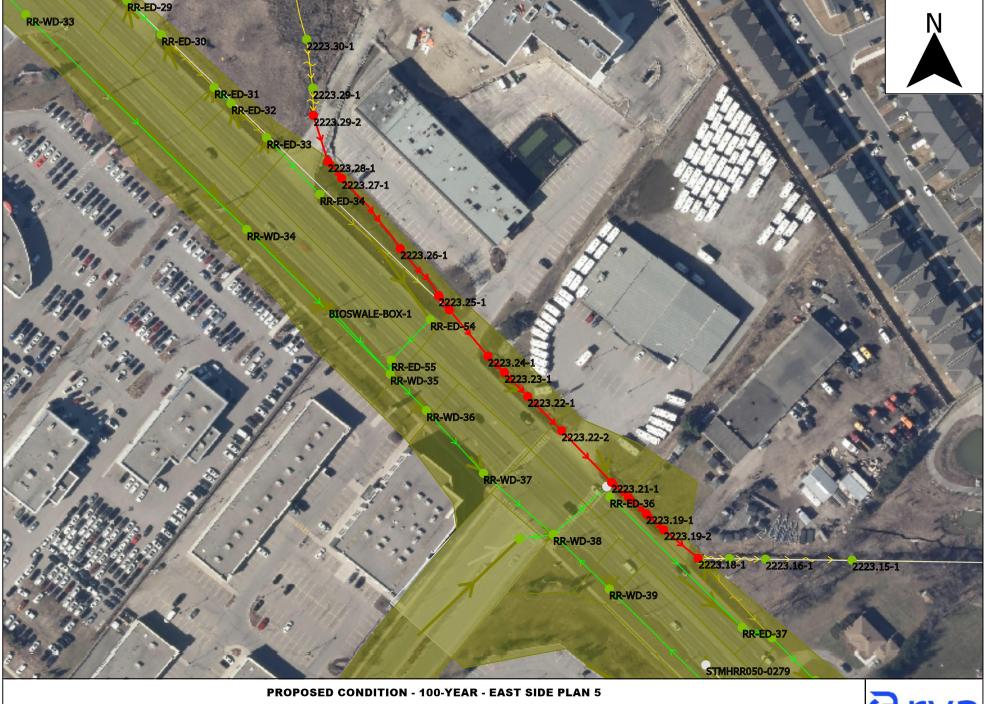
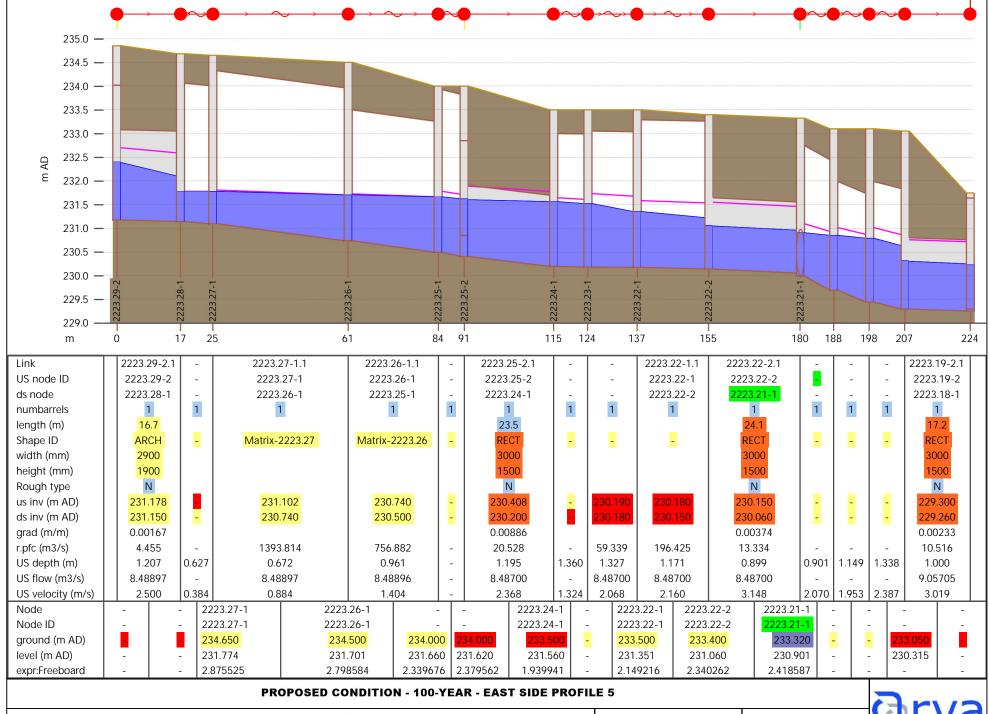
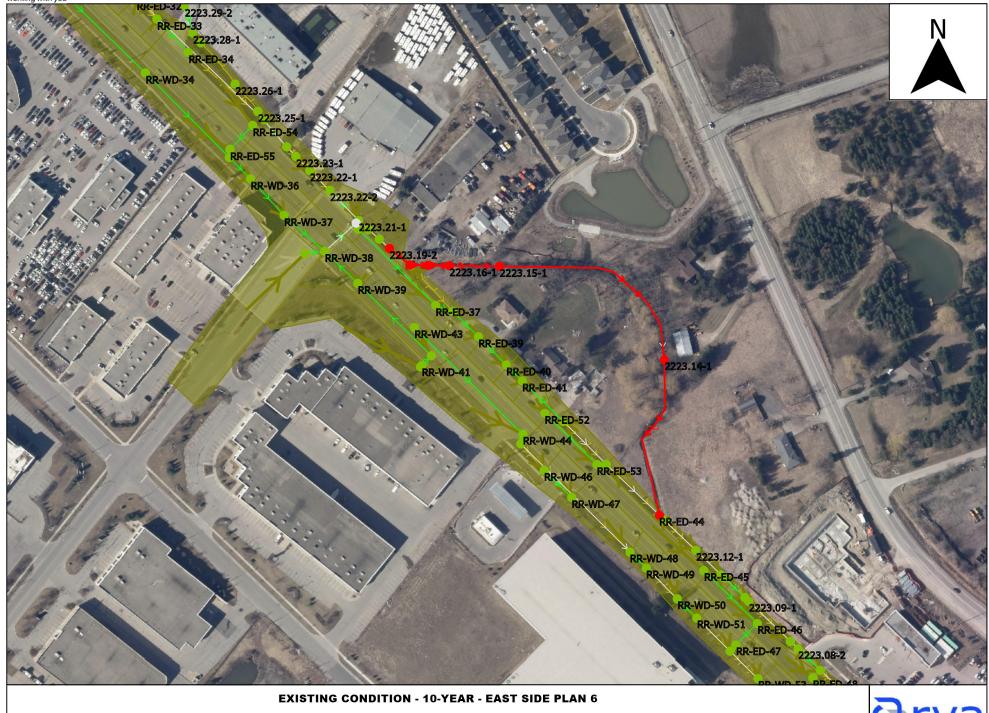


FIGURE NO. 25G AUGUST 2021





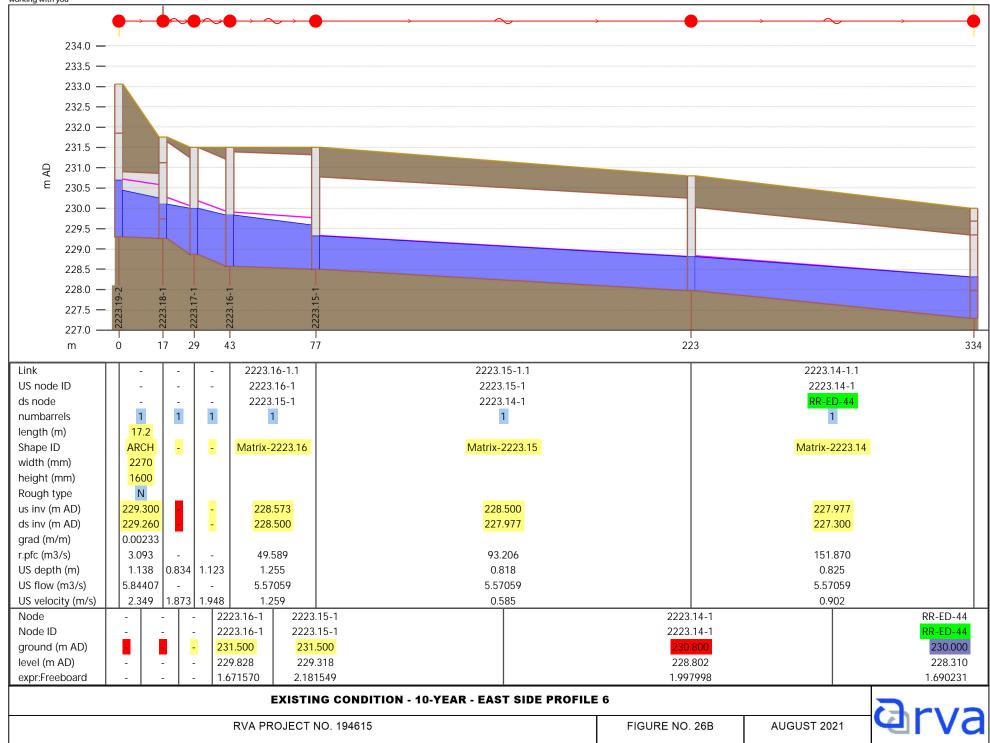




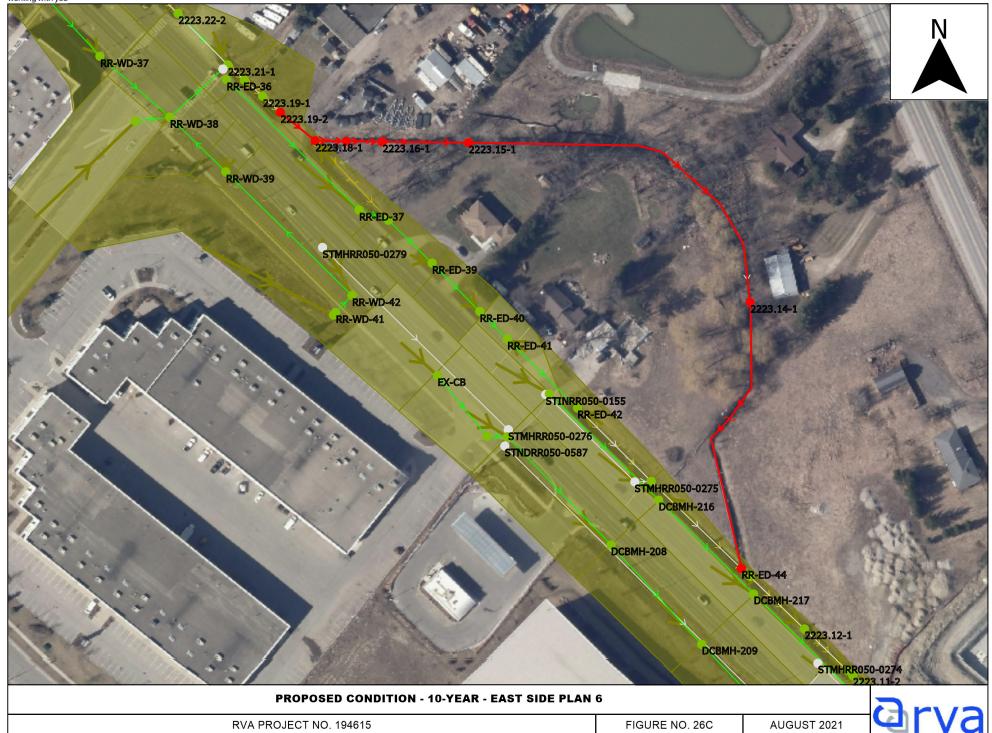
AUGUST 2021

FIGURE NO. 26A

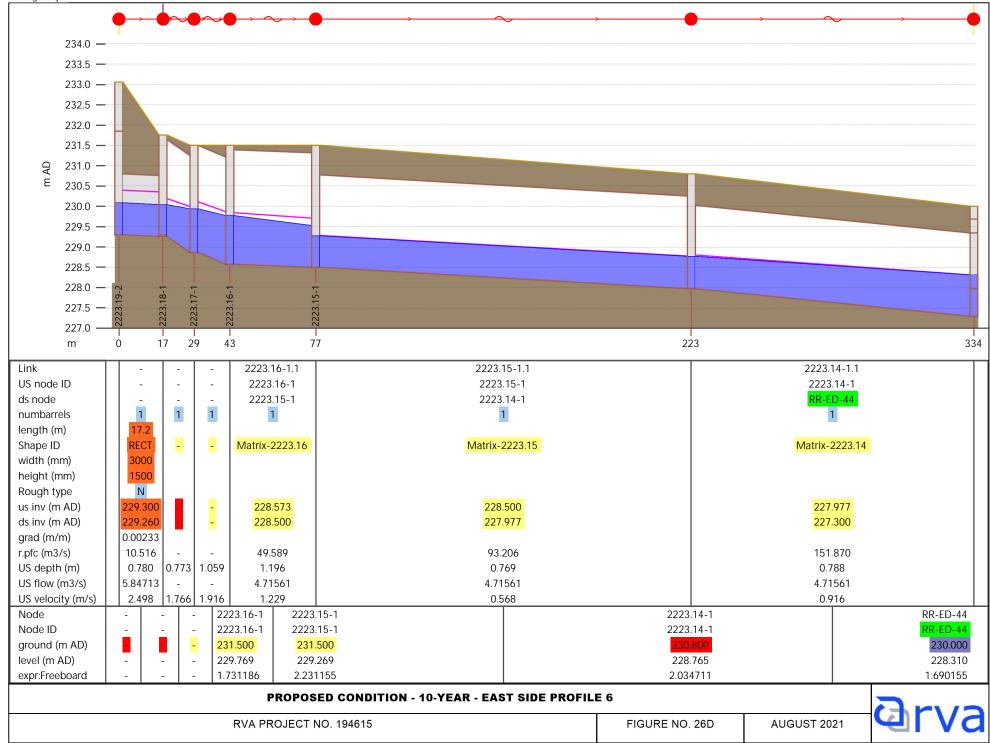














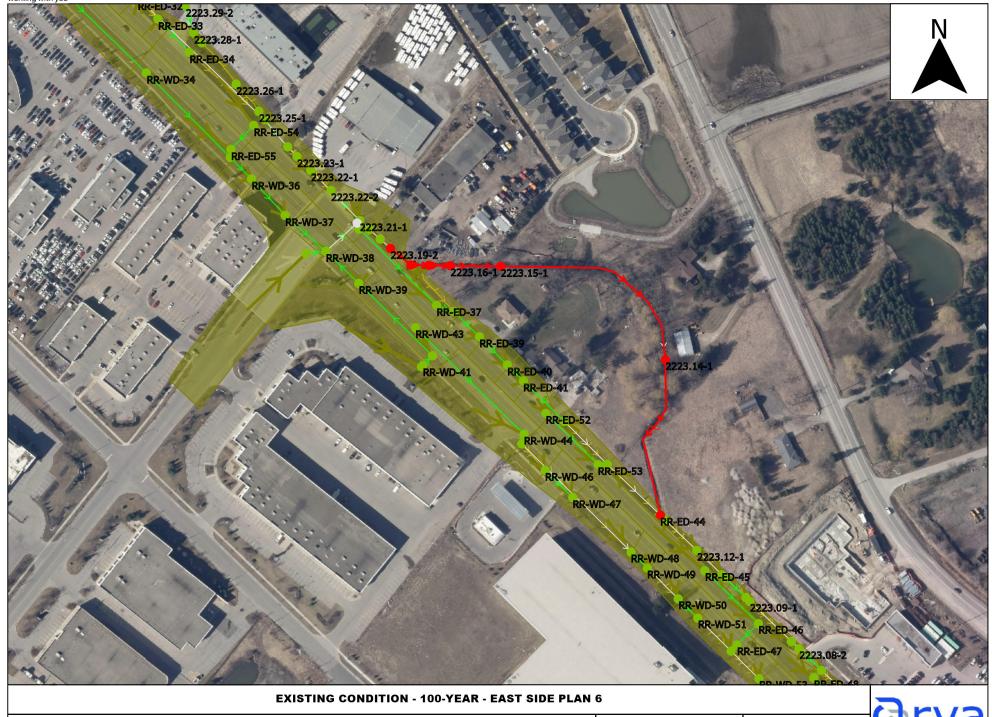
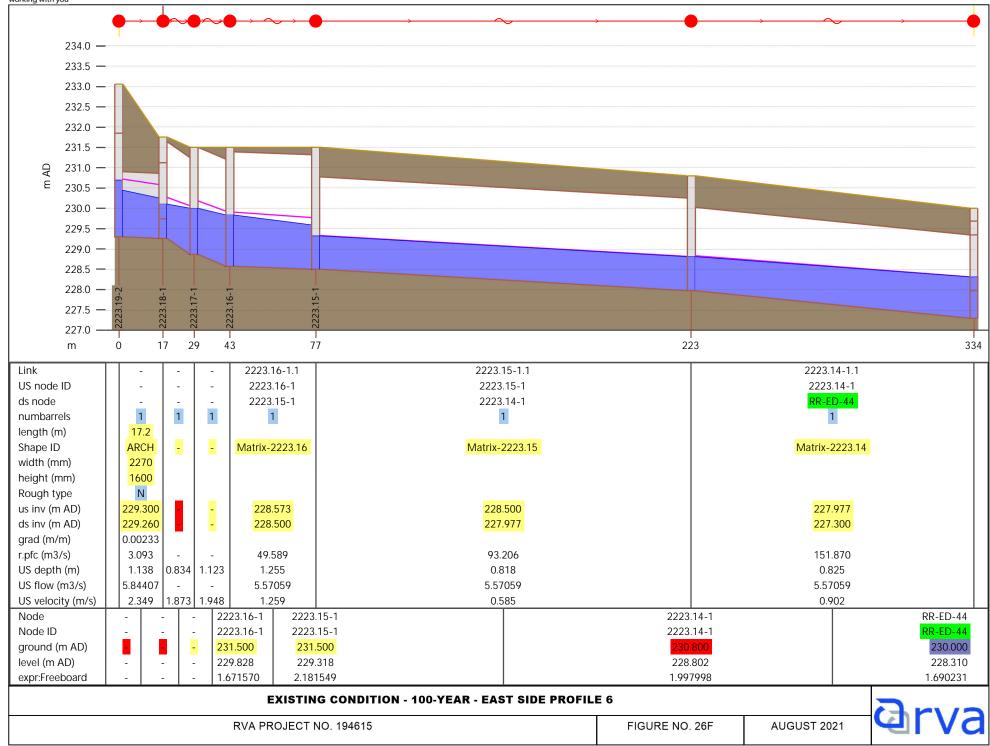
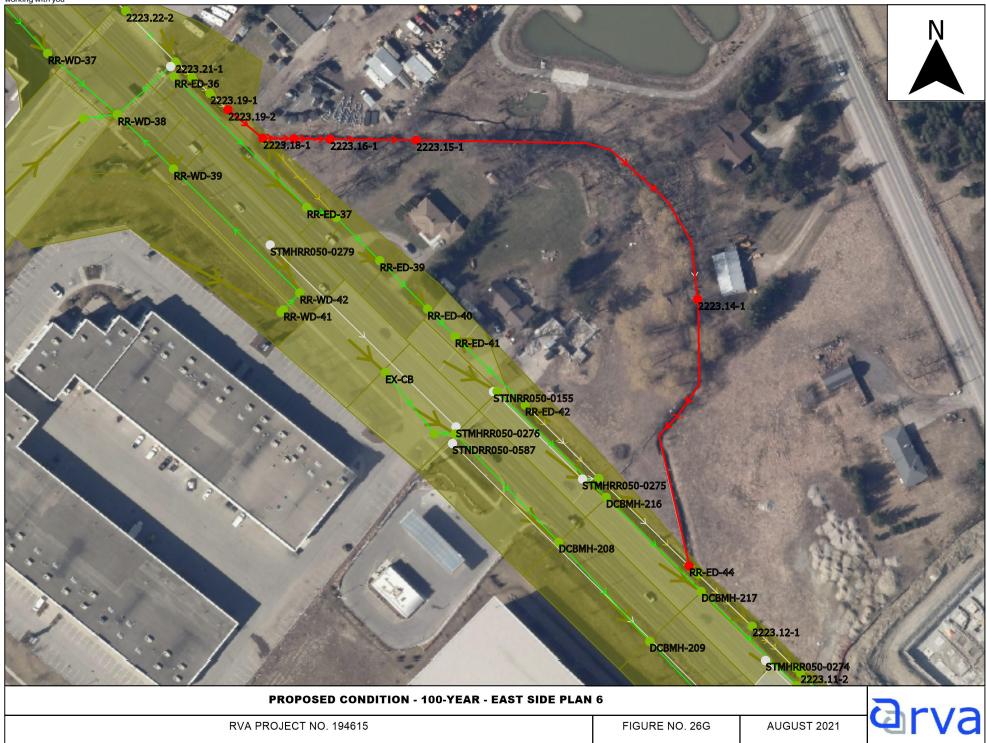


FIGURE NO. 26E

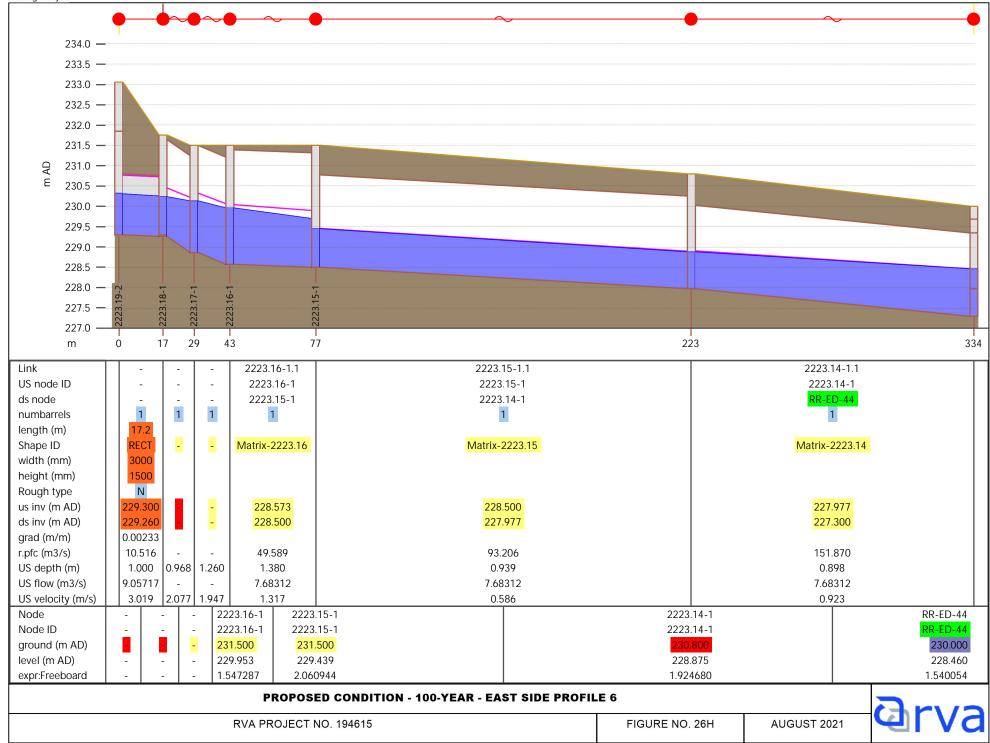




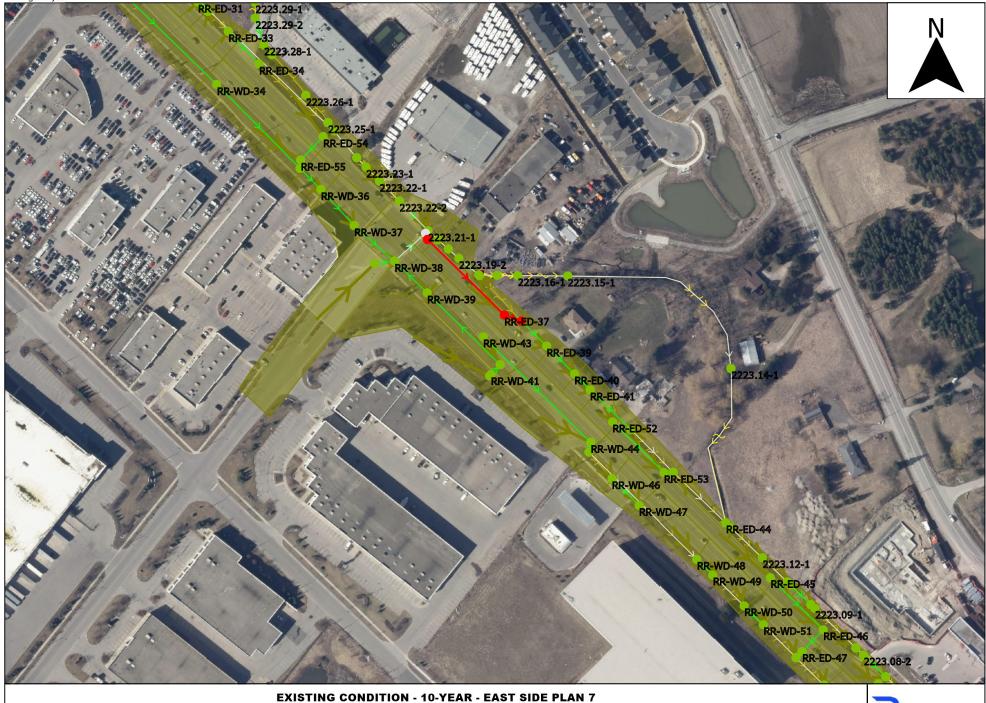






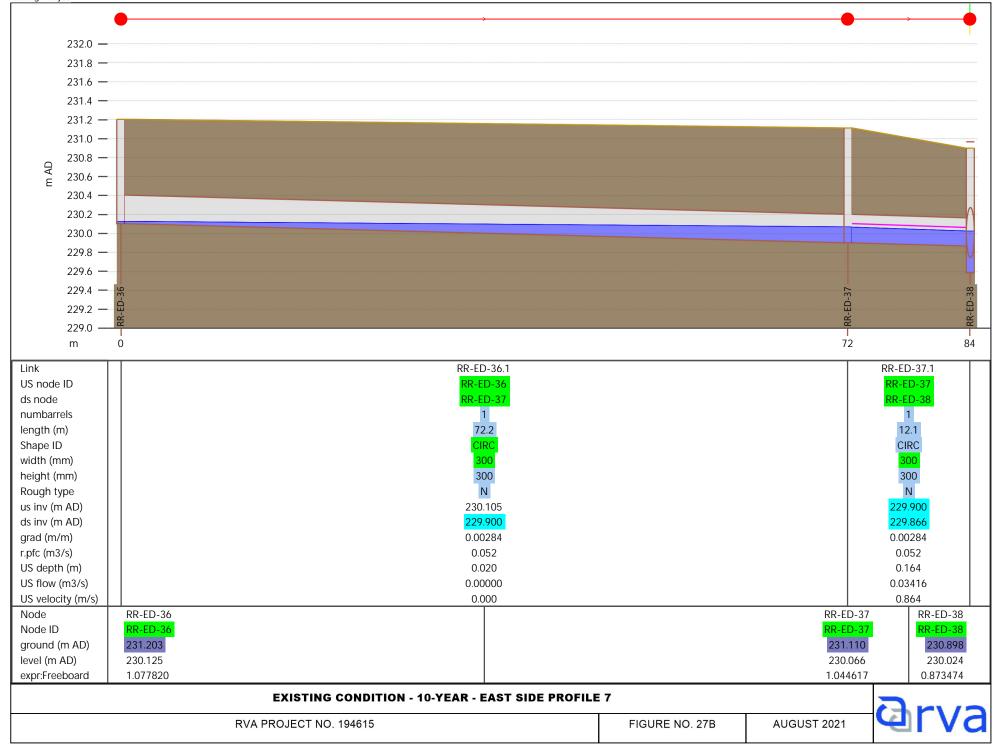




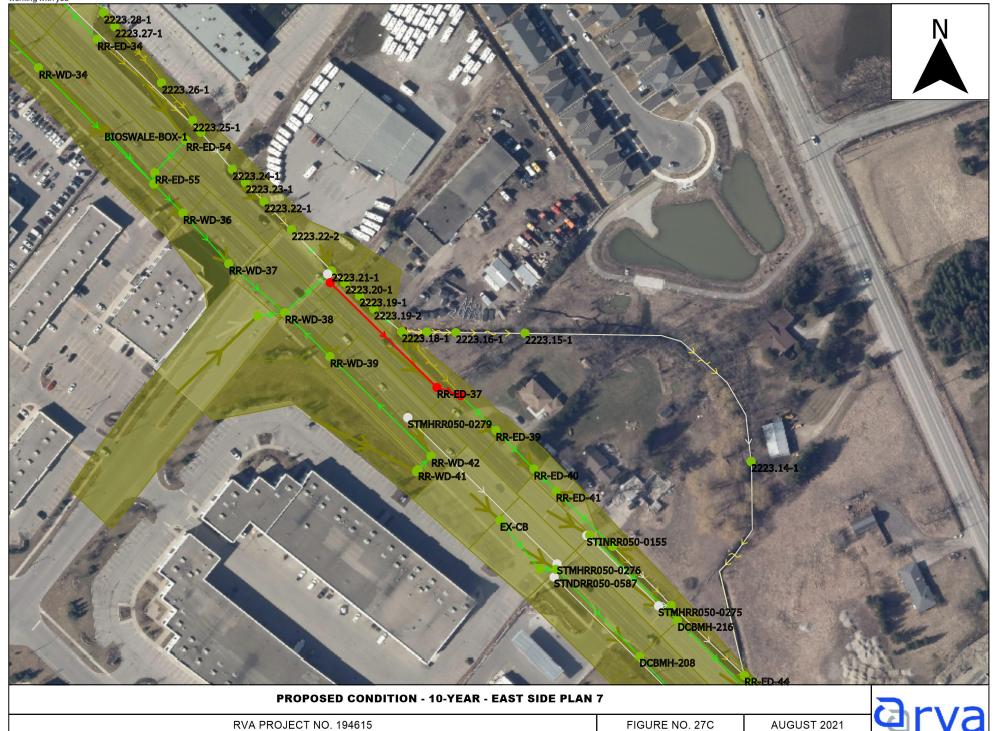




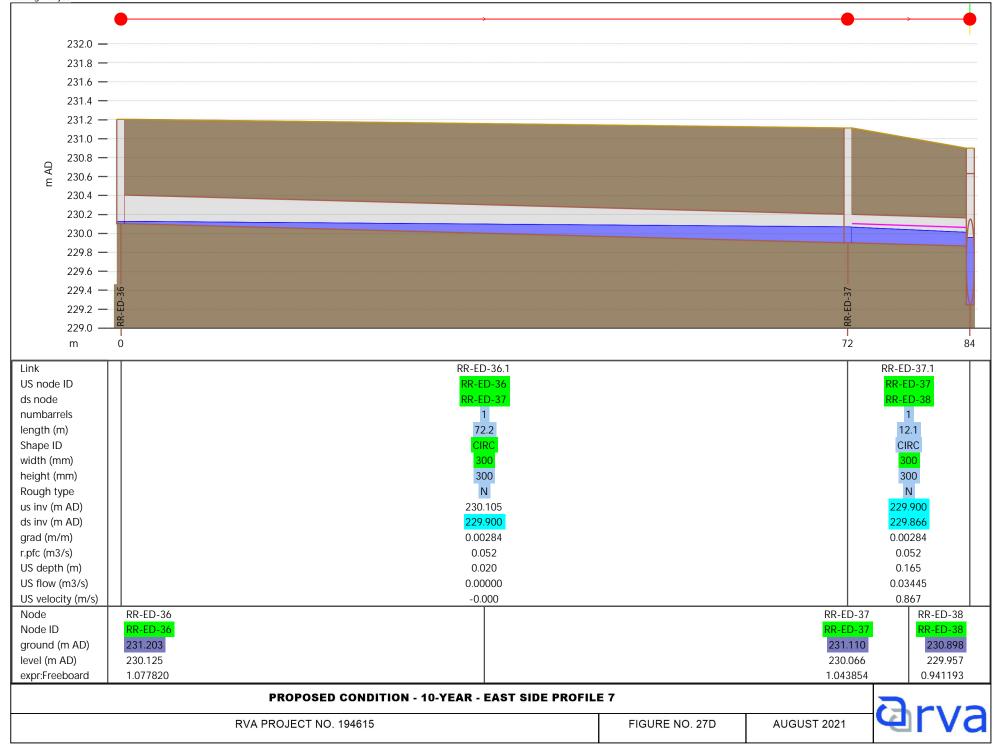














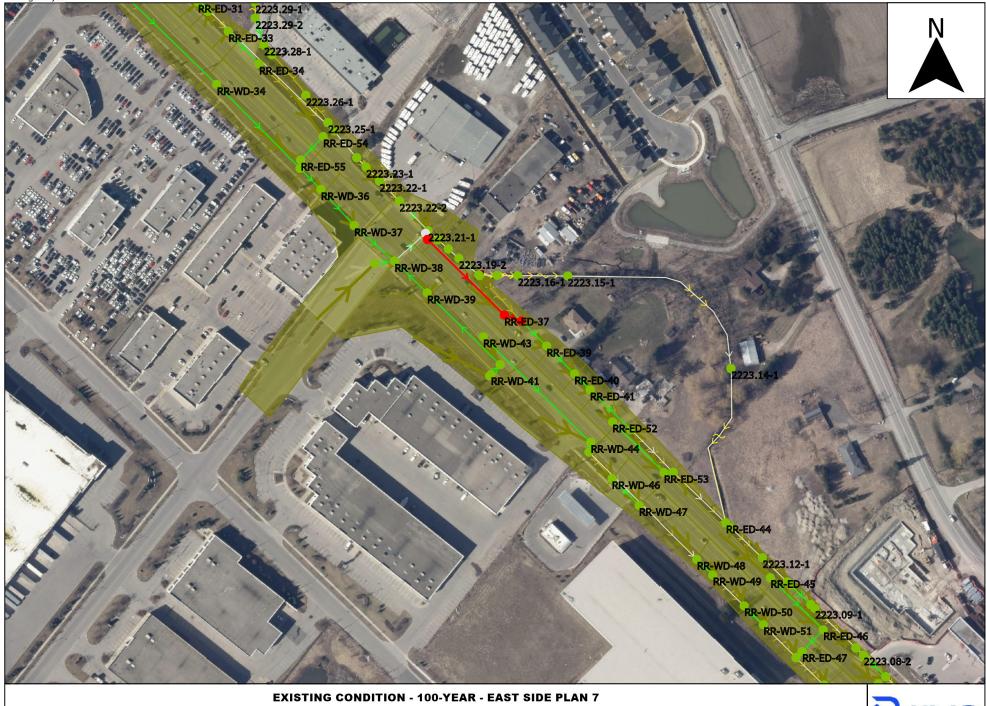
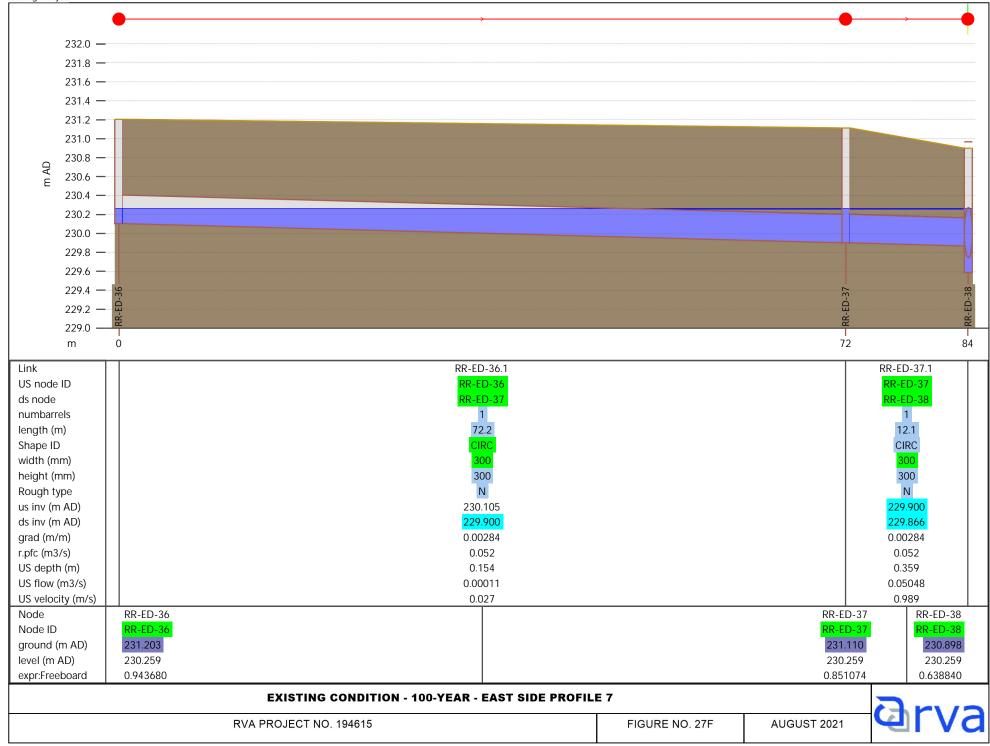
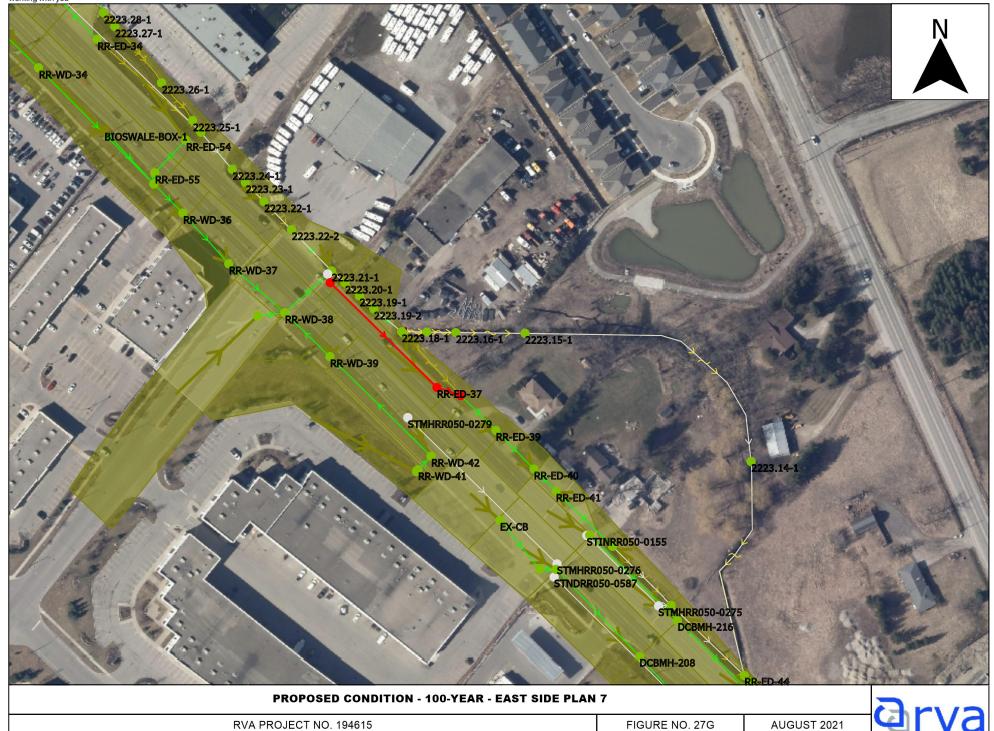


FIGURE NO. 27E AUGUST 2021

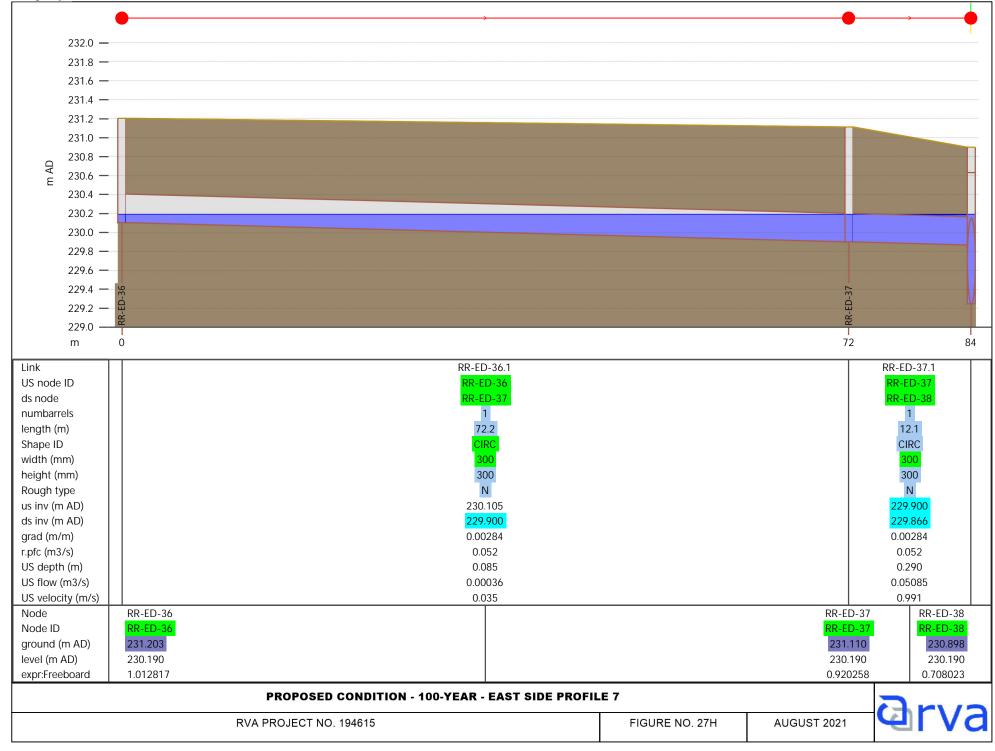




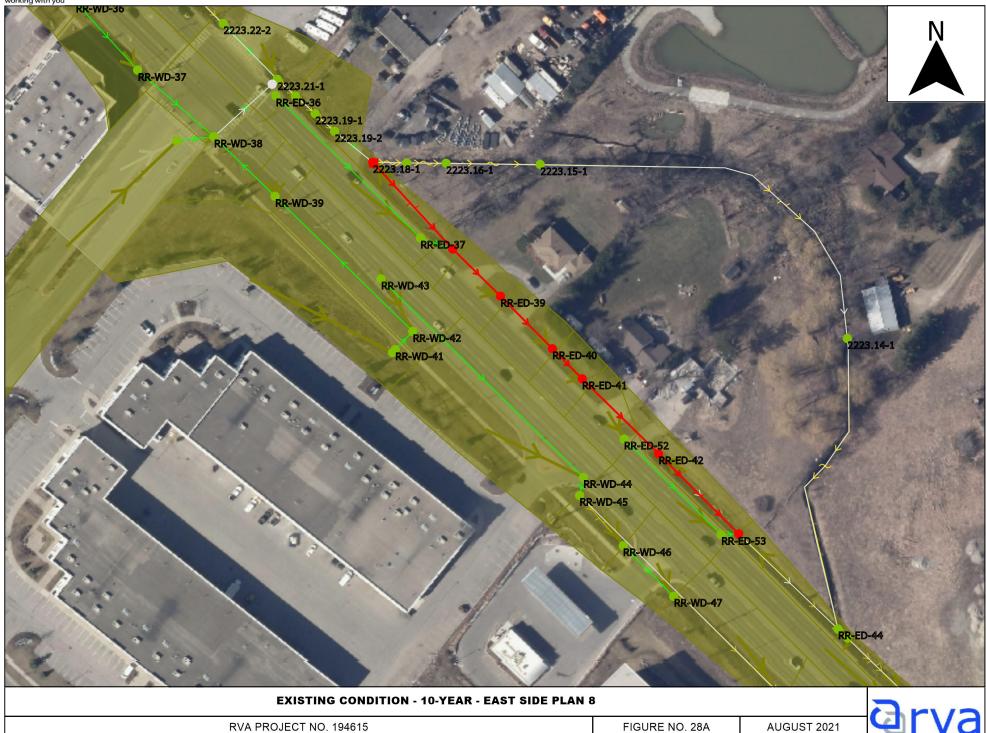






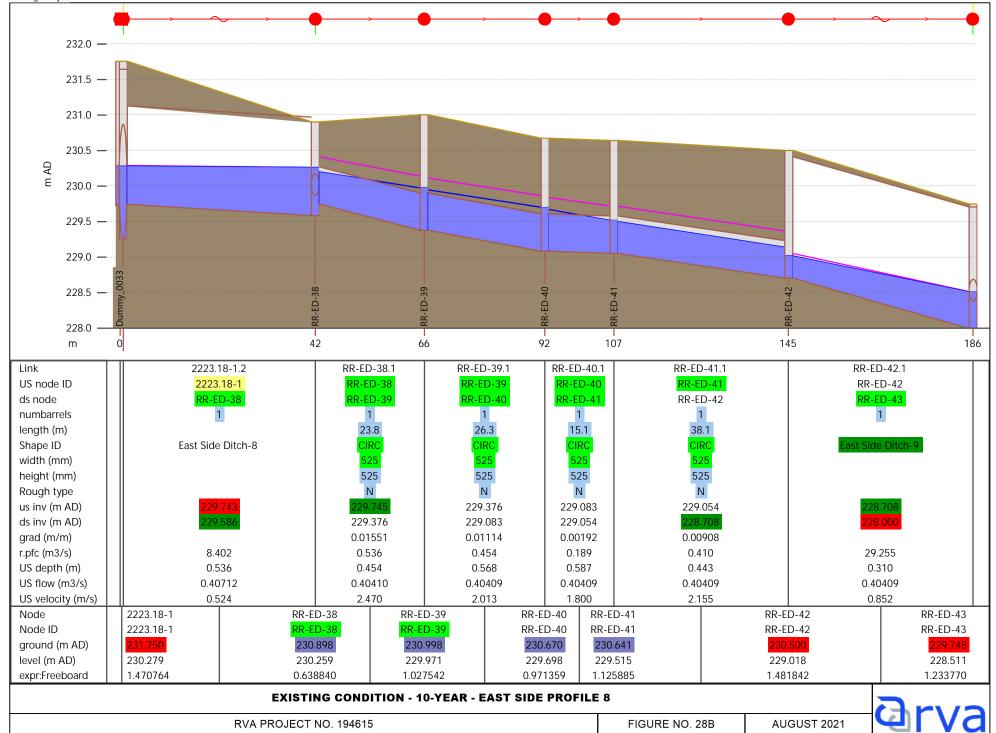




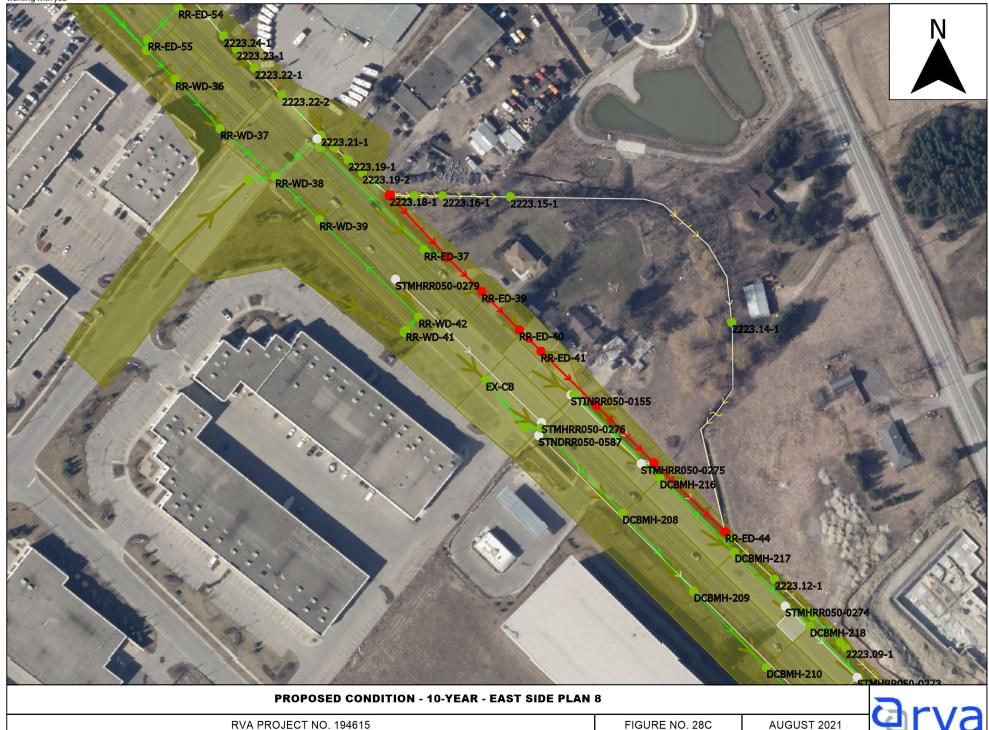


AUGUST 2021 FIGURE NO. 28A

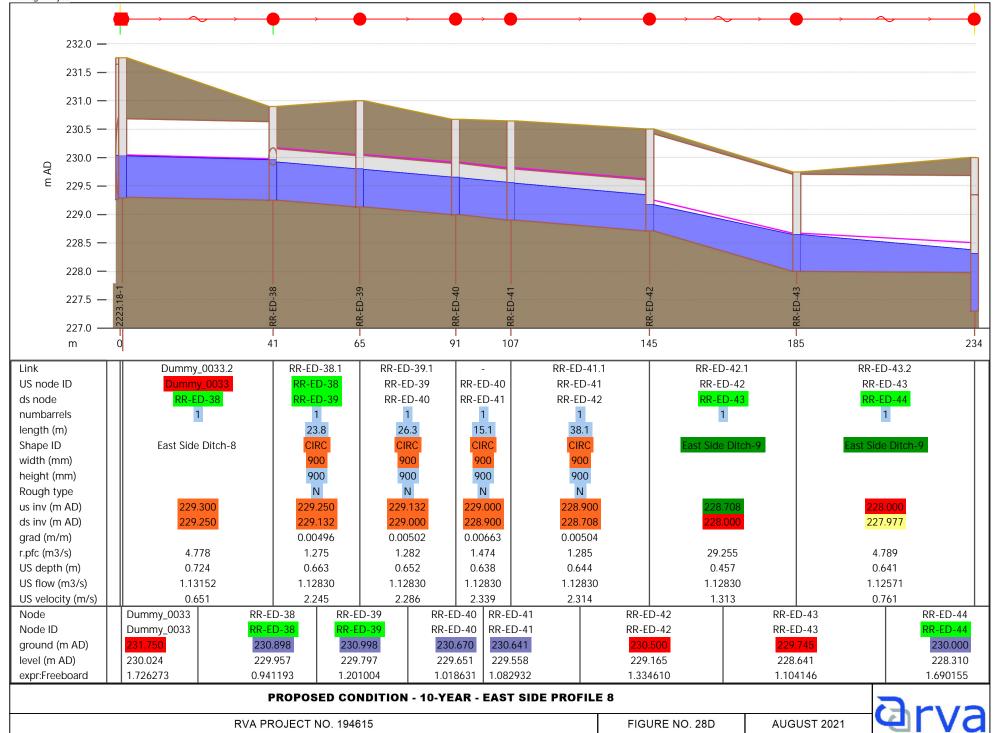




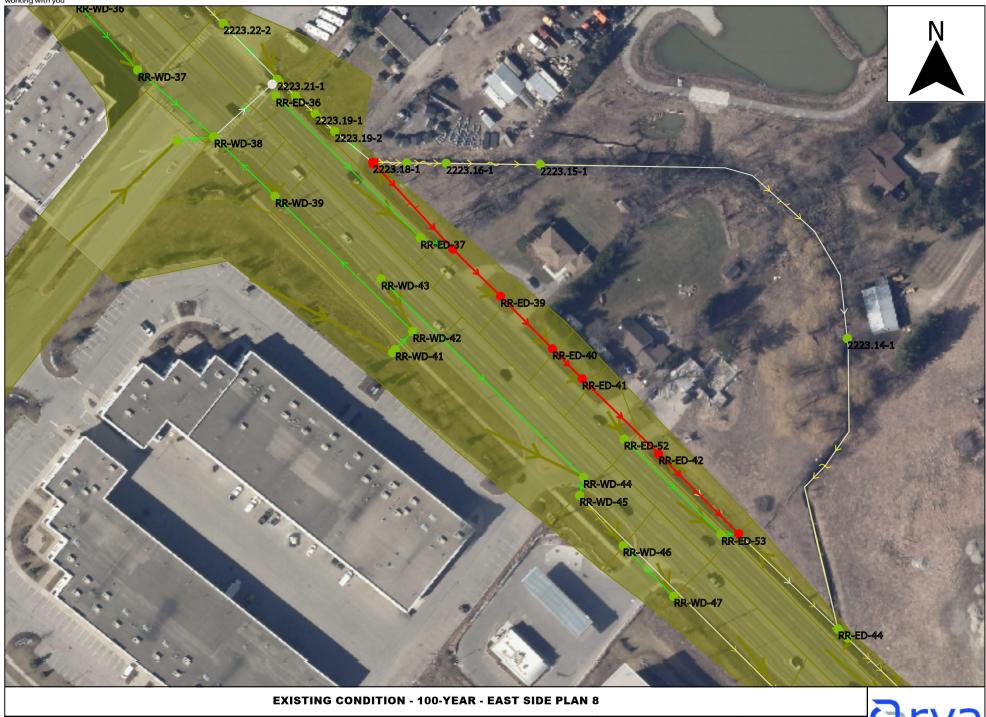












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FIGURE NO. 28E



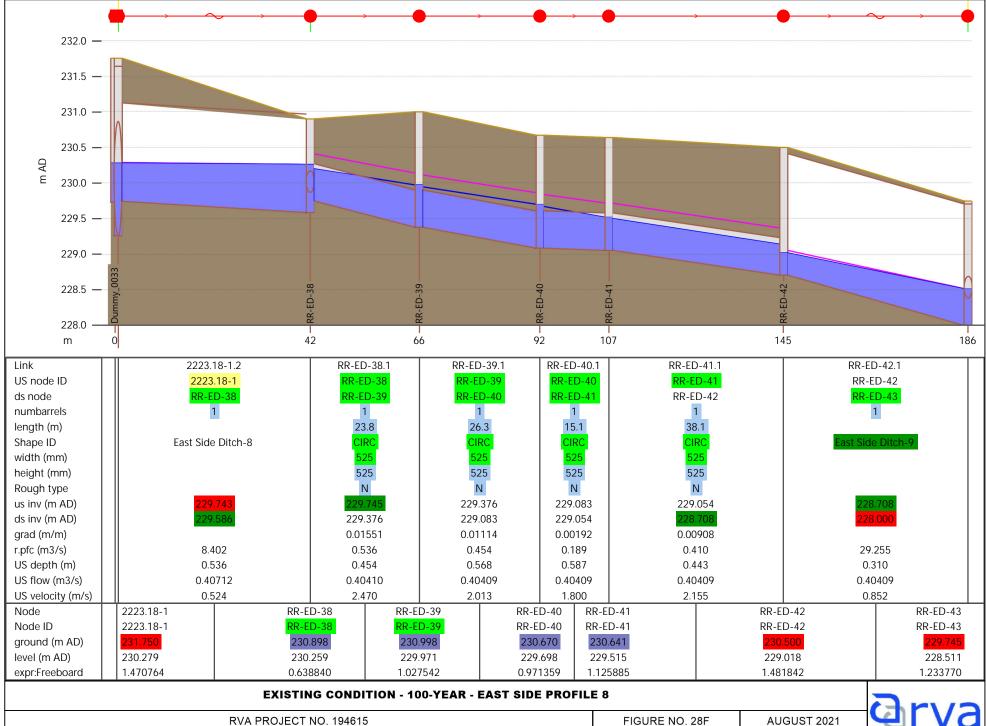


FIGURE NO. 28F AUGUST 2021



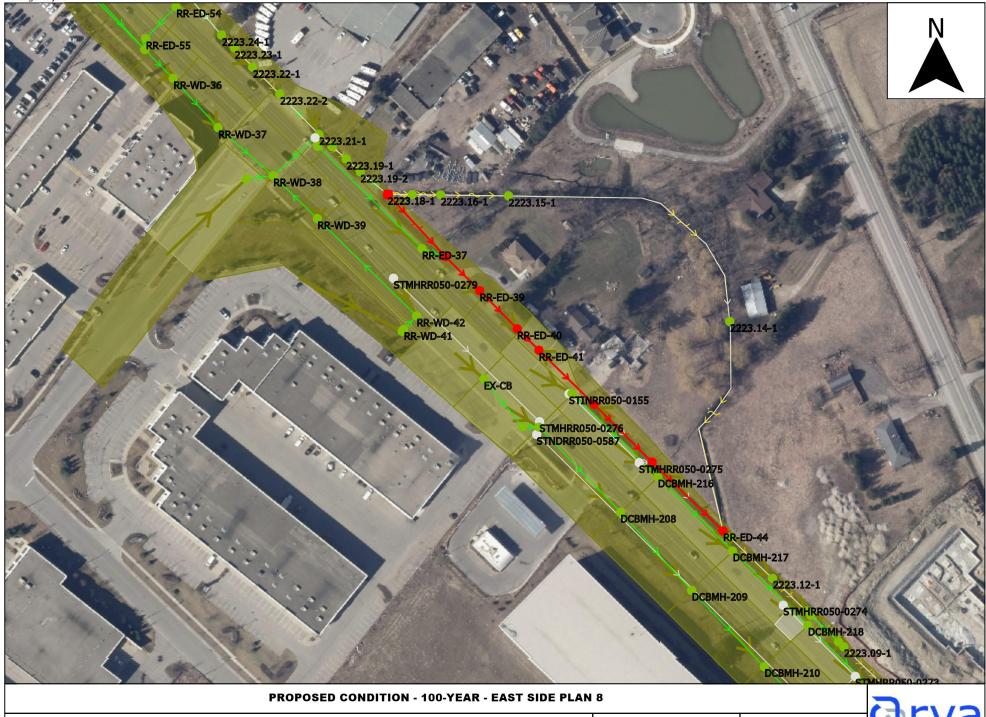
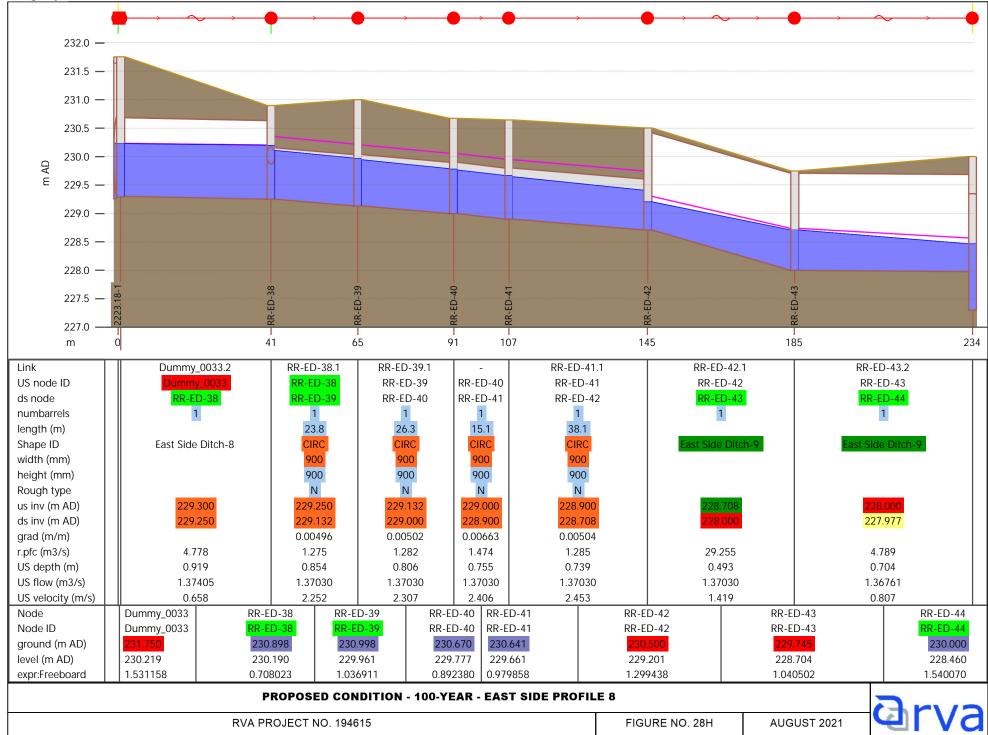
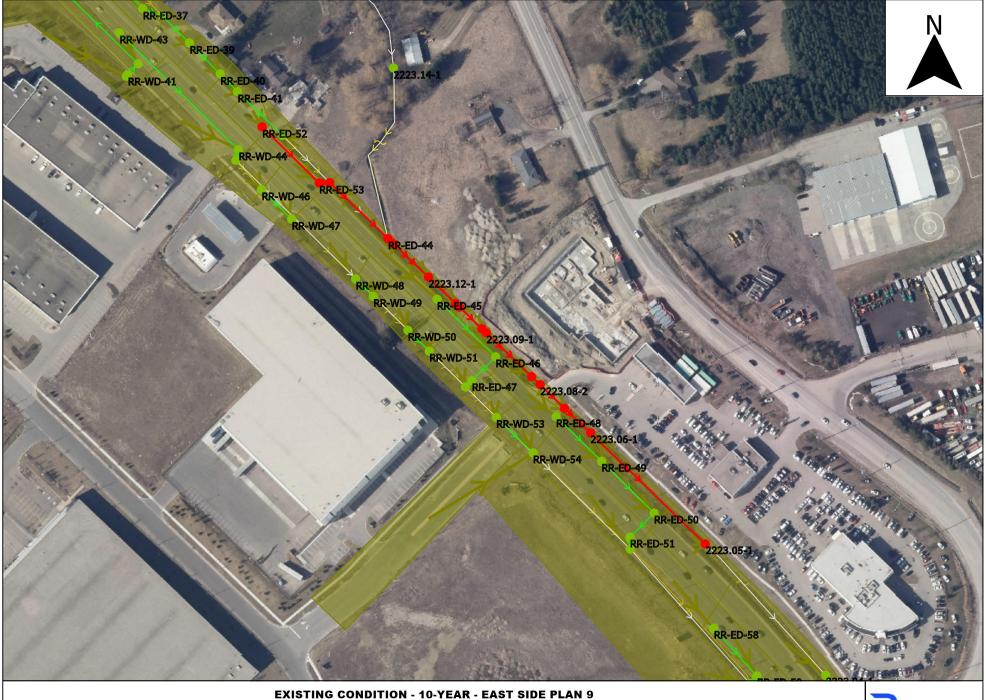


FIGURE NO. 28G AUGUST 2021









EXISTING CONDITION - TO-TEAK - EAST SIDE TEAK S

RVA PROJECT NO. 194615

FIGURE NO. 29A





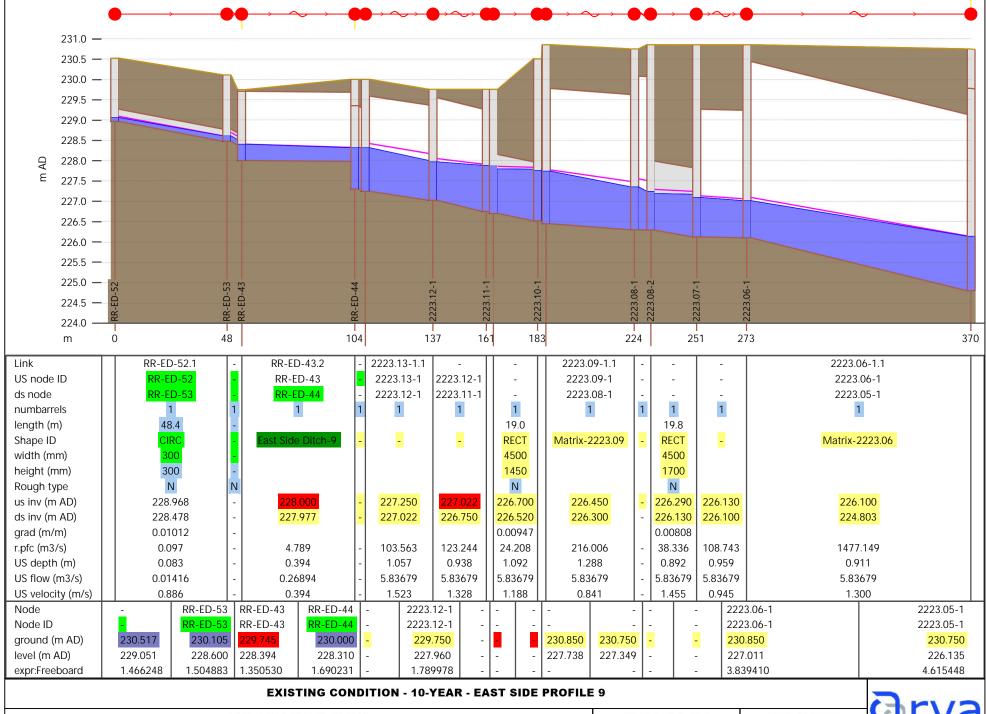
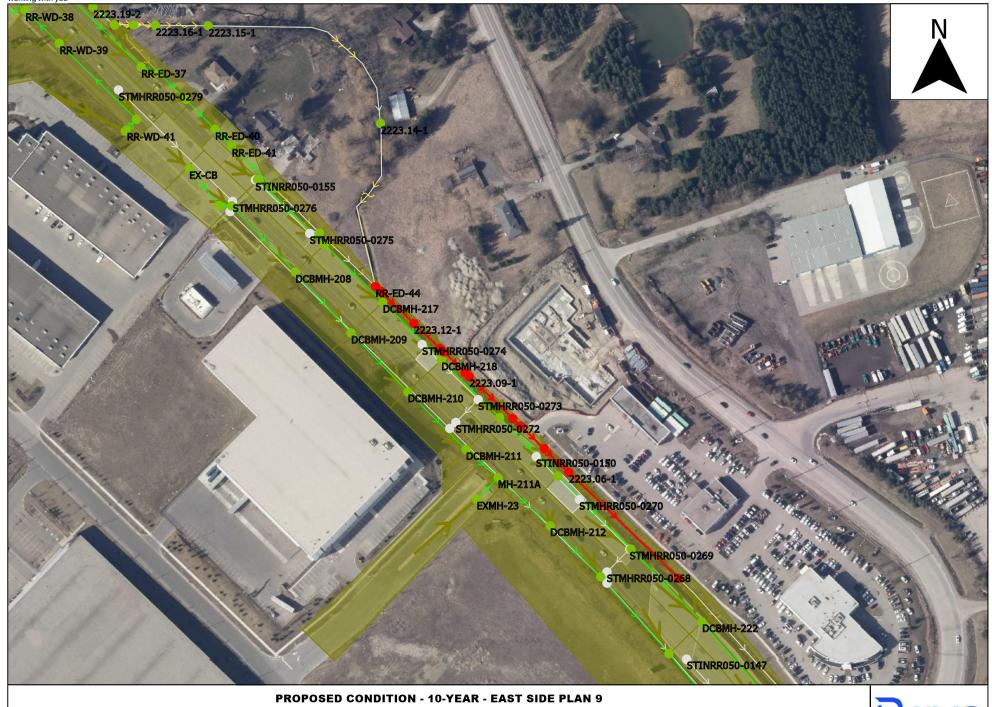


FIGURE NO. 29B







RVA PROJECT NO. 194615

AUGUST 2021

FIGURE NO. 29C



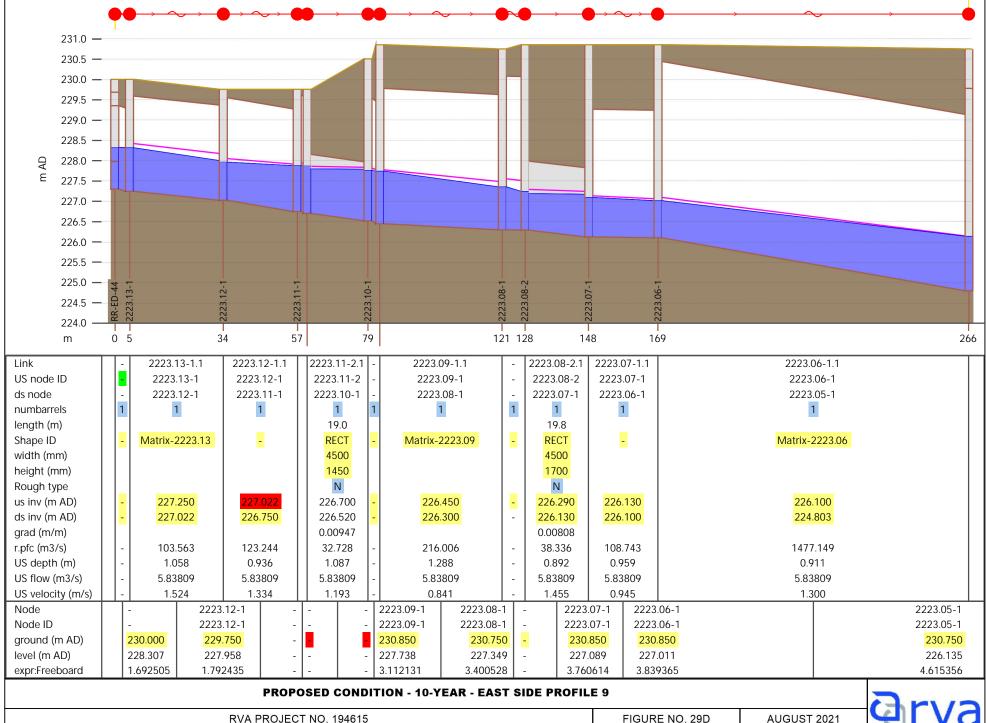
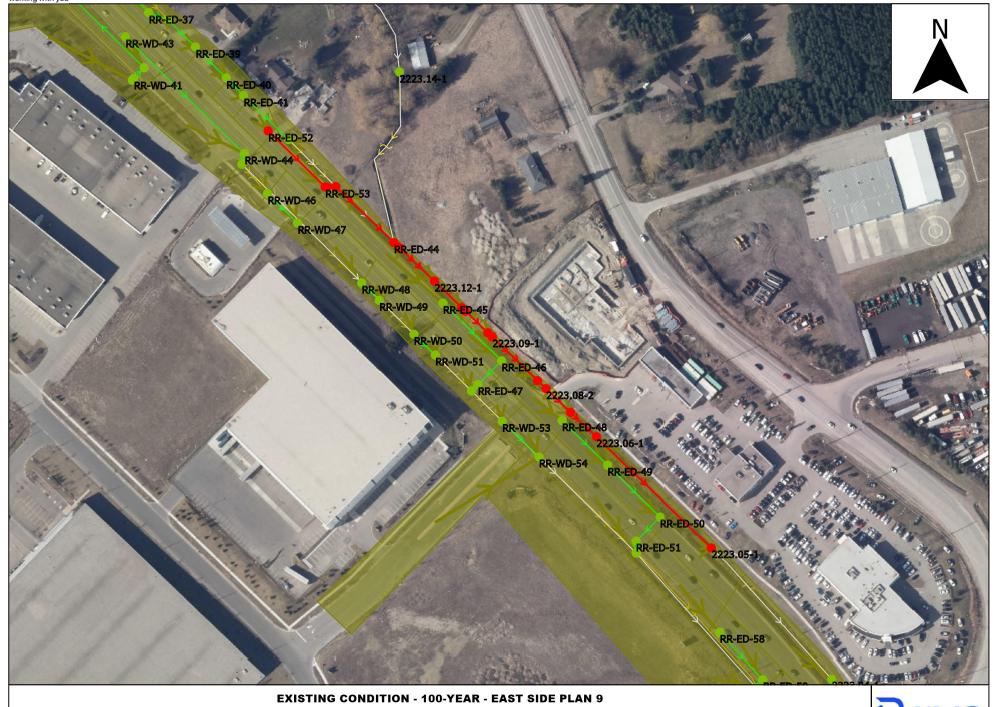


FIGURE NO. 29D AUGUST 2021

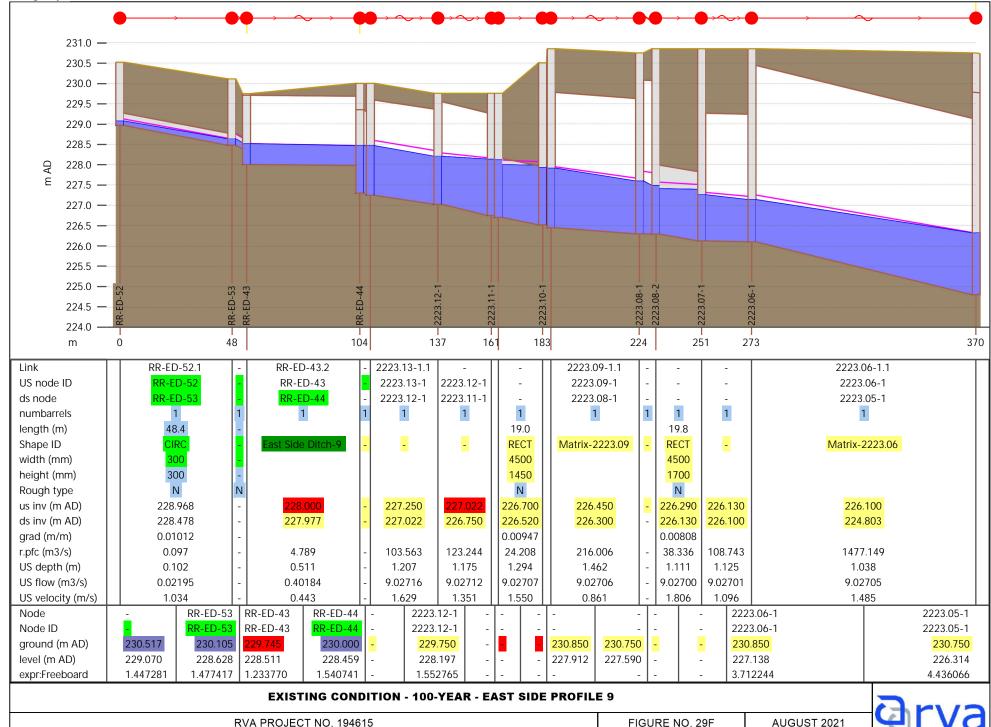




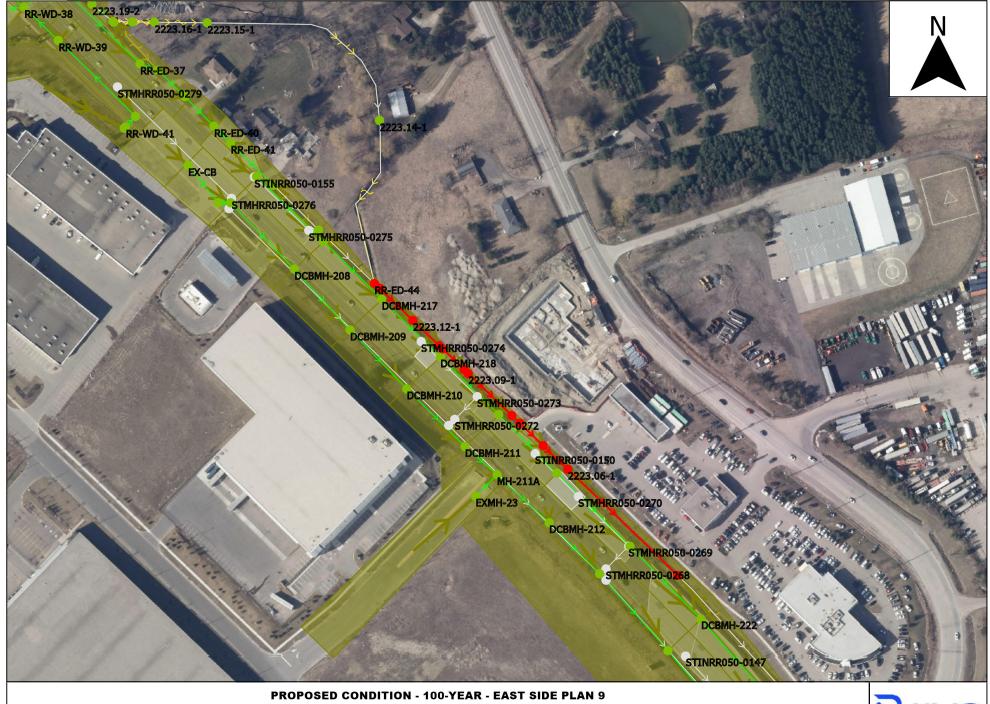
RVA PROJECT NO. 194615

FIGURE NO. 29E









AUGUST 2021 FIGURE NO. 29G



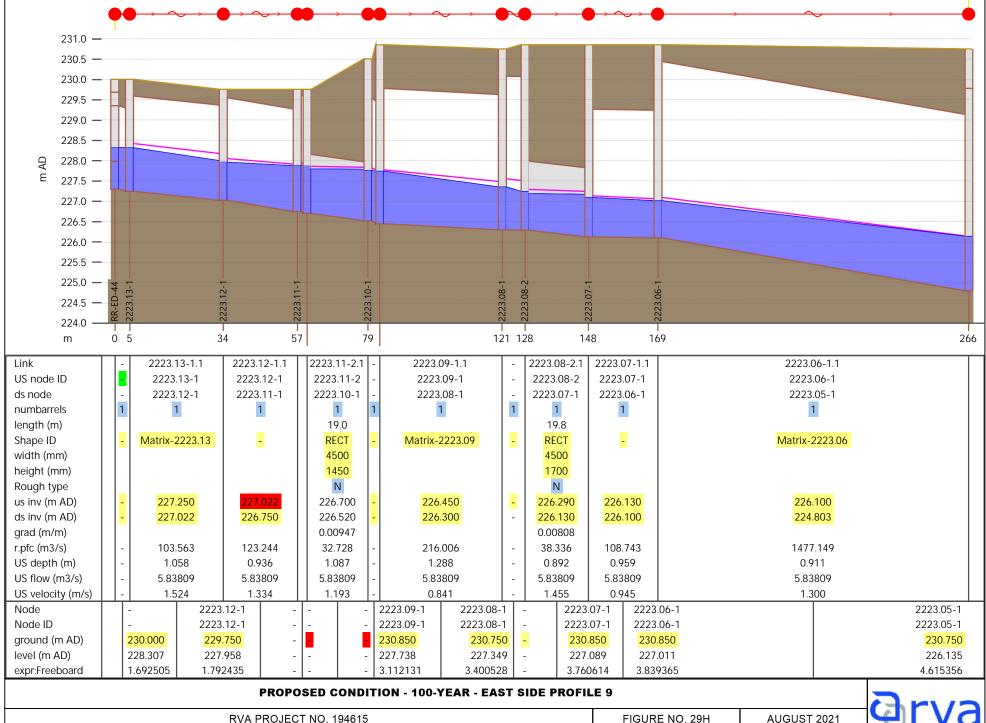
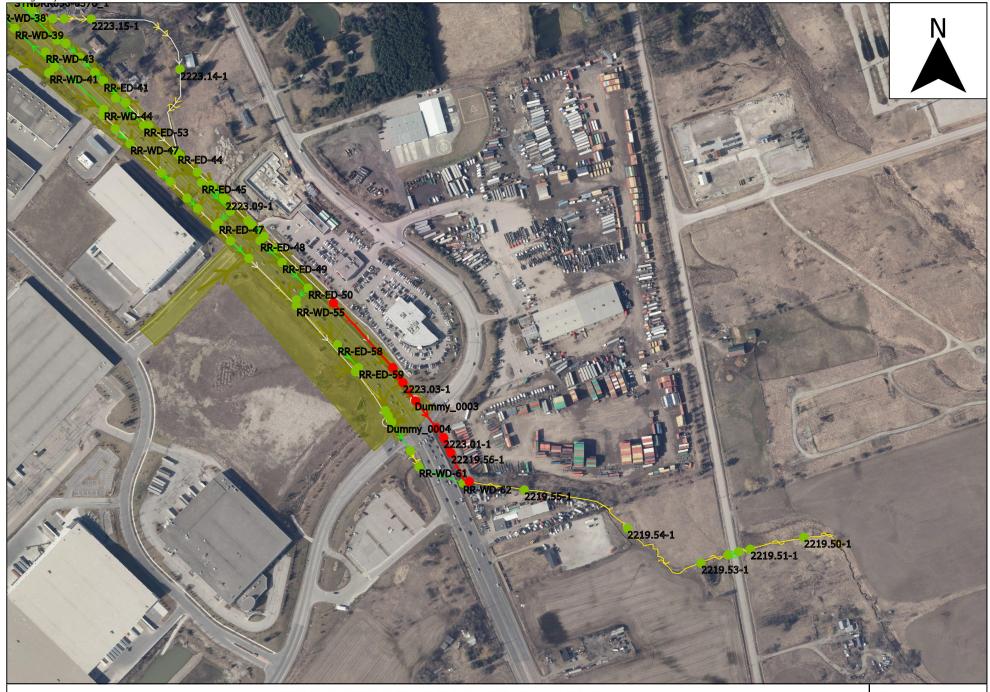


FIGURE NO. 29H AUGUST 2021





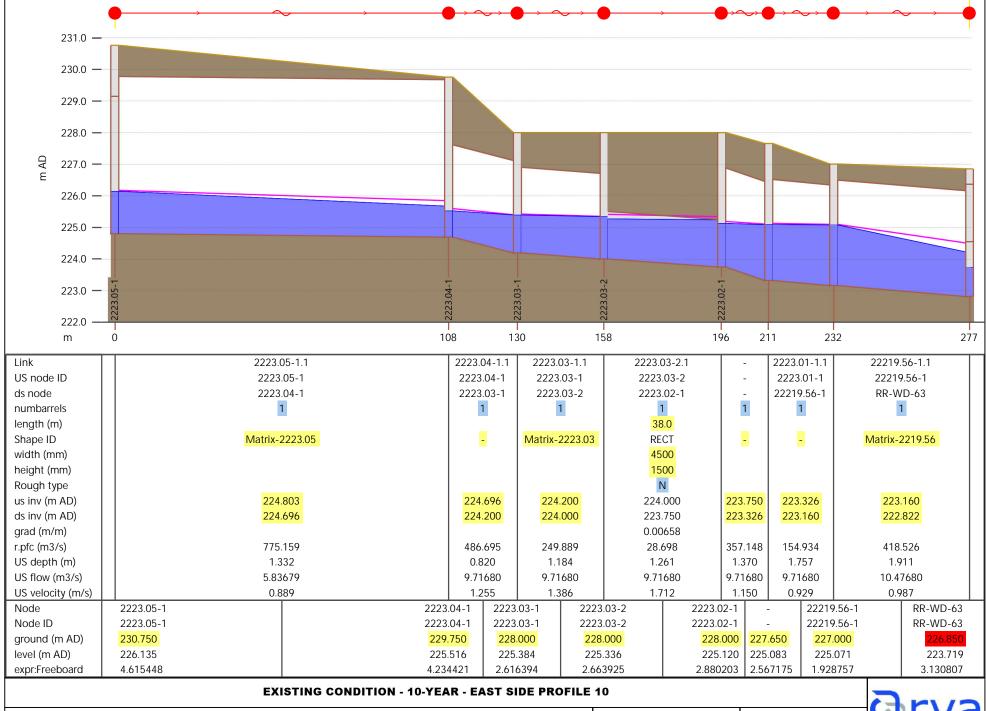


RVA PROJECT NO. 194615

FIGURE NO. 30A



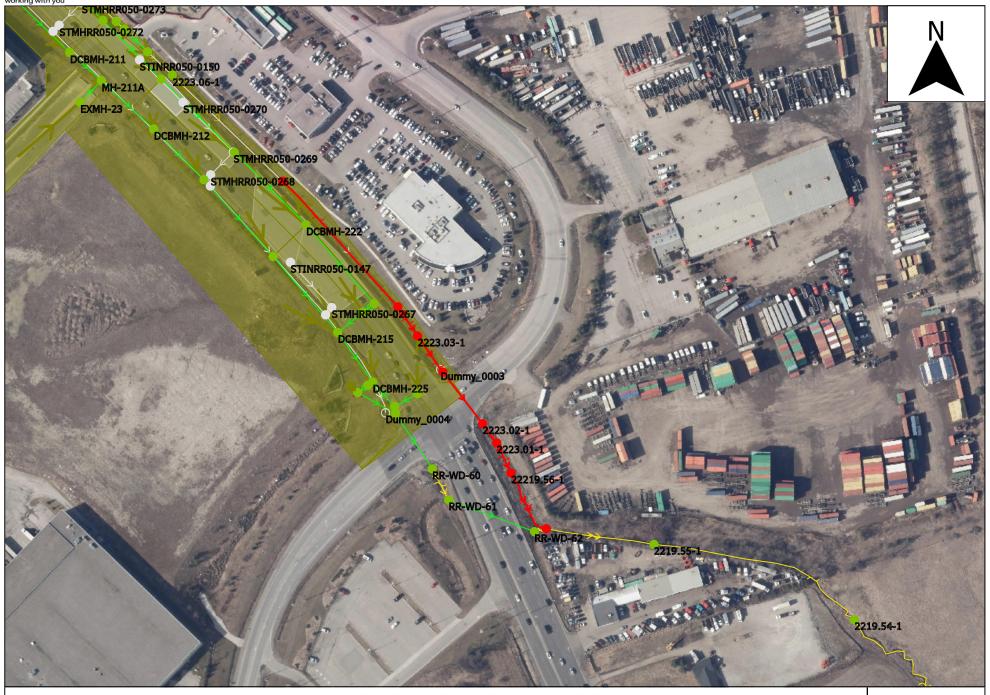




RVA PROJECT NO. 194615

FIGURE NO. 30B AUGUST 2021



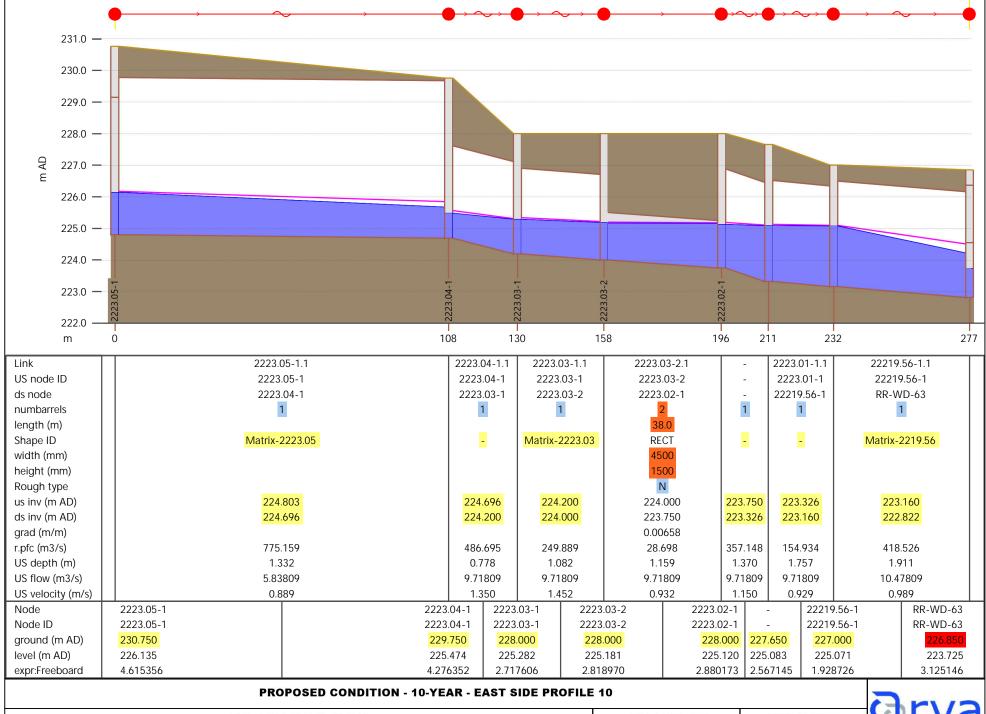


PROPOSED CONDITION - 10-YEAR - EAST SIDE PLAN 10

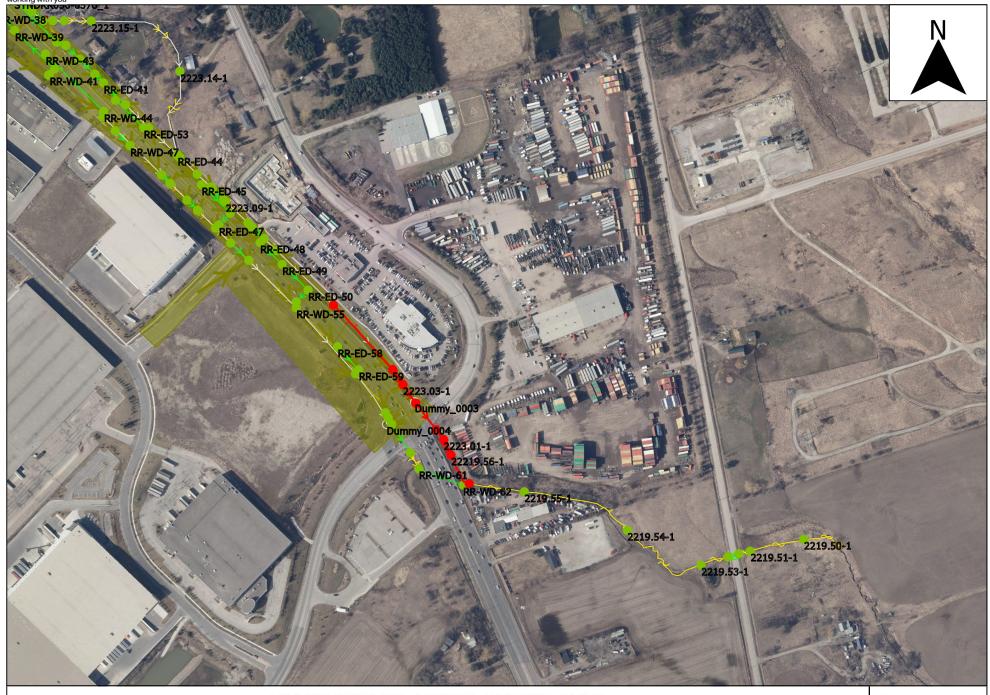
FIGURE NO. 30C











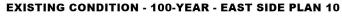
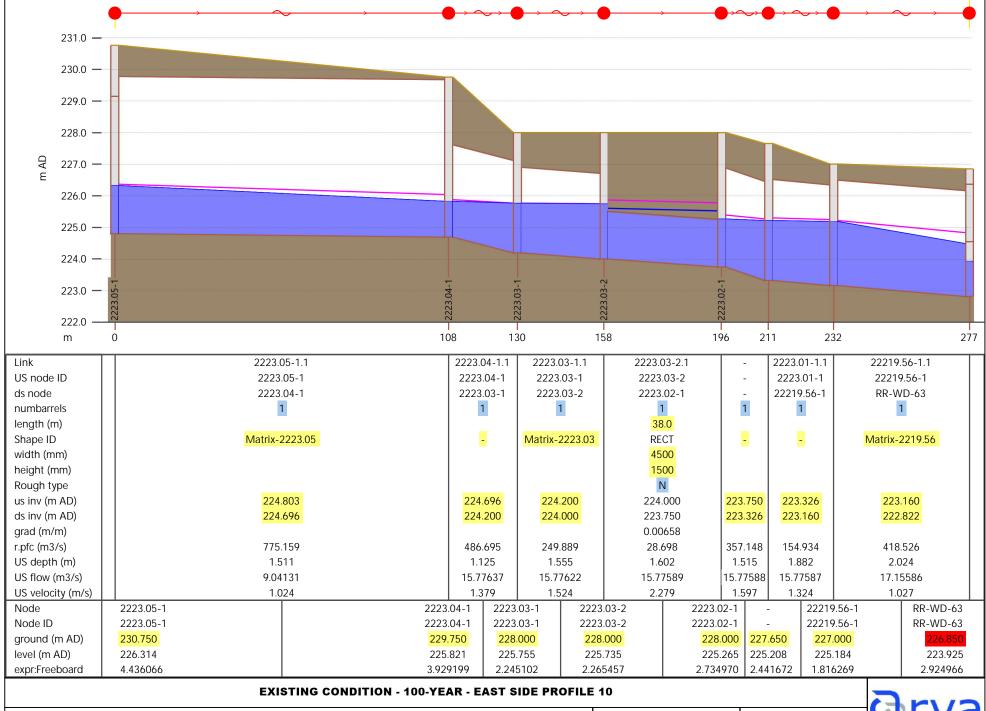


FIGURE NO. 30E



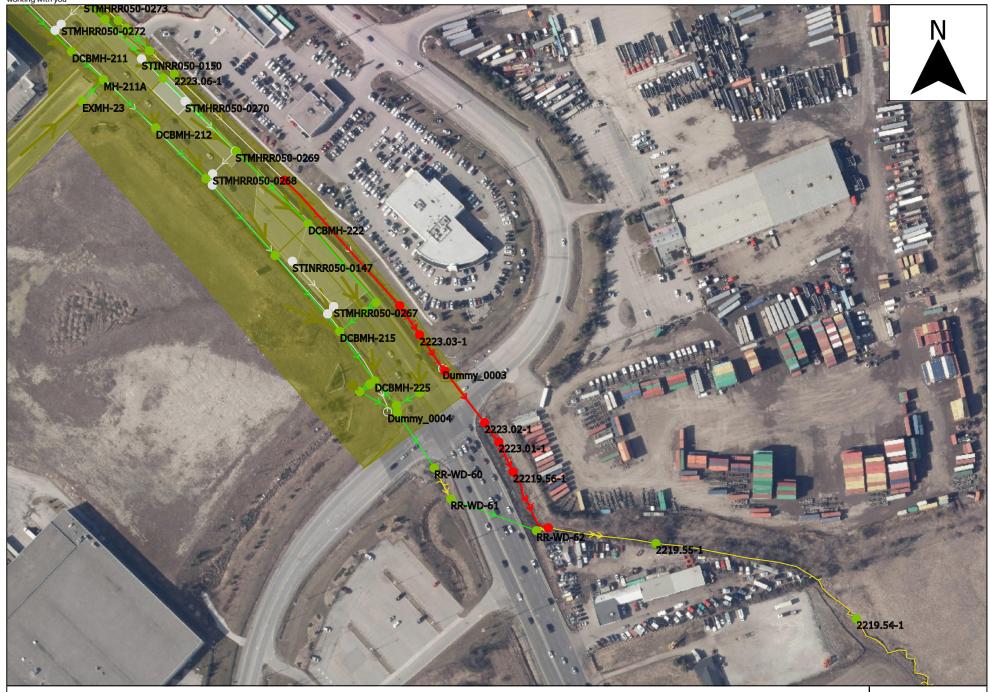


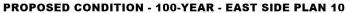


RVA PROJECT NO. 194615

FIGURE NO. 30F AUGUST 2021



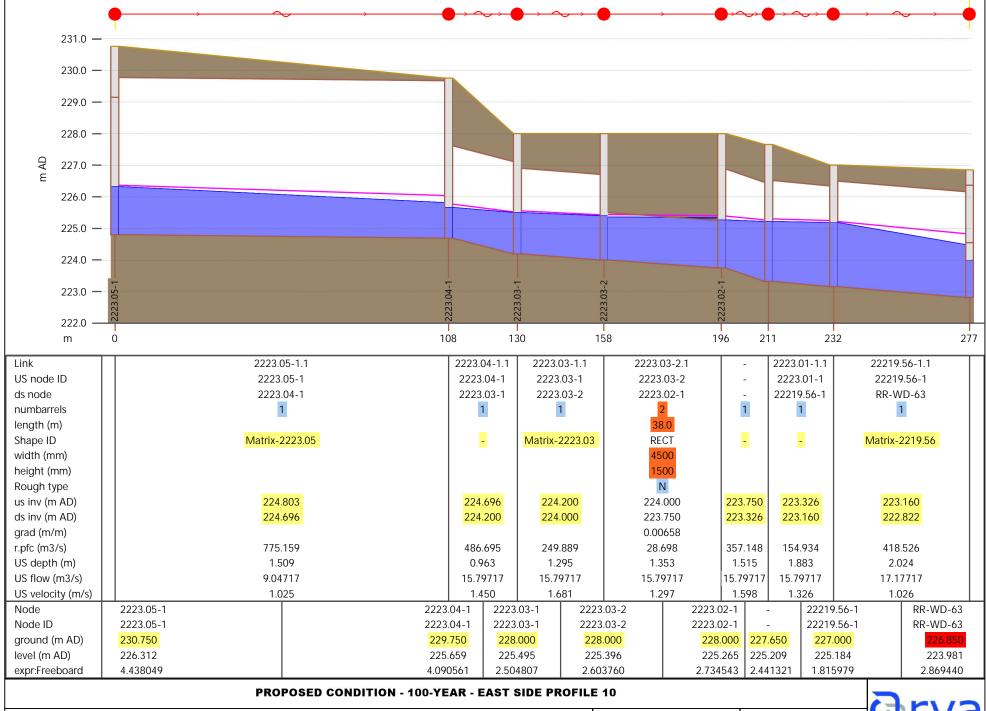




RVA PROJECT NO. 194615 FIGURE NO. 30G

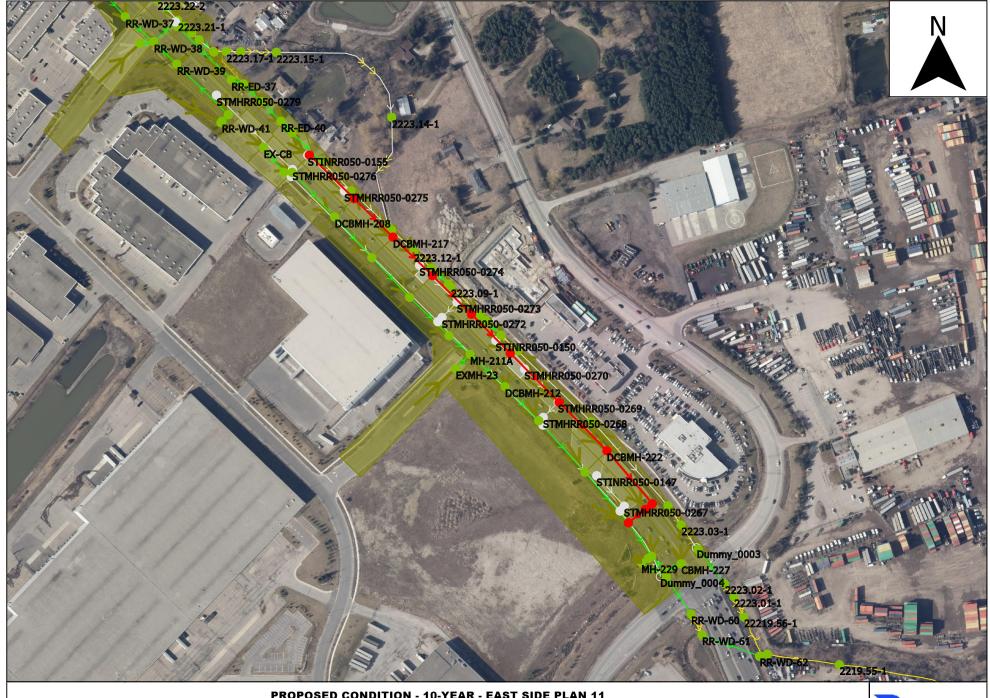












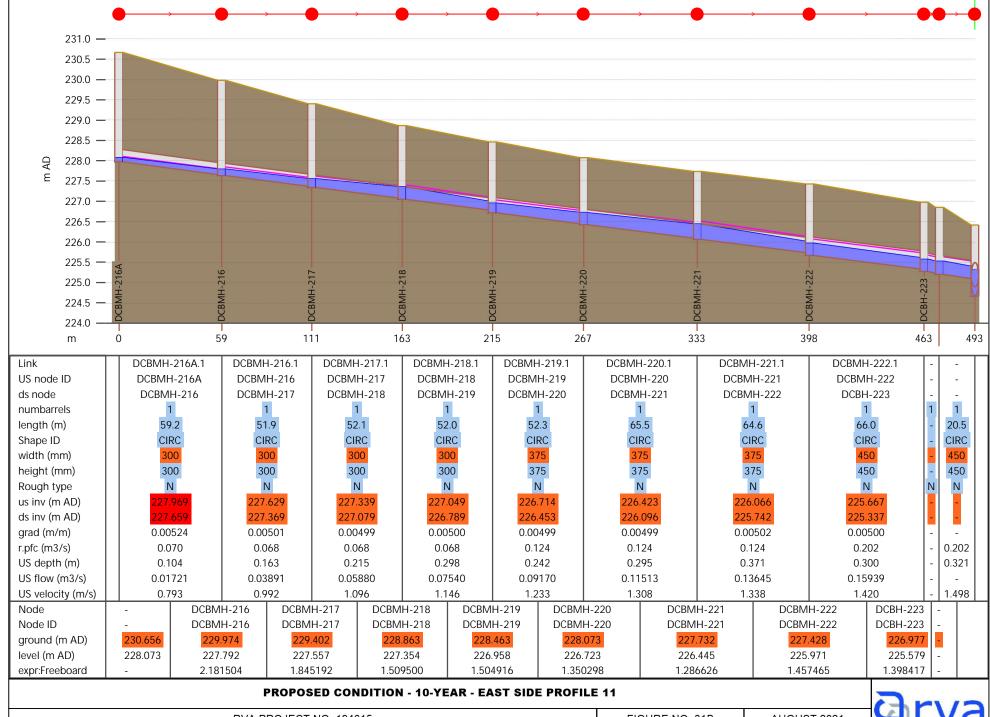
PROPOSED CONDITION - 10-YEAR - EAST SIDE PLAN 11

RVA PROJECT NO. 194615

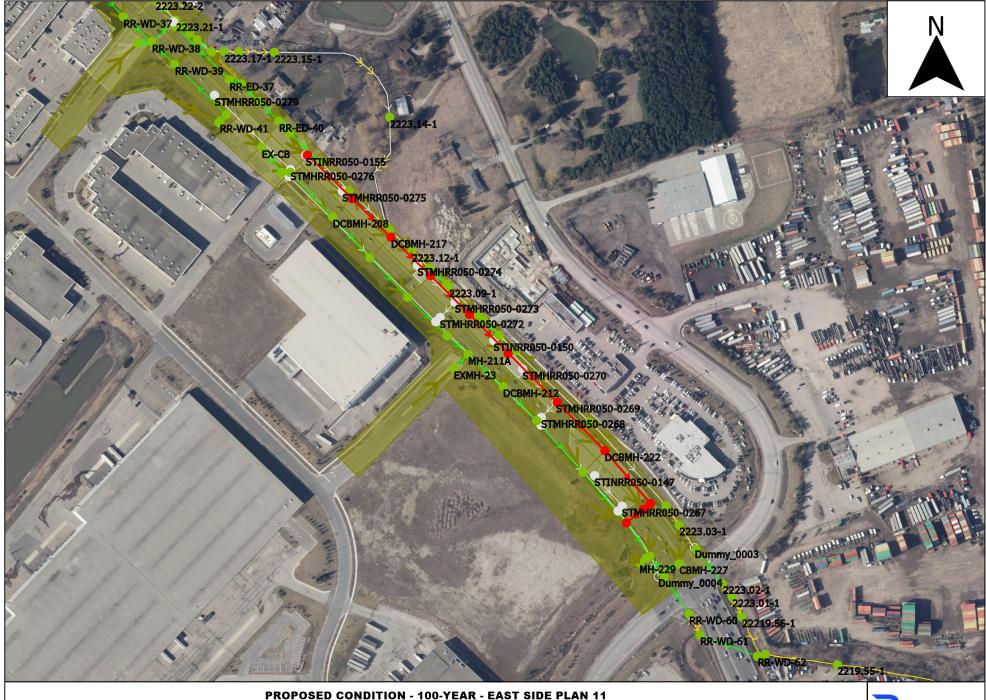
FIGURE NO. 31A











PROPOSED CONDITION - 100-YEAR - EAST SIDE PLAN 1

FIGURE NO. 31C





Appendix C

OGS & Catchbasin Shield Owner's Manual





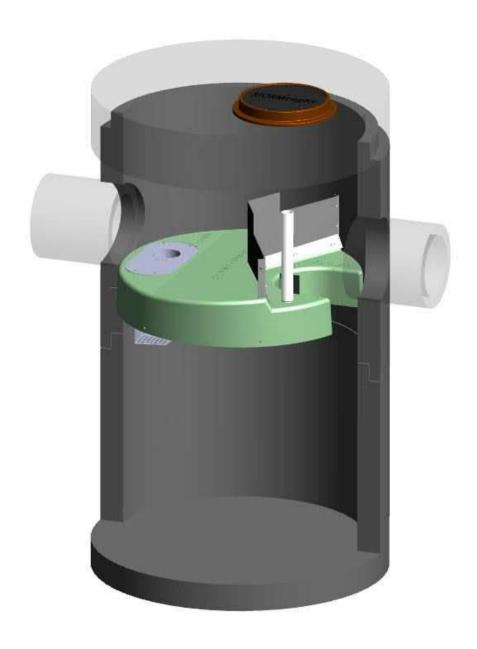






Stormceptor® EF

Owner's Manual





Stormceptor is protected by one or more of the following patents:

Canadian Patent No. 2,137,942 Canadian Patent No. 2,180,305 Canadian Patent No. 2,327,768 Canadian Patent No. 2,694,159 Canadian Patent No. 2,697,287 U.S. Patent No. 6,068,765 U.S. Patent No. 6,371,690 U.S. Patent No. 7,582,216 U.S. Patent No. 7,666,303 Australia Patent No. 693.164 Australia Patent No. 729,096 Australia Patent No. 2008,279,378 Australia Patent No. 2008,288,900 Japanese Patent No. 5,997,750 Japanese Patent No. 5,555,160 Korean Patent No. 0519212 Korean Patent No. 1451593 New Zealand Patent No. 583,008 New Zealand Patent No. 583,583 South African Patent No. 2010/00682

South African Patent No. 2010/01796

Patent pending



Table of Contents:

- 1 Stormceptor EF Overview
- 2 Stormceptor EF Operation, Components
- 3 Stormceptor EF Model Details
- 4 Stormceptor EF Identification
- 5 Stormceptor EF Inspection & Maintenance
- **6 Stormceptor Contacts**



OVERVIEW

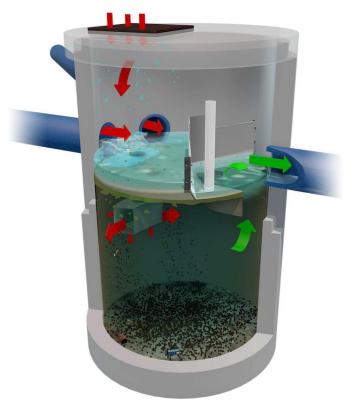
Stormceptor® EF is a continuation and evolution of the most globally recognized oil grit separator (OGS) stormwater treatment technology - *Stormceptor®*. Also known as a hydrodynamic separator, the enhanced flow Stormceptor EF is a high performing oil grit separator that effectively removes a wide variety of pollutants from stormwater and snowmelt runoff at flow rates higher than the original Stormceptor. Stormceptor EF captures and retains sediment (TSS), free oils, gross pollutants and other pollutants that attach to particles, such as nutrients and metals. Stormceptor EF's patent-pending treatment and scour prevention platform ensures sediment is retained during all rainfall events.

Stormceptor EF offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe, multiple inlet pipes, and/or from the surface through an inlet grate. Stormceptor EF can also serve as a junction structure, accommodate a 90-degree inlet to outlet bend angle, and be modified to ensure performance in submerged conditions. With its scour prevention and internal bypass, Stormceptor EF can be installed online, eliminating the need for costly additional bypass structures.



OPERATION

- Stormwater enters the Stormceptor upper chamber through the inlet pipe(s) or a surface inlet grate. A specially designed insert reduces the influent velocity by creating a pond upstream of the insert's weir. Sediment particles immediately begin to settle. Swirling flow sweeps water, sediment, and floatables across the sloped surface of the insert to the inlet opening of the drop pipe, where a strong vortex draws water, sediment, oil, and debris down the drop pipe cone.
- Influent exits the cone into the drop pipe duct. The duct has two large rectangular outlet openings as well as perforations in the backside and floor of the duct. Influent is diffused through these various opening in multiple directions and at low velocity into the lower chamber.
- Free oils and other floatables rise up within the channel surrounding the central riser pipe and
 are trapped beneath the insert, while sediment settles to the sump. Pollutants are retained for
 later removal during maintenance cleaning.
- Treated effluent enters the outlet riser, moves upward, and discharges to the top side of the insert downstream of the weir, where it flows out the outlet pipe.
- During intense storm events with very high influent flow rates, the pond height on the upstream side of the weir may exceed the height of the weir, and the excess flow passes over the top of the weir to the downstream side of the insert, and exits through the outlet pipe. This internal bypass feature allows for in-line installation, avoiding the cost of additional bypass structures. During bypass, the pond separates sediment from all incoming flows, while full treatment in the lower chamber continues at the maximum flow rate.
- Stormceptor EF's patent-pending enhanced flow and scour prevention technology ensures
 pollutants are captured and retained, allowing excess flows to bypass during infrequent, high
 intensity storms.





COMPONENTS

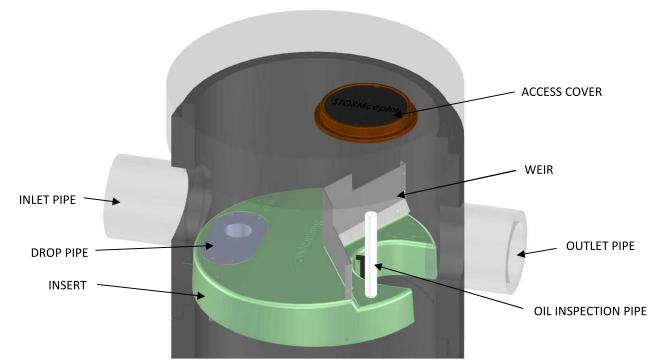


Figure 1

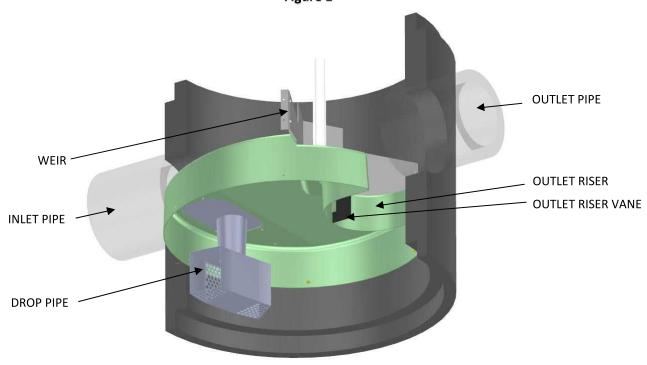
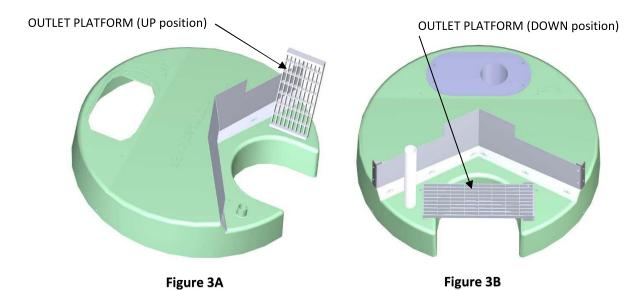


Figure 2





- Insert separates vessel into upper and lower chambers, and provides double-wall containment of hydrocarbons
- Weir creates stormwater ponding and driving head on top side of insert
- **Drop pipe** conveys stormwater and pollutants into the lower chamber
- **Outlet riser** conveys treated stormwater from the lower chamber to the outlet pipe, and provides primary inspection and maintenance access into the lower chamber
- **Outlet riser vane** prevents formation of a vortex in the outlet riser during high flow rate conditions
- Outlet platform (optional) safety platform in the event of manned entry into the unit
- Oil inspection pipe primary access for measuring oil depth



PRODUCT DETAILS

METRIC DIMENSIONS AND CAPACITIES

Table 1

Stormceptor Model	Inside Diameter (m)	Minimum Surface to Outlet Invert Depth (mm)	Depth Below Outlet Pipe Invert (mm)	Wet Volume (L)	Sediment Capacity ¹ (m³)	Hydrocarbon Storage Capacity ² (L)	Maximum Flow Rate into Lower Chamber ³ (L/s)	Peak Conveyance Flow Rate ⁴ (L/s)
EF4 / EFO4	1.22	915	1524	1780	1.19	265	22.1 / 10.4	425
EF6 / EFO6	1.83	915	1930	5070	3.47	610	49.6 / 23.4	990
EF8 / EFO8	2.44	1219	2591	12090	8.78	1070	88.3 / 41.6	1700
EF10 / EFO10	3.05	1219	3251	23700	17.79	1670	138 / 65	2830
EF12 / EFO12	3.66	1524	3886	40800	31.22	2475	198.7 / 93.7	2830

¹ Sediment Capacity is measured from the floor to the bottom of the drop pipe cone. Sediment Capacity can be increased to accommodate specific site designs and pollutant loads. Contact your local representative for assistance.

U.S. DIMENSIONS AND CAPACITIES

Table 2

Stormceptor Model	Inside Diameter (ft)	Minimum Surface to Outlet Invert Depth (in)	Depth Below Outlet Pipe Invert (in)	Wet Volume (gal)	Sediment Capacity ¹ (ft ³)	Hydrocarbon Storage Capacity ² (gal)	Maximum Flow Rate into Lower Chamber ³ (cfs)	Peak Conveyance Flow Rate ⁴ (cfs)
EF4 / EFO4	4	36	60	471	42	70	0.78 / 0.37	15
EF6 / EFO6	6	36	76	1339	123	160	1.75 / 0.83	35
EF8 / EFO8	8	48	102	3194	310	280	3.12 / 1.47	60
EF10 / EFO10	10	48	128	6261	628	440	4.87 / 2.30	100
EF12 / EFO12	12	60	153	10779	1103	655	7.02 / 3.31	100

¹ Sediment Capacity is measured from the floor to the bottom of the drop pipe cone. Sediment Capacity can be increased to accommodate specific site designs and pollutant loads. Contact your local representative for assistance.

² Hydrocarbon Storage Capacity is measured from the bottom of the outlet riser to the underside of the insert. Hydrocarbon Storage Capacity can be increased to accommodate specific site designs and pollutant loads. Contact your local representative for assistance.

³ EF Maximum Flow Rate into Lower Chamber is based on a maximum surface loading rate (SLR) into the lower chamber of 1135 L/min/m². EFO Maximum Flow Rate into Lower Chamber is based on a maximum surface loading rate (SLR) into the lower chamber of 535 L/min/m².

⁴ Peak Conveyance Flow Rate is limited by a maximum velocity of 1.5 m/s.

² Hydrocarbon Storage Capacity is measured from the bottom of the outlet riser to the underside of the insert. Hydrocarbon Storage Capacity can be increased to accommodate specific site designs and pollutant loads. Contact your local representative for assistance.

³ EF Maximum Flow Rate into Lower Chamber is based on a maximum surface loading rate (SLR) into the lower chamber of 27.9 gpm/ft². EFO Maximum Flow Rate into Lower Chamber is based on a maximum surface loading rate (SLR) into the lower chamber of 13.1 gpm/ft².

 $^{^{\}rm 4}\,\text{Peak}$ Conveyance Flow Rate is limited by a maximum velocity of 5 fps.



IDENTIFICATION

Each Stormceptor EF/EFO unit is easily identifiable by the trade name *Stormceptor*® embossed on the access cover at grade as shown in *Figure 3*. The tradename *Stormceptor*® is also embossed on the top of the insert upstream of the weir as shown in *Figure 3*.

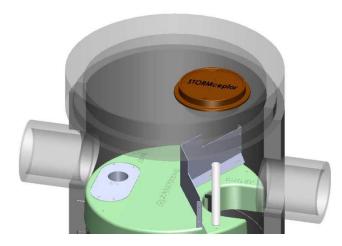


Figure 4

The specific Stormceptor EF/EFO model number is identified on the top of the aluminum Drop Pipe as shown in **Figure 4**. The unit serial number is identified on the top of the insert upstream of the weir as shown in **Figure 4**.

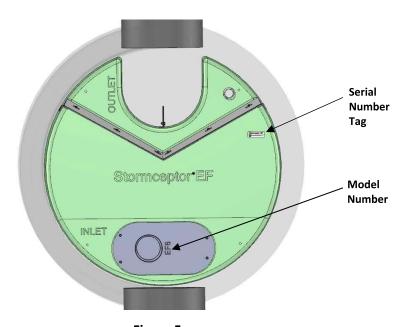


Figure 5



INSPECTION AND MAINTENANCE

It is very important to perform regular inspection and maintenance. Regular inspection and maintenance ensures maximum operation efficiency, keeps maintenance costs low, and provides continued of natural waterways.

Quick Reference

- Typical inspection and maintenance is performed from grade
- Remove manhole cover(s) or inlet grate to access insert and lower chamber
 NOTE: EF4/EFO4 requires the removal of a flow deflector beneath inlet grate
- Use Sludge Judge® or similar sediment probe to check sediment depth through the outlet riser
- Oil dipstick can be inserted through the oil inspection pipe
- Visually inspect the **insert** for debris, remove debris if present
- Visually inspect the drop pipe opening for blockage, remove blockage if present
- Visually inspect insert and weir for damage, schedule repair if needed
- Insert vacuum hose and jetting wand through the outlet riser and extract sediment and floatables
- Replace flow deflector (EF4/EFO4), inlet grate, and cover(s)
- **NOTE:** If the unit has an **outlet platform**, the outlet platform is typically in the UP position (see Figure 3A) for normal treatment conditions, and for inspection and maintenance. If manned entry into the unit is required, the outlet platform must first be placed in the DOWN position (see Figure 3B). After manned entry is completed, return the outlet platform to the UP position for treatment.

When is inspection needed?

- o Post-construction inspection is required prior to putting the Stormceptor into service.
- o Routine inspections are recommended during the first year of operation to accurately assess pollutant accumulation.
- o Inspection frequency in subsequent years is based on the maintenance plan developed in the first year.
- Inspections should also be performed immediately after oil, fuel, or other chemical spills.

What equipment is typically required for inspection?

- Manhole access cover lifting tool
- Oil dipstick / Sediment probe with ball valve (typically %-inch to 1-inch diameter)
- o Flashlight
- o Camera
- Data log / Inspection Report
- Safety cones and caution tape
- O Hard hat, safety shoes, safety glasses, and chemical-resistant gloves



When is maintenance cleaning needed?

- o If the post-construction inspection indicates presence of construction sediment of a depth greater than a few inches, maintenance is recommended at that time.
- o For optimum performance and normal operation the unit should be cleaned out once the sediment depth reaches the recommended maintenance sediment depth, see **Table 3**.
- o Maintain immediately after an oil, fuel, or other chemical spill.

Table 3

Recommended Sediment Depths for					
Maintenance Service*					
MODEL	Sediment Depth				
MODEL	(in/mm)				
EF4 / EFO4	8 / 203				
EF6 / EFO6	12 /305				
EF8 / EFO8	24 / 610				
EF10 / EFO10	24 / 610				
EF12 / EFO12	24 / 610				

^{*} Based on a minimum distance of 40 inches (1,016 mm) from bottom of outlet riser to top of sediment bed

The frequency of inspection and maintenance may need to be adjusted based on site conditions to ensure the unit is operating and performing as intended. Maintenance costs will vary based on the size of the unit, site conditions, local requirements, disposal costs, and transportation distance.

What equipment is typically required for maintenance?

- Vacuum truck equipped with water hose and jet nozzle
- Small pump and tubing for oil removal
- Manhole access cover lifting tool
- o Oil dipstick / Sediment probe with ball valve (typically ¾-inch to 1-inch diameter)
- o Flashlight
- o Camera
- Data log / Inspection Report
- Safety cones
- Hard hats, safety shoes, safety glasses, chemical-resistant gloves, and hearing protection for service providers
- Gas analyzer, respiratory gear, and safety harness for specially trained personnel if confined space entry is required (adhere to all OSHA / CCOSH standards)

What conditions can compromise Stormceptor performance?

- Presence of construction sediment and debris in the unit prior to activation
- Excessive sediment depth beyond the recommended maintenance depth
- o Oil spill in excess of the oil storage capacity
- Clogging or restriction of the drop pipe inlet opening with debris
- o Downstream blockage that results in a backwater condition



Maintenance Procedures

- Maintenance should be conducted during dry weather conditions when no flow is entering the unit.
- Stormceptor is maintained from grade through a standard surface manhole access cover or inlet grate.
- In the case of submerged or tailwater conditions, extra measures are likely required, such as plugging the inlet and outlet pipes prior to conducting maintenance.
- Inspection and maintenance of upstream catch basins and other stormwater conveyance structures is also recommended to extend the time between future maintenance cycles.

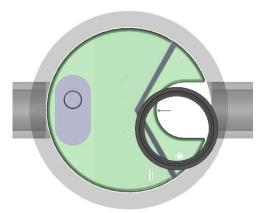


Figure 6

- Sediment depth inspections are performed through the **Outlet Riser** and oil presence can be determined through the **Oil Inspection Pipe**.
- Oil presence and sediment depth are determined by inserting a Sludge Judge or measuring stick to quantify the pollutant depths.

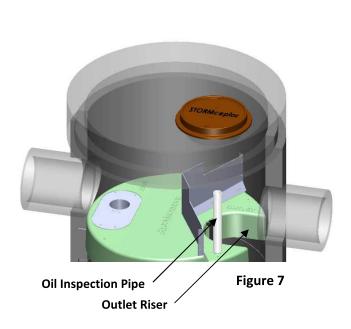




Figure 8

- Visually inspect the insert, weir, and drop pipe inlet opening to ensure there is no damage or blockage.
- **NOTE:** If the unit has an **outlet platform**, the outlet platform is typically in the UP position (see Figure 3A) for normal treatment conditions, and for inspection and maintenance. If manned entry into the unit is required, the outlet platform must first be placed in the DOWN position (see Figure 3B). After manned entry is completed, return the outlet platform to the UP position for treatment.



• When maintenance is required, a standard vacuum truck is used to remove the pollutants from the lower chamber of the unit through the **Outlet Riser**.



Figure 9

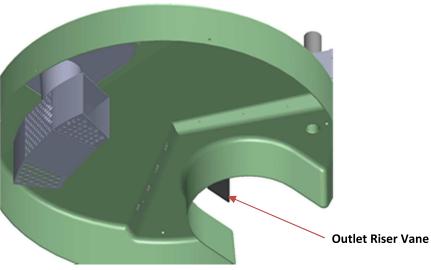


Figure 10

NOTE: The Outlet Riser Vane is durable and flexible and designed to allow maintenance activities with minimal, if any, interference.



Removable Flow Deflector

• Top grated inlets for the Stormceptor EF4/EFO4 model requires a removable flow deflector staged underneath a 24-inch x 24-inch (600 mm x 600 mm) square inlet grate to direct flow towards the inlet side of the insert, and avoid flow and pollutants from entering the outlet side of the insert from grade. The EF6/EFO6 and larger models do not require the flow deflector.

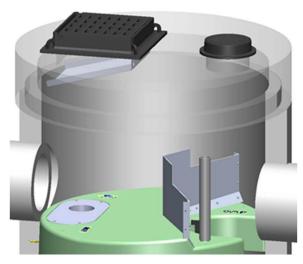
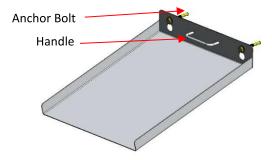


Figure 11

How to Remove:

- 1. Loosen anchor bolts
- 2. Pull up and out using the handle



Removable Flow Deflector



Hydrocarbon Spills

Stormceptor is often installed on high pollutant load hotspot sites with vehicular traffic where hydrocarbon spill potential exists. Should a spill occur, or presence of oil be identified within a Stormceptor EF/EFO, it should be cleaned immediately by a licensed liquid waste hauler.

Disposal

Maintenance providers are to follow all federal, state/ provincial, and local requirements for disposal of material.

Oil Sheens

When oil is present in stormwater runoff, a sheen may be noticeable at the Stormceptor outlet. An oil rainbow or sheen can be noticeable at very low oil concentrations (< 10 mg/L). Despite the appearance of a sheen, Stormceptor EF/EFO may still be functioning as intended.

Oil Level Alarm

To mitigate spill liability with 24/7 detection, an electronic monitoring system can be employed to trigger a visual and audible alarm when a pre-set level of oil is captured within the lower chamber or when an oil spill occurs. The oil level alarm is available as an optional feature to include with Stormceptor EF/EFO as shown in **Figure 11**. For additional details about the Oil Level Alarm please visit http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-systems.

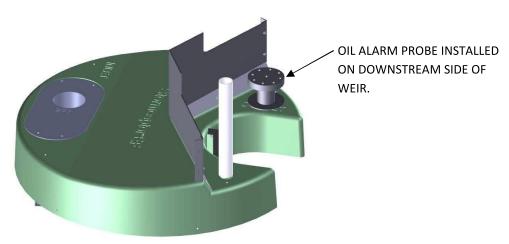


Figure 12

Replacement Parts

Stormceptor has no moving parts to wear out. Therefore inspection and maintenance activities are generally focused on pollutant removal. Since there are no moving parts during operation in a Stormceptor, broken, damaged, or worn parts are not typically encountered. However, if replacement parts are necessary, they may be purchased by contacting your local Stormceptor representative.



Stormceptor Inspection and Maintenance Log

Stormceptor Model No:	
Serial Number:	
Installation Date:	
Location Description of Unit:	
Recommended Sediment Maintenance Depth:	

DATE	SEDIMENT DEPTH (inch or mm)	OIL DEPTH (inch or mm)	SERVICE REQUIRED (Yes / No)	MAINTENANCE PERFORMED	MAINTENANCE PROVIDER	COMMENTS

Other Comments:



Contact Information

Questions regarding Stormceptor EF/EFO can be addressed by contacting your local Stormceptor representative or by visiting our website at www.stormceptor.com.

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