

Appendix K:

Fluvial Geomorphology Report

Municipal Class Environmental Assessment Airport Road Between King Street and Huntsmill Drive, Town of Caledon

Fluvial Geomorphological Assessment Centreville Creek and East Credit River Tributary



Prepared for: IBI Group 100 – 175 Galaxy Blvd Toronto, ON M9W 0C9

August 20, 2020 Project No. 17101





Report Prepared by:	GEO Morphix Ltd. 36 Main Street North Campbellville, ON LOP 1B0
Report Title:	Municipal Class Environmental Assessment Airport Road Between King Street and Huntsmill Drive Town of Caldeon Fluvial Geomorphological Assessment Centreville Creek and East Credit River Tributary
Project Number:	17101
Status:	Final Draft
Version:	1.0
First Submission Date:	August 20, 2020
Revision Date:	
Prepared by:	Suzanne St. Onge, M.Sc. Kevin Tabata, M.Sc., CAN-CISEC
Approved by:	Paul Villard, Ph.D., P.Geo., CAN-CISEC
Approval Date:	

Table of Contents

1	Intro	duction1
2	Study	y Site History
3	Subw	atershed-scale Characteristics4
	3.1	Geology and Physiography4
4	Drain	age Basin Characteristics5
	4.1	Centreville Creek
	4.2	East Credit River
5	Wate	rcourse Characteristics
	5.1	Reach Delineation
		5.1.1 General Reach Observations7
		5.1.2 Reconnaissance-level Assessments
6	Mean	der Belt Width Delineation 11
7	Cross	sing Structure Recommendations13
	7.1	Proposed Road Improvements
	7.2	Crossing Guidelines
	7.3	Other Crossing Considerations
	7.4	Crossing Recommendations14
8	Sumr	nary16
9	Refer	rences

List of Figures

Figure 1: Reach ex	tents assessed		2
--------------------	----------------	--	---

List of Tables

Table 1: Local physiography and surficial geology at each crossing	5
Table 2: Portions of watercourse assessed along Airport Road from north of Huntsmill Drive t King Street	0 7
Table 3: Summary of reconnaissance-level assessments	0
Table 4: Modelled meander belt widths for Boyce's Creek and Centreville Creek 1	2
Table 5: Existing and proposed crossing sizes 1	4

Appendices

Appendix A: Historical Aerial Imagery

Appendix B: Photographic Record

Appendix C: Field Sheets

Appendix D: Meander Belt Width Assessment

1 Introduction

The Regional Municipality of Peel is undertaking a Schedule C Municipal Class Environmental Assessment (Class EA) in support of long-term improvements to Airport Road (Regional Road 7) between King Street (Regional Road 9) and Huntsmill Drive in the Town of Caledon. Airport Road consists of two through lanes in a north-south orientation. Within the Community of Caledon East, the roadway has an urban cross section, while the remainder of Airport Road has a rural cross section. Airport Road currently supports a significant volume of commuter and truck traffic and is identified as a primary truck route. The Long Range Transportation Update (Region of Peel, 2012) recommended the widening of Airport Road by 2031 to include up to four lanes of through traffic, and other infrastructure to enable the efficient movement of people and goods.

GEO Morphix Ltd. was retained as part of a multi-disciplinary consulting team led by IBI Group to provide fluvial geomorphological support for the Class EA process. Seven regulated watercourse crossings were identified as part of this study and are located within the jurisdictions of Credit Valley Conservation (CVC) and Toronto and Region Conservation Authority (TRCA). Three of the features that cross Airport Road are classified as headwater drainage features (HDFs) and were investigated by the TRCA as part of the Natural Environmental Existing Conditions Report (September 2017). As such, the HDFs were not included in the fluvial geomorphological assessments. The following four regulated watercourse crossings were assessed as part of this study:

- Boyce's Creek, a tributary of Centreville Creek (Crossing 1)
- Unnamed tributary of Centreville Creek (Crossing 2)
- Centreville Creek (Crossing 3)
- Unnamed tributary of the East Credit River (Crossing 7)

Centreville Creek and its tributaries travel from west to east across Airport Road, while the East Credit River tributary travels from east to west (**Figure 1**).

The activities listed below were completed in support of the geomorphological assessment:

- Review available background reports and mapping (e.g., soils, physiography, geology, and topography)
- Complete a historical assessment using aerial photographs to identify changes to the system due to land use and past channel modifications
- Delineate the meander belt width in the vicinity of Airport Road, where feasible
- Determine meander migration rates, where feasible
- Conduct rapid geomorphological field assessments for portions of accessible channel upstream and downstream of each of the three watercourse crossings to document channel conditions and verify the results of the desktop assessment
- Assess the effects of the existing crossing structures to channel form and function
- Evaluate the crossing structure alternatives with respect to potential impacts on channel form and function
- Provide recommendations, from a fluvial geomorphological perspective, on crossing structure spans for replacements and/or enhancements for culvert modifications/extensions, with consideration to other factors such as hydraulics, ecology, fisheries and various physical constraints as determined through the study.



2 Study Site History

A series of historical aerial photographs were reviewed to determine changes to the channel and surrounding land use/cover. This information, in part, provides an understanding of the historical factors that have contributed to current channel morphodynamics, as well as the basis for understanding the potential future changes to the channel. Aerial photographs from 1954 (scale 1:15,840) and 1978 (scale 1:10,000) from the Ministry of Natural Resources and Forestry (MNRF) and recent satellite imagery from Google Earth Pro (2016) were reviewed to complete the historical assessment. Refer to **Appendix A** for copies of the imagery.

In 1954, the predominant land use was agricultural and rural residential. Several natural areas and woodlots were also present on either side of Airport Road. Innis Lake, Elliot Lake and Widget Lake were prominent natural features east of Airport Road. The Community of Caledon East, now with approximately 5,000 residents, had established by 1954. Boyce's Creek was not visible upstream and downstream of Airport Road at Crossing 1 due to the presence of woodlands. The channel appeared to have an extensive riparian buffer upstream (west) of Airport Road, but this buffer may have contained significant localized gaps in the vicinity of lands cleared for agricultural fields/pastures.

The upstream reaches of tributaries to Centreville Creek (west of Airport Road) travelled through forests that obscured the channel. Cultivated fields and/or pasture were present between tributaries upstream of Airport Road in the area of what is now Walkers Road West. These tributaries travelled through the Caledon East and across Airport Road (Crossing 2). It is likely that upstream portions of the tributary were piped prior to 1954 to facilitate rural development, with additional enclosures on the east side of Airport Road to accommodate development in subsequent years.

The former Canadian National Railway (CNR) rail line (now the Caledon Trailway) was present in 1954. Upstream reaches of Centreville Creek travelled through large woodlands west of Airport Road. Where the channel crossed agricultural fields farther upstream, west of Mountainview Road, there was no riparian buffer and the channel had been straightened. This likely increased stream power as well as the likelihood of subsequent systematic channel adjustments (e.g., widening, downcutting, meander bend development). The channel planform was only visible near Airport Road on the north side of the CNR rail line adjacent to rural residences before crossing the Trailway and then Airport Road to a woodland with agricultural/pasture lands on either side. Where the channel was visible, it was generally straight with the exception of a relatively large meander bend on rural property west of Airport Road. Riparian vegetation was likely also actively removed/maintained while the CNR rail line was in operation.

The tributary of the East Credit River was straightened prior to 1954 and lacked a riparian buffer. The channel appeared to originate in a relatively small woodland on the east side of Airport Road that had likely been impacted by selective tree clearing and adjacent agricultural/pasture land uses. It then crossed Airport Road at Crossing 7, and subsequently travelled through agricultural fields where it was previously straightened. Adjacent agricultural activities and the lack of a riparian buffer likely resulted in frequent fine sediment inputs.

By 1978, the Community of Caledon East had expanded south of the CNR rail line, and to the east and west of Airport Road. There was limited change near Crossing 1, except where agricultural fields had begun to naturalize farther upstream and two large ponds had been excavated west of Airport Road. A direct connection between Boyce's Creek and the ponds was not discernible due to the woody vegetation cover. At Crossing 2, additional sections of the tributary were likely piped east of Airport Road to facilitate construction of new residences and infrastructure along Robert Carson Drive. Residential development had expanded south of the CNR line in 1978 near Crossing 3. Portions of visible channel planform upstream of Airport Road, adjacent to the rail line, were slightly more sinuous, while on the east side of Airport Road, former agricultural fields appeared to no longer be actively farmed immediately south of the CNR line. Rural residential and industrial/commercial development had expanded slightly in vicinity of Crossing 7, mostly on the east side of Airport Road; however, the watercourse remained largely unchanged. The upstream drainage area within the woodland on the east side of Airport Road had been allowed to expand and naturalize.

Between 1978 and 2015, the Community of Caledon East had expanded further. Naturally forested areas had also continued to expand along Boyce's Creek upstream of Crossing 1. Huntsmill Drive was constructed within the approximate footprint of a former access road that was visible in 1978 imagery. Several landscaped ponds were also constructed northeast of Airport Road and Crossing 1. Additional residences were constructed adjacent to the tributaries of Centreville Creek, upstream of Crossing 2, while maintaining a riparian buffer. There was limited change downstream of Crossing 2. At Crossing 3, riparian conditions west of Airport Road had improved since 1978 through the expansion of woody vegetation that now likely provides additional shade and cover to Centreville Creek. This is likely due to decommissioning of the CNR line in the 1980s and its conversion to the Caledon Trailway, a multi-use trail intended for nonmotorized vehicles and pedestrians. The large meander bend upstream of Airport Road, within residential property, was more pronounced and had migrated northwards. In addition, a pond had been constructed on the south side of the Trailway and drained to Centreville Creek. On the immediate east side of Airport Road, agricultural fields were converted to residential housing; however, Centreville Creek travelled through a natural area with forest cover. A wetland had also formed between the Trailway and Centreville Creek and had a boardwalk/viewing platform extending from the Trailway to the south side of the watercourse.

Overall, land use within the study area has largely remained as rural residential, with the gradual expansion of the Community of Caledon East. With the exception of additional enclosures of sections of watercourse near Crossing 2 and the continued lack of a riparian buffer downstream of Crossing 7, the naturalization of several former agricultural fields and the conversion of the CNR line to the Caledon Trailway have likely allowed for the natural local improvement of channel form and function, as well as aquatic and riparian habitats, along Airport Road.

3 Subwatershed-scale Characteristics

3.1 Geology and Physiography

Geology and physiography act as primary governing variables with respect to channel geomorphology. These factors determine the nature and quantity of the availability and type of sediment. Secondary variables that affect the channel include land use and riparian vegetation. These factors are explored as they not only offer insight into existing conditions, but also potential changes that could be expected in the future as they relate to a proposed activity. Although the crossings are in close proximity geographically, the local physiography and surficial geology varies and are summarized in the **Table 1**.

Crossings 1, 2 and 3 area located in the Oak Ridges Moraine, which serves as a drainage divide between the Lake Ontario drainage basin to the south and Georgian Bay and Trent River drainage basins to the north. This area is a significant source of groundwater due to the permeability of the soils (e.g., sand and gravel) and therefore helps to keep watercourses flowing year-round through the provision of baseflow.

Watercourse	Physiographic Region (Chapman and Putnam, 1984)	Local Physiography (Chapman and Putnam, 2007)	Surficial Geology (Ontario Geological Survey, 2010)
Boyce's Creek (Crossing 1)	Oak Ridges Moraine	Kame moraines	Ice-contact stratified deposits (sand and gravel, minor silt, clay and till)
Centreville Creek Tributary (Crossing 2)	Oak Ridges Moraine	Spillway	Glaciofluvial deposits with delta topset facies
Centreville Creek (Crossing 3)	Oak Ridges Moraine	Spillway	Glaciofluvial deposits with delta topset facies
East Credit River Tributary (Crossing 7)	South Slope	Drumlinized till plains	Clay to silt textured till derived from glaciolacustrine deposits or shale

Table 1: Local physiography and surficial geology at each crossing

Crossing 7, in contrast, is in the South Slope physiographic region, which, as the name suggests, is a gently sloping area. It is composed of a plain of clay to silt textured glacial till and is therefore less permeable.

The bedrock geology at the four crossings consists of shale, limestone, dolostone, and siltstone of the Queenston Formation (OGS, 2011). No exposed bedrock was encountered in any sections of channel assessed in the field.

4 Drainage Basin Characteristics

As noted in the Natural Environment Report (TRCA, 2017), the study area includes a number of designated natural areas, including wetlands, Environmentally Significant Areas (ESAs) and Areas of Natural and Scientific Interest (ANSIs). Two subwatersheds are located within the study area: Centreville Creek, which includes Boyce's Creek (a tributary), and the East Credit River.

4.1 Centreville Creek

Centreville Creek, which joins the main branch of the Humber River at Albion Hills Conservation Area, has a drainage area of approximately 47 km² and is located entirely within the Town of Caledon. Land use is predominantly rural with natural and managed forests, wetlands, croplands, pastures, dairy estate properties and major greenspace areas (TRCA, 2008). The majority of the subwatershed is located on the Oak Ridges Moraine, a significant groundwater recharge area, and a minor portion is located on the Niagara Escarpment (TRCA, 2008). Many kettle depressions occur in the subwatershed, forming locally and provincially significant wetlands (PSWs) and three small lakes (Elliot, Innis, and Widget Lakes), located east of Airport Road. Many of the watercourses in the subwatershed are coldwater streams providing high quality habitat for sensitive species including Brook Trout (*Salvelinus fontinalis*).

Due to highly permeable soils and the underlying surficial geology of the Oak Ridges Moraine, which favours infiltration over surface runoff, this system is influenced to a lesser degree by

precipitation than those located on the South Slope and Peel Plain (TRCA, 2008). During dry periods, many of the first, second and third order watercourses contain baseflow due to groundwater inputs from the Oak Ridges Aquifer Complex (ORAC). However, downstream of the Community of Caledon East, Centreville Creek flows through the highly permeable sediments of the Caledon East Meltwater Channel, and this section of channel acts as a groundwater recharge area.

4.2 East Credit River

The East Credit River has a drainage area of approximately 51 km² and is located entirely within the Town of Caledon (CVC, 2002). Similar to Centreville Creek, the landscape is dominated by the Niagara Escarpment and the Oak Ridges Moraine. This subwatershed discharges to the Credit River upstream of the Village of Inglewood, located approximately 8 km southwest of the study area (CVC, 2002). Predominant land uses include intensive and non-intensive agriculture, which are largely located in the southern portion of the subwatershed.

The main channel of the East Credit River is approximately 11 km in length, with major tributaries draining from the northwest, originating along the Niagara Escarpment. The mainstem is positioned within glacial spillways and has a generally low gradient and wide floodplain, while the main tributaries have steep gradients where water flows down the escarpment (CVC, 2002). The Niagara Escarpment and Oak Ridges Moraine provide groundwater discharge to the East Credit River that helps to sustain healthy populations of Brook Trout (CVC, 2007).

5 Watercourse Characteristics

5.1 Reach Delineation

Reaches are homogeneous segments of channel used in geomorphological investigations. Reaches are studied semi-independently as each is expected to function in a manner that is at least slightly different from adjoining reaches. This method allows for a meaningful characterization of a watercourse as the aggregate of reaches, or an understanding of a reach, for example, as it relates to a proposed activity.

Reaches are typically delineated based on changes in the following:

- Channel planform
- Channel gradient
- Physiography
- Land cover (land use or vegetation)
- Flow, due to tributary inputs
- Soil type and surficial geology
- Historical channel modifications

Reaches are delineated following scientifically defensible methodology proposed by Montgomery and Buffington (1997), Richards et al. (1997), and the Toronto and Region Conservation Authority (2004). For this study, the full length of each reach was not verified due to site access limitations. Furthermore, limited details are available for portions of the Boyce's Creek channel upstream and downstream of Crossing 1 as permission to access the lands could not be obtained. **Table 2** provides a list of the portions of watercourses assessed upstream and downstream of each crossing, as well as their locations and defining characteristics.

Table 2: Portions	of watercourse	assessed	along	Airport	Road	from	north	of
Huntsmill Drive to	King Street							

Watercourse	Reach	Extent Assessed	Length (m)	Defining Characteristics
Boyce's Creek (Crossing 1)	BC-1	Channel assessed within the Airport Road ROW	100	Unconfined valley, no channel development, heavily vegetated
Centreville Creek Tributary (Crossing 2)	CCT-1	North of Ivan Avenue, East of Ella Street	80	Unconfined valley, sinuous planform, no riffles or pools
Centreville Creek Tributary (Crossing 2)	CCT-2	North of Caledon Trailway from 60 m east of Airport Road to elevated boardwalk	80	Unconfined valley, minimal channel development, heavily vegetated, straight planform
Centreville Creek (Crossing 3)	CC-1	Caledon Trailway from Dufferin Street to elevated boardwalk east of Airport Road	330	Unconfined valley, straightened channel, riffle- pool morphology
East Credit River Tributary (Crossing 7)	ECRT-1	Airport Road 250 m south of Olde Base Line Road to 5943 Airport Road	220	Unconfined valley, minimal channel development, extensively vegetated, straight planform

5.1.1 General Reach Observations

Field investigations were completed on August 23, 2018 and included the following:

- Habitat sketch maps based on Newson and Newson (2000) outlining channel substrate, flow patterns, geomorphological units (e.g., riffle, run, pool), and riparian vegetation for the extent of each reach assessed
- Descriptions of riparian conditions
- Estimates of bankfull channel dimensions
- Bed and bank material composition and structure
- Observations of erosion, scour or deposition
- Collection of photographs to document the watercourses, riparian areas and/or valley, surrounding land use, and channel disturbances such as crossing structures

These observations and measurements are summarized below. The descriptions are supplemented and supported with representative photographs, which are included in **Appendix B**. Field sheets, including reach summaries, habitat sketch maps and rapid assessments, are provided in **Appendix C**.

Crossing 1 – Boyce's Creek

Reach BC-1

Crossing 1 was a 0.90 m diameter CSP culvert located approximately 70 m north of Huntsmill Drive. The culvert inlet was slightly perched (8 cm) above the channel bed. The area upstream (west) of Crossing 1, was occupied by a cattail marsh with no defined watercourse. This marsh likely developed as a result of the perched culvert inlet, which would create a backwater and a low-energy environment conducive to wetland vegetation establishment.

At the culvert outlet, there was a 0.3 m deep scour pool, but also backwatering into the culvert. Flows dispersed across a phragmites-dominated wetland such that there was no defined channel within the road allowance. As flows entered private property, the landscape changed to a forest and a well-defined channel formed. The channel had a riffle and pool morphology. Riffle substrate was predominantly gravel and cobble with some larger boulders, and pool substrate was composed of gravel and sand. Bankfull channel measurements were taken just within the road allowance, and the average bankfull width and average maximum bankfull depth were measured to be 2.20 and 0.6 m, respectively.

Through the private property, the TRCA (2017) documented a defined channel with an average width of 0.68 m and an average depth of 0.07 m. The channel was significantly smaller than it was within the road allowance as it was presumably no longer affected by culvert hydraulics (i.e., channel scouring at the culvert outlet). The TRCA also observed pools, glides and riffles with substrate up to cobble size.

Crossing 2 – Centreville Creek Tributary

Crossing 2 was a 0.85 m diameter CSP culvert with an inlet and outlet well outside of the Airport Road road allowance. The culvert inlet was approximately 30 m west of Airport Road on the north side of Parsons Avenue, and the outlet was 60 m east of Airport Road on the north side of the Caledon Trailway.

Reach CCT-1

Most of the tributary was piped in the vicinity of Airport Road due to the angled approach of the channel relative to Airport Road and the roads perpendicular to Airport Road. The upstream extent of observations was collected near the confluence of two branches of the tributary northeast of Ella Street and northwest of Ivan Avenue. The channel conveyed flows in a southeasterly direction through a sinuous, low-gradient channel with limited morphological bed variability. There was little instream vegetation, and the channel bed and banks were composed of predominantly sand and clay. The average bankfull width was 1 m, and the average maximum bankfull depth was 0.37 m. The channel travelled through woodland with dense immature deciduous trees and herbaceous vegetation.

As the channel approached the northeast to southeast bend of Ivan Avenue, which was not accessed as permission to enter the property was not obtained, flows entered a culvert and discharged to an open channel approximately 120 m southeast. The channel was only open for approximately 15 m before entering Crossing 2 on the north side of Parsons Avenue. Here, the channel was lined along one bank with flagstone. The average bankfull width of the channel was 0.82 m and the average maximum bankfull depth was 0.6 m.

Reach CCT-2

At the Crossing 2 outlet on the east side of Airport Road, there was a shallow pool with organic substrate. Beyond the pool, channel definition was poor as water flowed through a 4.6 m wide,

0.47 m deep ditch populated with reeds and cattails. Due to the lack of energy, there was a positive feedback relationship between in-channel vegetation establishment and sedimentation (i.e., bed aggradation), which created a roughly 0.4 m high backwater into the culvert outlet. The ditch was located between the fence line of the residential properties of Robert Carson Drive and the Caledon Trailway. An oily film was observed on the water surface at several locations within the ditch, and the water generally had an organic odour indicating poor water quality.

Crossing 3 – Centreville Creek

Crossing 3 was a 4.30 m span box culvert conveying flows of Centreville Creek across Airport Road from east to west. Immediately upstream (west) of the crossing, a channel conveying flows from a small pond formed a confluence with Centreville Creek. Flow occupied the full culvert span and the bed was uniformly composed of fine silt and sand. A woody debris accumulation that partially blocked flows was observed within the culvert. Two storm sewers outletted to the channel on the downstream (east) side of the culvert: a 1.45 m diameter CSP on the north side and a 0.70 m diameter concrete pipe on the south side.

Reach CC-1

Approximately 325 m of Centreville Creek, between Dufferin Street (west of Airport Road) and the elevated boardwalk adjacent to the Caledon Trailway (east of Airport Road), was assessed. The channel was unconfined (i.e., not in a valley setting) and had a riparian buffer generally composed of mature deciduous trees and grasses. The channel had a low gradient and was dominated by runs, with few riffles and pools. The channel substrate was predominantly composed of gravel, sand and silt, while cobbles were also observed in the few riffles. Average bankfull width was 3.45 m and average maximum bankfull depth was 0.65 m.

Crib walls, undermined and in generally poor condition, were observed along the creek at the rear of the properties located along Emma Street. These were previously constructed to prevent erosion and protect pedestrian bridges over the creek. Watercress, an indicator of possible groundwater input, was observed within the channel at several locations and was most abundant immediately downstream of a relatively large meander bend at 4 Emma Street. Downstream of Airport Road, the channel had a higher width-to-depth ratio, and the channel corridor was more akin to a wetland system.

Crossing 7 – East Credit River Tributary

The Crossing 7 outlet consisted of a 2 m span box culvert located approximately 250 m south of Olde Baseline Road. The culvert conveyed flows of a tributary of the East Credit River westwards across Airport Road. The culvert inlet was located at the rear of private property, which was not accessible and well outside of the road allowance, and therefore no observations were collected. On the west side of Airport Road, there was a culvert under a driveway at 15332 Airport Road that conveyed ditch flows to the north side of the creek. The rip-rap around the driveway culvert outlet partially blocked the Crossing 7 outlet at Airport Road.

Reach ECRT-1

Downstream of the culvert, a small, straight swale conveyed flows between two residential properties for approximately 45 m. Immediately downstream of the culvert and along the south side, the swale travelled along a small garden, at 15324 Airport Road, with a wooden retaining wall. The channel had no woody riparian cover as the channel was bounded by manicured grass, except for the portion with the small garden. The channel had an average width of 2.35 m and an average maximum depth of 0.6 m; these measurements were taken relative to the top of channel bank due to a lack of bankfull indicators. Bed material was uniformly composed of clay,

silt and decomposing organic material. At the west property line, there was a high point in the channel and this created a backwater effect with no perceptible flow through the swale.

Beyond the residential properties on the west side of Airport Road, the channel lost definition upon entering a wet meadow where flows apparently dispersed through the vegetation. Approximately 50 m from the property line, however, a small channel conveying baseflow was observed within the wet meadow. This channel had an average bankfull width of 0.5 m and a maximum bankfull depth of 0.28 m.

5.1.2 Reconnaissance-level Assessments

Channel stability was semi-quantified through the application of the Ontario Ministry of the Environment's (2003) Rapid Geomorphic Assessment (RGA). Observations were quantified using an index that identifies channel sensitivity based on evidence of aggradation, degradation, channel widening, and planimetric form adjustment. The index produces values that indicate whether the channel is stable/in regime (score <0.20), stressed/transitional (score 0.21-0.40) or adjusting (score >0.41).

The Rapid Stream Assessment Technique (RSAT) was also employed to provide a broader view of the system and considers the ecological function of the watercourse (Galli, 1996). Observations were made of channel stability, channel scouring or sediment deposition, instream and riparian habitats, and water quality. The RSAT score ranks the channel as maintaining a poor (<13), fair (13-24), good (25-34), or excellent (35-42) degree of stream health.

Reaches were also classified according to a modified Downs (1995) Channel Evolution Model. The Downs Model describes successional stages of a channel as a result of a perturbation, namely hydromodification. Understanding the current stage of the system is beneficial as this allows one to predict how the channel will continue to evolve or respond to an alteration to the system.

These reconnaissance-level assessments were applied to alluvial or semi-alluvial systems. For this study, only Reaches CCT-1 (Centreville Creek Tributary) and CC-1 (Centreville Creek) were eligible. The results are summarized in **Table 3**.

	RGA (MOE, 2003)				Downs		
Watercourse and Reach*	Score	Condition	Dominant Systematic Adjustment	Score	Condition	Limiting Feature(s)	(1995) Channel Evolution Model
Centreville Creek Tributary Reach CCT-1 (Crossing 2)	0.15	In Regime	Aggradation	28	Good	Channel Scouring/ Sediment Deposition	D - depositional
Centreville Creek Reach CC-1 (Crossing 3)	0.13	In Regime	Aggradation	29	Good	Channel Scouring/ Sediment Deposition	S - stable

Table 3: Summary of reconnaissance-level assessments

*Boyce's Creek (Crossing 1), the downstream section of Centreville Creek Tributary Reach CCT-2 (Crossing 2) and East Credit River Tributary Reach ECRT-1 (Crossing 7) were not assessed due to the absence of a defined channel.

In Centreville Creek Tributary Reach CCT-1 at Crossing 2, the only significant morphological change observed was sediment deposition as evidenced by poor longitudinal sorting of bed materials, overbank sediment deposition, and point bar accretion. This is consistent with the depositional (D) stage of the Downs (1995) model. However, due to the lack of other channel adjustment observations, the RGA resulted in a score of 0.15, which indicates that the reach is 'in regime.' The RSAT indicates good stream health with a score of 28. The limiting factors in RSAT score were sediment deposition, and poor physical instream habitat due to the lack of morphological bed variability.

Similar to the tributary, Centreville Creek Tributary Reach CC-1 exhibited limited adjustments other than aggradation, which was indicated by pool siltation and overbank sediment deposition. This resulted in an RGA score of 0.15 (in regime) and indicated a stable (S) stage of evolution according to Downs (1995). The RSAT score was 29 (good condition) with sediment deposition as the limiting factor, although minor.

6 Meander Belt Width Delineation

Most watercourses in southern Ontario have a natural tendency to develop and maintain a meandering planform, provided there are no spatial constraints. A meander belt width, or erosion hazard assessment, estimates the lateral extent that a meandering channel has historically occupied and will likely occupy in the future. This assessment is therefore useful for determining, for example, the potential limit of an activity (e.g., land development) adjacent to a watercourse, or the floodplain width required to restore a stream to a naturally functioning state.

The meander belt widths of Boyce's Creek (Crossing 1) and Centreville Creek (Crossing 3) were delineated within the Airport Road road allowance to determine the potential erosion hazard to the road as well as other surrounding infrastructure, and to estimate the optimal corridor width needed for a dynamic, fully alluvial system. Centreville Creek Tributary (Crossing 2) was omitted from this part of the study as there is no open channel within the road allowance, and the likelihood of daylighting the creek within the road allowance is low due to the conflict with another road (Parsons Avenue) and private properties. Similarly, the meander belt width of East Credit River Tributary (Crossing 7) was not determined as the culvert inlet was located in private property well outside of the road allowance. It should be noted here that meander bends migrate laterally and in the downstream direction and therefore determining the potential erosion hazard of a channel downstream of a road does not provide useful information (unless the aim is to assess the hazard downstream of a road).

All years of the available historical imagery were examined to determine the largest meander amplitude in proximity to each crossing. Due to the size of the watercourses and limitations in aerial photography, meander amplitude could only be measured for Centreville Creek (Crossing 3). For this watercourse, meander amplitudes were measured upstream and downstream of Airport Road. The largest meander amplitude, 13 m, was measured downstream of Airport Road on the 2016 photograph as well as upstream of Airport Road on the 2005 photograph. To calculate the meander belt width, the average channel bankfull width was added to the maximum meander amplitude. A 20% factor of safety was also applied, resulting in a final meander belt width of 20 m. This approach is consistent with TRCA (2004) guidelines, where a 20% factor of safety is required for channels with a maximum meander amplitude less than 50 m.

A modelling approach can be used where the channel has been previously modified or its position cannot be determined in the imagery due to tree cover or poor photograph resolution, for example. These models are scientifically defensible and have been verified in past projects as suitable for



use in southern Ontario. Empirical relations from Williams (1986) were applied using bankfull channel dimensions measured in the field to estimate the meander belt width (m), B_{W} :

$$B_w = 18A^{0.65} + W_b$$
 [Eq. 1]

$$B_w = 4.3W_b^{1.12} + W_b$$

where A is bankfull cross-sectional area (m^2) and W_b is bankfull channel width (m). An additional 20% buffer, or factor of safety, was applied to the computed results to addresses issues of under prediction.

The Ward et al. (2002) model was also used to meander belt widths (ft), Bw:

$$B_w = 6W_h^{1.12}$$

[Eq. 3]

[Eq. 2]

Again, an additional 20% buffer, or factor of safety, was applied to the results

The modelled meander belts for Boyce's Creek (Crossing 1) and Centreville Creek (Crossing 3) are provided in **Table 4**. Although a measured meander belt width has already been provided for Centreville Creek, the modelled results are presented for comparison. Moreover, it should be noted that the modelled results for Centreville Creek are based on the average channel measurements from TRCA (2017) as permission for field personnel to enter private property could not be obtained by the Region of Peel. Meander belts are shown graphically in **Appendix D**.

Table 4: Modelled meander belt widths for B	oyce's Creek and Centreville Creek
---	------------------------------------

		Meander Belt Width (m)*			
Watercourse	Reach	Williams – Area (1986)	Williams – Width (1986)	Ward et al. – Width (2002)	
Boyce's Creek (Crossing 1)	BC-1	3.8	4.2	5.4	
Centreville Creek (Crossing 3)	CC-1	34.9	24.8	33.2	

*Includes 20% factor of safety

The modelled results for Centreville Creek (Crossing 3) in **Table 4** are higher than the measured 20 m meander belt width (including 20% factor of safety). This suggests that the modelled meander belt width for Boyce's Creek (Crossing 1) may also be conservative. It should be repeated that Boyce's Creek is a marsh on either side of Airport Road. There is therefore no erosion hazard to the road, unless a single-thread channel develops by replacing the Crossing 1 culvert with one that allows for upstream bed adjustments, for example.

Determination of the 100-year erosion limits at each crossing could not be completed as the channel banks could either not be accurately delineated or were not visible in historical aerial imagery. However, based on our historical assessment, where the channel was visible there were no significant changes in channel alignment in the vicinity of Airport Road. In addition, no significant erosion was observed during the field investigations.

7 Crossing Structure Recommendations

7.1 Proposed Road Improvements

Proposed improvements to Airport Road, between King Street and Huntsmill Drive generally include the following:

- Construction of roundabouts and turning lanes to improve intersections
- Local improvements to Old Church Road and an extension west of Airport Road to Ivan Avenue
- New or improved pedestrian and cycling facilities
- Stormwater drainage upgrades
- Replacement of all crossing structures, except Crossing 2 and 5 which will be maintained without modification

There is no proposed widening of Airport Road to accommodate additional lanes of through traffic.

7.2 Crossing Guidelines

TRCA (2015) and CVC (2015) have developed crossing guidelines to address natural hazards and the maintenance of channel form and function from a geomorphological perspective. TRCA recommends that crossing structures span the meander belt width, where feasible, or, at minimum, the 100-year erosion limit to avoid the migration of the channel into the crossing structures within the next 100 years. The TRCA guidelines also allow smaller crossing structures that accommodate relatively small, stable watercourses provided that they consider physical channel characteristics (e.g., alignment, width and depth) and fluvial processes (e.g., erosion and scour).

CVC (2015) highlights several recommendations from a geomorphological perspective:

- Where possible, the crossing structure design should avoid the need for channel armouring or adjustment
- Where feasible, the crossing structure should have a span that accommodates the channel's 100-year erosion limit or a lesser planning horizon determined through consultation with CVC
- The crossing should be at minimum three times the bankfull channel width for channels less than 4 m wide.
- The crossing should ensure that sediment transport processes and flow velocities are not impacted during frequent storm events

7.3 Other Crossing Considerations

The replacement, rehabilitation or modification of crossing structures must not only consider fluvial geomorphology but also hydraulics and their impacts to surrounding lands. According to the Stormwater Management Report, prepared by IBI Group (2019), Crossings 1 (Boyce's Creek) and 3 (Centreville Creek) have insufficient hydraulic capacity and therefore require replacement. **Table 5** provides a summary of existing and proposed crossing sizes at Crossings 1 and 3 to address this deficiency. Crossing 2 from this study has been omitted from the table as the existing crossing structure will be maintained.



		Existing			Proposed		
Crossing	Watercourse	Size (mm)	Туре	Length (m)	Size (mm)	Туре	Length (m)
1	Boyce's Creek	900	CSP	22.35	3658 x 1067	Open Footing Concrete Box	24.1
3	Centreville Creek	4350 x 870	Concrete Box	18.18	12192 x 1370	Open Footing Concrete Box	18.4
7	East Credit River Tributary	450	CSP	23.19	1830 x 900	Concrete Box	23.7

lable 5:	Existing	and	proposed	crossing	sizes

Fish and fish habitat must also be considered, if applicable to the crossing. Based on the aquatic habitat assessment completed by the TRCA (2017), Boyce's Creek (Crossing 1) and Centreville Creek (Crossing 3), contain direct fish habitat. Crossing 2 (Centreville Creek Tributary) was assessed by the TRCA (2017) to be a barrier to fish passage due to the crossing configuration and the length of enclosure, and therefore the piped portion of the tributary provided indirect habitat. The TRCA (2018) also concluded that the East Credit River Tributary at Crossing 7 contributes to downstream features and habitats, suggesting that it is indirect habitat.

As Boyce's Creek (Crossing 1) and Centreville Creek (Crossing 3) are considered direct fish habitat, any need for culvert replacement, rehabilitation or modification should be coupled with enhancements to channel form and function to the extent possible. In doing so, aquatic habitat conditions would also be improved.

7.4 Crossing Recommendations

The proposed spans at Crossings 1 and 3 (3.658 m and 12.192 m, respectively) are satisfactory from a fluvial geomorphological perspective. In each case, the span of the box culvert is more than three times the bankfull channel width and exceeds CVC's (2015) recommendation (it is recognized that both crossings are located in TRCA jurisdiction). Neither watercourse warrants a crossing structure that spans the meander belt due to the lack of notable channel erosion or migration observed near Airport Road as well as the conflicts with existing surrounding infrastructure that such a structure would present.

Both replacement crossing structures are open footing box culverts and these are preferred over the alternatives as they allow for placement of natural substrate for better continuity with existing bed materials beyond the ends the culvert. Open footing culverts also locally facilitate groundwater connectivity with surface flows.

In general, both crossing structures should be designed to be as short as possible so as not to deter fish from entering. This would also help to limit channel disturbance as well as the need for restoration, although larger scale channel restoration may be warranted for reasons other than culvert replacement.

At Crossing 1, the presence of a cattail marsh and the lack of defined channel on the west (upstream) side of the road indicates that it is locally a low-energy system and there is limited erosion hazard and no risks associated with channel migration. This, however, assumes that a low-flow channel will not develop over time. The marsh can be maintained by ensuring that the bed elevation through the proposed culvert is also maintained, thereby preventing increased drainage. This can be accomplished by sizing the substrate through the culvert to resist

entrainment over the expected range of stormflows, thus creating a base level control. A channel capable of conveying bankfull flows (e.g., flows associated with storms with a recurrence interval of 1.5 to 2 years) should also be established through the box culvert in order to concentrate flows and increase water depths during lower flow periods and improve conditions for fish passage.

Given the presence of a scour pool at the Crossing 1 outlet, a formal pool should be considered during detailed design. While the results of the hydraulic analysis may indicate that a scour pool is not required due to flow changes resulting from the larger culvert, it does offer other benefits such as a resting pool for fish. By using the downstream edge of the scour pool as a grade control, it can also create a backwater into the culvert. Beyond the culvert outlet, whether or not there is a scour pool, the disturbed portion of channel should be restored to a condition that ensures fish passage, preferably with habitat enhancements.

At Crossing 3, the Centreville Creek channel should be restored through the box culvert, ensuring that each bank is seamlessly aligned upstream and downstream of the culvert. The results of the hydraulic analysis can be used as a guide to determine a suitable method of achieving bank stability, keeping in mind that stability will likely not be provided by vegetation due to the lack of sunlight through the culvert. Consideration should also be given to scour prevention to avoid potential exposure of the box culvert footings. This can be accomplished by placing subsurface stones along the footings, for example.

Beyond the ends of the box culvert to the limit of channel disturbance, the banks can be bioengineered for stability and aquatic habitat benefits. While there was no strong evidence of channel migration at Airport Road, it would be prudent to promote bank stability with bioengineering, especially on the west (upstream) side of the culvert. The appropriate type of bioengineering measure can be determined largely based its anticipated long-term ability to resist degradation due to instream hydraulics.

If possible, the channel through the box culvert should be restored with substrate similar to that upstream and downstream so as not to impede movement of benthic organisms. This would also ensure that there is no disruption in sediment transport through the system.

The following additional recommendations are provided as standard best management practices:

- All work within areas regulated by the TRCA or CVC must be conducted during the appropriate in-water timing window to protect fish and fish habitat
- The in-water work area should be fully isolated to ensure that sediment is not released to the watercourse
- Any fish trapped within the isolated work area must be removed and transferred to a suitable downstream habitat by a technician with a Licence to Collect Fish for Scientific Purposes
- Natural flow levels upstream and downstream of the isolated work area must be maintained at all times
- Intake ends of pump hoses used for bypass pumping around isolated works areas must have a screen in accordance with Fisheries and Oceans Canada requirement
- Work within the isolated in-water work area should be conducted in the dry by pumping water into an approved water filtration system located at least 30 m from the receiving watercourse or other waterbody
- Minimize the area and duration of in-water works to the extent possible

8 Summary

A fluvial geomorphological assessment was completed for four of the seven regulated watercourses (associated with Crossings 1, 2, 3 and 7) that cross Airport Road between just north of Huntsmill Drive and King Street. The remaining three regulated watercourse crossings were previously assessed as headwater drainage features by the TRCA and therefore were not included in the fluvial geomorphology study. This investigation included a review of previously completed reports and secondary source information, a review of site history, meander belt width assessments (where appropriate), field reconnaissance along portions of accessible watercourse, and recommendations to be considered during the detailed design stage.

Land use within the study area has largely remained as rural residential over the period covered by historical imagery, with the gradual expansion of the community of Caledon East. With the exception of the enclosure of sections of the Centreville Creek Tributary in vicinity of Airport Road (Crossing 2) and the continued lack of a riparian buffer along the East Credit River Tributary downstream of Crossing 7, the naturalization of several former agricultural fields and the conversion of the CNR rail line to the Caledon Trailway have likely improved local channel form and aquatic and riparian habitats. Results of the field assessments indicated that all four regulated watercourses were generally stable, with limited evidence of active erosion within the extents assessed.

The meander belt widths for Boyce's Creek (Crossing 1) and Centreville Creek (Crossing 3) were determined based on a modelling approach and measurements, respectively. Boyce's Creek has a modelled width of 4.2 m, based on Williams (1986) using bankfull channel width as the independent variable, while Centreville Creek has a measured meander belt of 20 m. These meander belt widths are theoretical hazard limits and do not necessarily dictate crossing structure spans. Instead, given the lack of significant channel erosion and migration in the vicinity of Airport Road, particularly upstream of the road, the minimum recommended crossing structure spans were based on three times the bankfull channel width (CVC, 2015). In this case, the culvert dimensions deemed suitable based on hydraulic modelling exceeded the channel-width-based criterion.

The watercourses at Crossings 1 and 3 should be restored to a condition that is better than existing and more natural. Given the wider culvert spans, the channel banks can be re-established across Airport Road. This would not only help to partially restore channel form and function, but also improve habitat conditions for resident fish populations and encourage fish passage through the culverts. The recommended bed restoration strategy differs at these two crossings as the maintenance of the marsh (and prevention of box culvert footing exposure) at Crossing 1 requires a bed with materials that will be stable over the range of expected flows, while the substrate at Crossing 3 can be more natural to facilitate sediment transport.

9 References

Brierley, G.J. and Fryirs, K.A. 2005. Geomorphology and River Management: Applications of the River Styles Framework. Blackwell Publishing. Malden, MA, USA.

Chapman, L.J. and Putnam, D.F. 1984. The Physiography of Southern Ontario. Ontario Geological Survey, Special Volume 2, Map 226.

Chapman, L.J. and Putnam, D.F. 2007. Physiography of southern Ontario. Ontario Geological Survey, Miscellaneous Release—Data 228.

Credit Valley Conservation (CVC). 2002. East Credit Subwatershed Study Background Report.

Credit Valley Conservation (CVC). 2007. East Credit Subwatershed Study Phase 1 Report, Subwatershed Characterization.

Credit Valley Conservation (CVC). 2015. Credit Valley Conservation Fluvial Geomorphic Guidelines.

Downs, P.W. 1995. Estimating the probability of river channel adjustment. Earth Surface Processes and Landforms, 20: 687-705.

Galli, J. 1996. Rapid Stream Assessment Technique, Field Methods. Metropolitan Washington Council of Governments.

Ministry of the Environment (MOE). 2003. Ontario Ministry of the Environment. Stormwater Management Guidelines.

Montgomery, D.R. and Buffington, J.M. 1997. Channel-reach morphology in mountain drainage basins. Geological Society of America Bulletin (109), 5: 596-611.

Newson, M. D., and Newson, C. L. 2000. Geomorphology, ecology and river channel habitat: mesoscale approaches to basin-scale challenges. Progress in Physical Geography, 2: 195–217.

Ontario Geological Survey (OGS). 2010. Surficial geology of Southern Ontario. Ontario Geological Survey. Miscellaneous Release – Data 128-REV.

Ontario Geological Survey (OGS). 2011. 1:250 000 Scale Bedrock Geology of Ontario. Ontario Geological Survey, Miscellaneous Release ---Date 126-Revision 1.

Region of Peel. 2012. Peel Long Range Transportation Update Plan.

Toronto and Region Conservation Authority (TRCA). 2004. Belt Width Delineation Procedures.

Toronto and Region Conservation Authority (TRCA). 2008. Centreville Creek Subwatershed Study Synthesis Report.

Toronto and Region Conservation Authority (TRCA). 2015. Crossing Guideline for Valley and Stream Corridors.

Toronto and Region Conservation Authority (TRCA). 2017. Municipal Class Environmental Assessment, Airport Road from King Street to Huntsmill Drive, Natural Environment Existing Conditions Report. Prepared for the Town of Caledon.



Toronto and Region Conservation Authority (TRCA). 2018. Addendum: Aquatic Habitat Crossing Assessment, Municipal Class Environmental Assessment, Airport Road from King Street to Huntsmill Drive, Natural Environment Existing Conditions Report. Prepared for the Town of Caledon.

Ward, A. D. Mecklenberg, J. Mathews, and D. Farver. 2002. Sizing Stream Setbacks to Help Maintain Stream Stability. Paper Number: 022239. 2002 ASAE Annual International Meeting. Chicago, IL, USA. July 28-July 31, 2002

Williams, G.P. 1986. River meanders and channel size. Journal of Hydrology, 88 (1-2): 147-164.

Appendix A: Historical Aerial Imagery













Appendix B: Photographic Record



































Appendix C: Field Sheets

GEO

M O R P H I X

General Site Characteristics			Project Code: 17101
Date:		Aug. 23/18	Stream/Reach: BC-1
Weath	ner:	Sunny 26°C	Location: Airport Pld; Huntsmill Drive
Field S	Staff:	AB, CVM	Watershed/Subwatershed: Humber Diver
Field S Featur X <t< th=""><th>Staff: Pes Reach break Cross-section Flow direction Riffle Pool Medial bar Eroded bank Undercut bank Rip rap/stabilization Leaning tree Fence Culvert/outfall Swamp/wetland Grasses Tree Instream log/tree Woody debris Station location Vegetated island Ype Standing water Scarcely perceptible Smooth surface flow Upwelling Rippled Unbroken standing water Stand Gravel Small cobble Large cobble Benchmark Backsight Downstream Woody debris jam Valley wall contact Bottom of slope</th><th>e flow wave ave S6 Small boulder S7 Large boulder S7 Large boulder S8 Bimodal S9 Bedrock/till EP Erosion pin R8 Rebar US Upstream TR Terrace FC Flood chute EP Elood plain</th><th>Watershed/Subwatershed: Humber 92.0c / Site Sketch: Image: Comparison of the state of the st</th></t<>	Staff: Pes Reach break Cross-section Flow direction Riffle Pool Medial bar Eroded bank Undercut bank Rip rap/stabilization Leaning tree Fence Culvert/outfall Swamp/wetland Grasses Tree Instream log/tree Woody debris Station location Vegetated island Ype Standing water Scarcely perceptible Smooth surface flow Upwelling Rippled Unbroken standing water Stand Gravel Small cobble Large cobble Benchmark Backsight Downstream Woody debris jam Valley wall contact Bottom of slope	e flow wave ave S6 Small boulder S7 Large boulder S7 Large boulder S8 Bimodal S9 Bedrock/till EP Erosion pin R8 Rebar US Upstream TR Terrace FC Flood chute EP Elood plain	Watershed/Subwatershed: Humber 92.0c / Site Sketch: Image: Comparison of the state of the st
BUS	Bottom of slope		
TOS	Top of slope	KP Knick point	

1

GEO M C

MORPHIX Earth Sc Observa

Gen	eral Site Cha	arac	teristics		Project Code:	1/101
Date:		Au	ig 23/18		Stream/Reach:	C(-2(2/2)'CCT-2
Weath	ner:	Su	My 26°C		Location:	Airpold Rd + Monterst
Field S	Staff:	AB	CVM		Watershed/Subwatershed:	Humber River
Featur	es		1]	Site Sketch: ELL	EVATED PATHWAY
х х	Cross-section					
	Flow direction					
\sim	Riffle				C BN (S	N N
\bigcirc	Pool					
CI III	Medial bar					
 	Eroded bank				him m	
	Undercut bank					Pall
XXXXXX	Rip rap/stabilizatior	n/gabi	on		KAN BO	
	Leaning tree				1 2 1 2 2 1	PCP/MIL
xx	Fence			1	-SYNU'	011
	Culvert/outfall				XXX	
\bigcirc	Swamp/wetland				IZR - (°)	VASAL
₩₩₩	Grasses				A TRADIC R	
\bigcirc	Tree				HSPN SIL A	
	Instream log/tree				1-2 1125	
* * *	Woody debris) N / AL
못	Station location					
$\overline{\mathbb{A}}$	Vegetated island				TXXXX	
Flow T	Гуре					
H1	Standing water	_		1	AL-A	
H2	Scarcely perceptible	e flow		2		
H3	Smooth surface flow	N		and and a	1701	T MIN / C S
H4	Upwelling				(2)	TIM
H5	Rippied				TT LAN	
10	Unbroken standing	wave				OX-XIIII
п/ ЦQ	Chute	ave		1	I T (X () U S	
но	Eree fall				\mathbf{V}	ST H
Substr	rate			-		ZIN MICT
S1	Silt	S 6	Small boulder		XXXX (11) A	$\downarrow \land \downarrow \qquad \uparrow \land \downarrow \qquad \uparrow \land \downarrow \land$
S2	Sand	S7	Large boulder		₹ <u></u> , <u></u> }(
S 3	Gravel	S 8	Bimodal			~ ~ ~ ~ ~ ~ ~ ~
S 4	Small cobble	S 9	Bedrock/till		* 2 -	7 7 10 0000
S 5	Large cobble	10 ST.			* * *	I U VI VI
Other				1		
вм	Benchmark	EP	Erosion pin			AAA
BS	Backsight	RB	Rebar		* 8	E & AT
DS	Downstream	US	Upstream		X	KI & KDY
WDJ	Woody debris jam	TR	Terrace		ELIDI 121001	>11
vwc	Valley wall contact	FC	Flood chute		2010 10:00	→ I V Scale:
BOS	Bottom of slope	FP	Flood plain		Additional Notes:	
TOS	Top of slope	KP	Knick point			

. . . .

Reach Characteristics Date:: And Use And Use And Use Channel Type M V UTM (Upstream) Outmon Use And Use And Use Channel Type Channel Zon V V V M W V Land Use And Use Channel Type Channel Zon V V V M W V V Control Land Use Channel Zon Channel Zon Control <	Project Code, ream/Reach: ream/Reach: atershed/Subwatershed: TM (Downstream) TM (Downstream) TM (Downstream) TM (Downstream) TM (Downstream) Type (Table 5) Type (Table 5)	(Phase:]7](0] Yossing Yossing Lund Coverage of Reach (%) Density of WD: Place Inderate Outwater Bank Angle Bank Angle Bank Angle Duderate Inderate Indercut Indercut Indercut Indercut Indercut Indercut Indercut Indercut	GEC Sayce's (Evidence: Evidence: Evidence: Water Qu Sayce's (Mater Qu Sayce's (Mater Qu Sayce's (Sayce's (MORPH Creck Creck Creck Creck Constant Creck Constant Creck Channe DS Channe DS Channe Channe	Rootlets
Veloctity (m/s)	stimated			WW = 1.3	10,2
		Completed by: (4)	2	Checked by:	-

GEO

MORPHIX Geomorphology Earth Science Geomorpholo Earth Science Observations

Gene	eral Site Cha	irad	teristics	Project Code: [710]
Date:		Au	a.23/18	Stream/Reach: (1/2)
Weath	ier:	Su	nn 76°C	Location: Auport Restaurst
Field S	Staff:	AR	CVM	Watershed/Subwatershed:
		() ()	2001	
Featur	es Beach break			Site Sketch: A ERPORT 140/71
××	Cross-section			EL STONIA
>	Flow direction			y china sin the
\sim	Riffle			A C O C O C O N
\bigcirc	Pool			Y Y Y Y Y Y Y Y
CTTTTD	Medial bar			CHARLEN NO.
+++++++++++++++++++++++++++++++++++++++	Eroded bank			S LA MAN
4	Undercut bank			
XXXXXX	Rip rap/stabilization	i/gabi	on	
	Leaning tree			+ 2C) 7 / / /
XX	Fence			
	Culvert/outfall			VXXXXX / // ///
\bigcirc	Swamp/wetland			
WWW	Grasses			M AR A
	Tree			
	Instream log/tree			
	Station location			
(VV)	Vegetated island			
Flow T	ype			
H1	Standing water			XXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXXX
H2	Scarcely perceptible	e flow		
H3	Smooth surface flow	v		X P M M M M M
H4	Upwelling			X S S S S S S S S S S S S S S S S S S S
H5	Rippled			m(1) = 12 + 12
H6	Unbroken standing	wave		XUZXX ZIZIA
H7	Broken standing wa	ve		X X P
H8	Chute			
H9 Subst				108-6-
Substr	Silt	56	Small boulder	
S2	Sand	57	Large boulder	KVXXVV BALLES J 4
S3	Gravel	S8	Bimodal	
S4	Small cobble	S9	Bedrock/till	X X D J L
S 5	Large cobble			
Other				
вм	Benchmark	EP	Erosion pin	
BS	Backsight	RB	Rebar	Duffein
DS	Downstream	US	Upstream	Avenue () ()
WDJ	Woody debris jam	TR	Terrace	m F m Y N
vwc	Valley wall contact	FC	Flood chute	Scale:
BOS	Bottom of slope	FP	Flood plain	Additional Notes:
TOS	Top of slope	KP	Knick point	

Date:	AU	9,23/2018	Stream/Reach:	Stream/Reach:			
Weather:		0	Watershed/Subwatershed:	Watershed/Subwatershed: Humbry			
Field Staff:	A	horphic Assessment Project Code: MAR Aug A3 / 2018 Stream/Reach: CC-2 Watershed/Subwatershed: Mumbra AB CVM Location: March Location: Mumbra AB CVM Location: Geomorphological Indicator Mumbra No. Description Lobate bar 2 Coarse materials in riffles embedded 3 3 Siltation in pools 4 4 Medial bars Accretion on point bars 6 Poor longitudinal sorting of bed materials 7 7 Deposition in the overbank zone Sum of indices = 1 Exposed bridge footing(s) 2 Exposed sanitary / storm sewer / pipeline / etc. 3 Elevated storm sever outfall(s) 4 4 4 Undermined gabion baskets / concrete aprons / etc. 5 5 Scour pools downstream of culverts / storm sewer outlets 6 6 Cut face on bar forms Sum of indices = 7 Head cutting due to knickpoint migration 8 8 Terace cut through older bar material	Cal	(an)			
	Geomorphic Assessment Aug 23/2018 r: aff: AB CVM ss No. Description 1 Lobate bar 2 Coarse materials in r 3 Siltation in pools 4 Medial bars 5 Accretion on point ba 6 Poor longitudinal sort 7 Deposition in the ove 1 Exposed bridge footin 2 Exposed sanitary / st 3 Elevated storm sewe 4 Undermined gabion to 5 Scour pools downstre 6 Cut face on bar form 7 Head cutting due to I 8 Terrace cut through 9 Suspended armour la 10 Channel worn into ur 1 Fallen / leaning trees 2 Occurrence of large of 3 Exposed tree roots 4 Basal scour on both si 6 Outflanked gabion ba 7 Length of basal scou 8 Exposed length of pr 9 Fracture lines along 1 10 Exposed building fou 11 Formation of chute(s) 5 Formation of island(s) 5 Formation of island(s) 5 Bar forms poorly form	eomorphological Indicator	- + Ji V	Dro	sent?	Factor	
Process	No	Description	comorphological indicator		Yes	No	Value
	1	Lobate bar		2	105		
	2	Coarse materials in r	iffles embedded			-	
Evidence of	3	Siltation in pools					-
Aggradation	4	Medial bars					-
(AI)	5	Accretion on point ba	ars			/	-
	6	Poor longitudinal sor	ting of bed materials				1
	7	Deposition in the over	erbank zone	e se la company de la company	1		7
7 Deposition in the overbank zone Sum of indices = 2 1 Exposed bridge footing(s)						5	617
					6		
	1	Exposed bridge footi	ng(s)				-
	2	Exposed sanitary / s	torm sewer / pipeline / etc.				-
	3	Elevated storm sewe	r outfall(s)				-
Evidence of	4	Undermined gabion I	baskets / concrete aprons / etc.		/		
Degradation	5	Scour pools downstr	eam of culverts / storm sewer outlets	8			-
Evidence of Degradation (DI)	6	Cut face on bar form					
	/	Head cutting due to					
	8	Terrace cut through	older bar material				-
	9	Suspended armour la	ayer visible in bank		110	-	-
	10			m of indices -	1	E.	O JAF
		1	34	in or indices –		0	0.15
	1	Fallen / leaning trees	s / fence posts / etc.		-		
	2	Occurrence of large	organic debris			/	-
	3	Exposed tree roots			×		-
Evidence of	4	Basal scour on inside	e meander bends			/	4
Widening	5	Basal scour on both	sides of channel through riffle			/	-
(WI)	6	Outflanked gabion ba	askets / concrete walls / etc.	electronic de la contra de la c		/	
	7	Length of basal scou	r >50% through subject reach	1) 			-
	8	Exposed length of pr	eviously buried pipe / cable / etc.				-
	9	Fracture lines along	top of bank				
	10	Exposed building fou	ndation			-	0
			Su	m of indices =		-/	0.125
	1	Formation of chute(s	;)			Summaria and a summaria	
E. islama of	2	Single thread channe	el to multiple channel			and an and a second	1
Evidence of Planimetric	3	Evolution of pool-riff	le form to low bed relief form			Standard Constanting State	1
Form	4	Cut-off channel(s)				Nandaranananan	1
Evidence of Planimetric Form Adjustment (PI)	5	Formation of island(s	5)				
	6	Thalweg alignment c	out of phase with meander form			/	
		1					1
	7	Bar forms poorly for	med / reworked / removed				

i.

1.12 In Adjustment Condition In Regime In Transition/Stress 问 0.00 - 0.20 0.41 SI score = □ 0.21 - 0.40

Completed by: AB____ Checked by: ____

Date:	RUG 25 / 2018	Stream/Reach:		CC-1	<u>(110,10,0</u>)
weather:	SUNAY	Location:		Caledon 100	ul attillent
Field Staff:	AB WM	Watershed/Subwate	rshed:	Humber	
Evaluation Category	Poor	Fair		Good	Excellent
	 < 50% of bank network stable Recent bank sloughing, slumping or failure frequently observed 	 50-70% of bank network stable Recent signs of bank sloughing, slumping or failure fairly common 	 71-80% stable Infreque sloughir failure 	o of bank network ent signs of bank ng, slumping or	 > 80% of bank network stable No evidence of bank sloughing, slumping or failure
Channeł	 Stream bend areas highly unstable Outer bank height 1.2 m above stream bank (2.1 m above stream bank for large mainstem areas) Bank overhang > 0.8-1.0 m 	 Stream bend areas unstable Outer bank height 0.9- 1.2 m above stream bank (1.5-2.1 m above stream bank for large mainstem areas) Bank overhang 0.8-0.9m 	 Stream Outer b m above 1.5 m a for large Bank ove 	bend areas stable ank height 0.6-0.9 e stream bank (1.2 bove stream bank e mainstem areas) verhang 0.6-0.8 m	 Stream bend areas very stable Height < 0.6 m above stream (< 1.2 m above stream bank for large mainstem areas) Bank overhang < 0.6 m
Stability	 Young exposed tree roots abundant > 6 recent large tree falls per stream mile 	 Young exposed tree roots common 4-5 recent large tree falls per stream mile 	 Exposed predom large, s scarce 2-3 rece per stree 	d tree roots inantly old and maller young roots ent large tree falls am mile	 Exposed tree roots old, large and woody Generally 0-1 recent large tree falls per stream mile
	 Bottom 1/3 of bank is highly erodible material Plant/soil matrix severely compromised 	 Bottom 1/3 of bank is generally highly erodible material Plant/soil matrix compromised 	 Bottom general plant/sc 	1/3 of bank is y highly resistant il matrix or material	 Bottom 1/3 of bank is generally highly resistant plant/soil matrix or material
	 Channel cross-section is generally trapezoidally- shaped 	 Channel cross-section is generally trapezoidally- shaped 	Channe general	V- or U-shaped	 Channel cross-section is generally V- or U-shaped
Point range	0 0 1 0 2	□ 3 □ 4 □ 5	□ 6	07 <u>/</u> 8	□ 9 □ 10 □ 11
	 > 75% embedded (> 85% embedded for large mainstem areas) 	 50-75% embedded (60 85% embedded for large mainstem areas) 	• 25-49% 59% en mainste	embedded (35- nbedded for large m areas)	 Riffle embeddedness < 25% sand-silt (< 35% embedded for large mainstem areas)
ĸ	 Few, if any, deep pools Pool substrate composition >81% sand- silt 	 Low to moderate number of deep pools Pool substrate composition 60-80% sand-silt 	 Moderati pools Pool sub 30-59% 	te number of deep ostrate composition o sand-silt	 High number of deep pools (> 61 cm deep) (> 122 cm deep for large mainstem areas) Pool substrate composition <30% sand-silt
Channel Scouring/ Sediment Deposition	 Streambed streak marks and/or "banana"-shaped sediment deposits common 	 Streambed streak marks and/or "banana"-shaped sediment deposits common 	 Streaml and/or sedimer uncomn 	bed streak marks 'banana"-shaped ht deposits hon	 Streambed streak marks and/or "banana"-shaped sediment deposits absent
	 Fresh, large sand deposits very common in channel Moderate to heavy sand deposition along major portion of overbank area 	 Fresh, large sand deposits common in channel Small localized areas of fresh sand deposits along top of low banks 	 Fresh, li uncomh Small lo fresh sa top of k 	arge sand deposits non in channel calized areas of nd deposits along w banks	 Fresh, large sand deposits rare or absent from channel No evidence of fresh sediment deposition on overbank
	 Point bars present at most stream bends, moderate to large and unstable with high amount of fresh sand 	 Point bars common, moderate to large and unstable with high amount of fresh sand 	• Roint ba well-veg armoure fresh sa	rrs small and stable, getated and/or ed with little or no nd	 Point bars few, small and stable, well-vegetated and/or armoured with little or no fresh sand
Point range		□ 3 □ 4	9	5 🗆 6	□ 7 □ 8

 $_{L} \sim \chi$

.

Date:	Aug. 23/2018	Reach:	-1	Project Code:	1-	7101
Evaluation Category	Poor	Fair		Good		Excellent
	 Wetted perimeter < 40% of bottom channel width (< 45% for large mainstem areas) 	 Wetted perimeter 40 60% of bottom chan width (45-65% for la mainstem areas) 	- • Wetted p of bottom rge (66-90% mainstem	erimeter 61-85% channel width for large areas)	• Wett of bo 90% area	ed perimeter > 85% ottom channel width (> for large mainstem s)
	 Dominated by one habitat type (usually runs) and by one velocity and depth condition (slow and shallow) (for large mainstem areas, few riffles present, runs and pools dominant, velocity and depth diversity low) 	 Few pools present, ri and runs dominant. Velocity and depth generally slow and shallow (for large mainstem areas, run and pools dominant, velocity and depth diversity intermediate 	ffles • Good mix runs and • Relatively and depties e)	between riffles, pools diverse velocity o of flow	 Riffle habit Dive of flo fast, wate 	es, runs and pool tat present rse velocity and depth ow present (i.e., slow, shallow and deep or)
Physical Instream	 Riffle substrate composition: predominantly gravel with high amount of sand < 5% cobble 	 Riffle substrate composition: predominantly small cobble, gravel and sa 5-24% cobble 	Riffle sub compositi gravel, co material • 25-49%	strate on: good mix of bble, and rubble	 Rifflection complexity grav with > 50 	e substrate position: cobble, el, rubble, boulder mix little sand % cobble
Habitat	 Riffle depth < 10 cm for large mainstem areas 	Riffle depth 10,15 cn large mainstern areas	n for • Riffle dep large mai	th 15-20 cm for nstem areas	• Riffle large	e depth > 20 cm for e mainstem areas
	 Large pools generally < 30 cm deep (< 61 cm for large mainstem areas) and devoid of overhead cover/structure 	 Large pools generally 46 cm deep (61-91 c for large mainstem areas) with little or n overhead cover/struct 	30 • Large poo cm deep large mai some ove ture cover/str	ols generally 46-61 (91-122 cm for nstem areas) with rhead ucture	 Large cm c large good cove 	e pools generally > 61 leep (> 122 cm for e mainstem areas) with l overhead r/structure
	 Extensive channel alteration and/or point bar formation/enlargement 	Moderate amount of channel alteration an moderate increase in point bar formation/enlargeme	d/or f/or nt	ount of channel and/or slight n point bar /enlargement	• No c signi form	hannel alteration or ficant point bar ation/enlargement
	 Riffle/Pool ratio 0.49:1 ; ≥1.51:1 	• Riffle/Pool ratio 0.5- 0.69:1 ; 1.31-1.5:1	• Riffle/Poc ; 1.11-1.	l ratio 0.7-0.89:1 3:1	• Riffle	e/Pool ratio 0.9-1.1:1
	 Summer afternoon water temperature > 27°C 	 Summer afternoon w temperature 24-27°C 	ater • Summer temperat	afternoon water ure 20-24°C	• Sum temp	mer afternoon water perature < 20°C
Point range	00102	□ 3 □ 4	A	5 🗆 6		□ 7 □ 8
	 Substrate fouling level: High (> 50%) 	 Substrate fouling leven Moderate (21-50%) 	el: Very light	fouling level: (11-20%)	 Subs Rock 	strate fouling level: underside (0-10%)
Water Quality	Brown colourTDS: > 150 mg/L	Grey colourTDS: 101-150 mg/L	 Slightly g TDS: 50- 	rey colour 100 mg/L	 Clease TDS 	r flow (< 50 mg/L
water Quality	 Objects visible to depth < 0.15m below surface 	Objects visible to dep 0.15-0.5m below sur	oth face • Objects v 0.5-1.0m	isible to depth below surface	• Obje > 1.	cts visible to depth Om below surface
	 Moderate to strong organic odour 	 Slight to moderate organic odour 	• Slight org	anic odour	• No o	dour
Point range	0 0 1 0 2	□ 3 □ 4		5 2 6		
Riparian	 Narrow riparian area of mostly non-woody vegetation 	 Riparian area predominantly woode but with major localiz gaps 	• Rorested • 31 m v • ortion o	buffer generally ride along major Fboth banks	• Wide fores bank	e (> 60 m) mature sted buffer along both ss
Conditions	 Canopy coverage: <50% shading (30% for large mainstem areas) 	 Canopy coverage: 50 60% shading (30-44 for large mainstem areas) 	• Canopy e % 60-79% s for large	overage: hading (45-59% mainstern areas)	• Cano >80º large	opy coverage: % shading (> 60% for e mainstem areas)
Point range	□ 0 □ 1			4 🗆 5		□ 6 □ 7
Total overall s	core (0-42) = 28	Poor (<13)	Fair (13-24)	Good (25-3	34)	Excellent (>35)

1

GEO MORPHIX

General Site Char	acteristics	Project Code: און ויסן
Date:	Aug. 23/2018	Stream/Reach: CCT-1
Weather:	Sunny 24°C	Location: Ella St. ; Ivan ac
Field Staff:	HB, CVM	Watershed/Subwatershed: Humber River
Features Reach break Cross-section Flow direction Riffle Pool Medial bar ####### Eroded bank Image: Section of the sect	gabion Jow low ave ave ave ave b S6 Small boulder S7 Large boulder S8 Bimodal S9 Bedrock/till EP Erosion pin R8 Rebar JS Upstream TR Terrace EC Flood chute EP Flood plain	Site Sketch: Site Sketch: Site Sketch: Site Sketch: Site Sketch: Site Sketch: Site Sketch: Site Sketch:
TOS Top of slope	(P Knick point	

	а .		GEO MORPHIX
Reach Charact	eristics	Project Code: 1710	Germanphology Earth Science Observations
Date:	8192318	Stream/Reach:	
Weather:	www.	Location: Airport Oropol	I Lla court
Field Staff:	AB OWM	Watershed/Subwatershed:	
UTM (Upstream)		UTM (Downstream)	
Land Use (Table 1)	ey Type Channel Type Channel Z Table 2) (Table 3) (Tab	Zone Flow Type DGroundwater ble 4) (Table 5) D	Evidence:
Riparian Vegetation		Aquatic/Instream Vegetation	Water Quality
Dominant Type: Cover.	age: ^{Channel} Age Class (yrs): Encroachmeni widths 1-4	nt: Type (Table8) Coverage of Reach (%)	Odour (Table 16)
Species:	igmented \Box 4-10 ∇ Established (5-30) \Box ntinuous $\lambda > 10$ \Box Mature (>30)	Present in Cutbank Low WDJ/50m: Present in Channel Moderate Not Present High	Turbidity (Table 17)
Channel Characteristics			
Sinuosity (Type)	Sinuosity (Degree) Gradient Num	nber of Channels Clay/Silt Sand Grav	l Cobble Boulder Parent Rootlet
(Table 9)	(Table 10) \mathcal{C} (Table 11) $igvee$ (Tab	ble 12) \bigcirc Riffle Substrate \Box \bigtriangledown	
Entrenchment	Type of Bank Failure Downs's Classification	Pool Substrate	
(Table 13)	(Table 14) (Table 15)	Bank Material	
Bankfull Width (m)	1.2 0,9 0,9 Wetted Width (m)	0.63 0.55 0.6 Bank Angle Bar	Erosion Notes:
Bankfull Depth (m)	0.45 0.35 0.30 Wetted Depth (m)	0.125 0.08 0.1	5% - 30%
Riffle/Pool Spacing (m)	% Riffles:	Meander Amplitude:	0 - 60%
Pool Depth (m)	0.45 Riffle Length (m) Undercuts (m)	Comments:	
Velocity (m/s)	Image: State of the second	// Estimated	4 2
		Completed hv.	Charked hv.

Date:	AUG	23/2018	Strea	m/Reach:	CCT-1		2. 00 %	ti dan n
Neather:	al		Wate	rshed/Subwatersh	ed: Dik por-	- Ron	1 151	1. 101.
	SUV	ing Duna	Local	ion:	1 himba	1	o ev	la UM
	HE	> WM	LUCA	.1011.	Humbe	/		
Process		G	eomorpholog	ical Indicator		P	resent?	Factor
1100033	No.	Description		50 6 10 10 10 10 10 10 10 10 10 10 10 10 10		Yes	No	Value
	1	Lobate bar					1	
	2	Coarse materials in r	iffles embed	ded				
Evidence of	3	Siltation in pools						JA4
Aggradation (AI)	4	Medial bars						0.0
	5	Accretion on point ba	nrs					
	6	Poor longitudinal sor	ting of bed n	naterials		1	1	
	7	Deposition in the ove	rbank zone				1	
					Sum of indices	= 3	Æ	
	1	Exposed bridge footi	na(s)					
	2	Exposed sanitary / st	orm sower	nineline / etc				-
	2	Flevated storm come	r outfall(c)	pipeline / etc.				-
Evidence of Degradation (DI)		Undermined gabion	askets / cor	crete aprons / etc		-		-
	5	Scour pools downstre			- ()			
	6	Cut face on har form				$- \bigcirc$		
		Head cutting due to	s knickpoint m	igration			~	-
	8	Terrace cut through	older har ma	terial				-
	9	Suspended armour la	ace°cut through older bar material					
	10	Channel worn into ur	disturbed or	verburden / bedrock				-
	10	Channel Work Inco a		verburden y bedrock	Sum of indices	=	5	
annan a 2na	1	Fallen / leaning trees	s / fence pos	ts / etc.				
	2	Occurrence of large	prganic debr	S				1
	3	Exposed tree roots			in a second s		1	-
	4	Basal scour on inside						
Evidence of	5	Basal scour on both			7 ()			
Widening (WI)	6	Outflanked gabion ba						
(001)	7	Length of basal scou	igth of basal scour >50% through subject reach					
	8	Exposed length of pr	angth of basal scour >50% through subject reach (posed length of previously buried pipe / cable / etc. (acture lines along top of bank					
	9	Fracture lines along						
	10	Exposed building fou	ndation					
	1	dan se in an in a se in			Sum of indices	= ()	6	
	1	Formation of chute(s	;)					
	2	Single thread channe	el to multiple	channel				-
Evidence of	3	Evolution of pool-riff			\neg			
Form	4	Cut-off channel(s)						
Adjustment	5	Formation of island(-			
(PI)	6	Thalweg alignment of	ut of phase	with meander form				-
	7	Bar forms poorly for	med / rewor	ked / removed				-
	I		,		Sum of indices	= 0	7	
Additional note	es:			Stability In	dex (SI) = (AI	+DI+WI	(+PI)/4 =	1/15
			Condition	In Regime	In Transition/	Stress	In Adju	ustment

Completed by: 115

_ Checked by: ____

Rapid Stream Assessment Technique				Project Code: (710)		
Date:	Aug 23/2018	Stream/Reach: Location: Watershed/Subwatershed:		Airport-Road, Ella Court Humber		
Weather:	Sonny					
Field Staff:	AB'CUM					
Evaluation Category	Poor	Fair		Good	Excellent	
	 < 50% of bank network stable Recent bank sloughing, slumping or failure frequently observed 	 50-70% of bank network stable Recent signs of bank sloughing, slumping or failure fairly common 	 71-80% of bank network stable Infrequent signs of bank sloughing, slumping or failure Stream bend areas stable Outer bank height 0.6-0.9 m above stream bank (1.2- 1.5 m above stream bank for large mainstem areas) Bank overhang 0.6-0.8 m 		 > 80% of bank network stable No evidence of bank sloughing, slumping or failure 	
Ċhannel	 Stream bend areas highly unstable Outer bank height 1.2 m above stream bank (2.1 m above stream bank for large mainstem areas) Bank overhang > 0.8-1.0 m 	 Stream bend areas unstable Outer bank height 0.9- 1.2 m above stream bank (1.5-2.1 m above stream bank for large mainstem areas) Bank overhang 0.8-0.9m 			 Stream bend areas very stable Height < 0.6 m above stream (< 1.2 m above stream bank for large mainstem areas) Bank overhang < 0.6 m 	
Stability	 Young exposed tree roots abundant > 6 recent large tree falls per stream mile 	 Young exposed tree roots common 4-5 recent large tree falls per stream mile 	 Exposed tree roots predominantly old and large, smaller young roots scarce 2-3 recent large tree falls per stream mile Exposed tree roots old, large and woody Generally 0-1 recent large tree falls per stream mile 			
	 Bottom 1/3 of bank is highly erodible material Plant/soil matrix severely compromised 	 Bottom 1/3 of bank is generally highly erodible material Plant/soil matrix compromised 	 Bottom 1/3 of bank is generally highly resistant plant/soil matrix or material Bottom 1/3 of bank is generally highly resistant plant/soil matrix or material 			
	 Channel cross-section is generally trapezoidally- shaped 	 Channel cross-section is generally trapezoidally- shaped 	 Channe general 	el cross-section is lly V- or U-shaped	 Channel cross-section is generally V- or U-shaped 	
Point range	00102	· □ 3 □ 4 □ 5		5 🗆 7 🗆 8	9 0 10 0 11	
Scouring/ Sediment Deposition	 > 75% embedded (> 85% embedded for large mainstem areas) 	 50-75% embedded (60- 85% embedded for large mainstem areas) 	 25-49% embedded (35- 59% embedded for large mainstem areas) 		 Riffle embeddedness < 25% sand-silt (< 35% embedded for large mainstem areas) 	
	 Few, if any, deep pools Pool substrate composition >81% sand- silt 	 Low to moderate number of deep pools Pool substrate composition 60-80% sand-silt 	 Moderate number of deep pools Pool*substrate composition 30-59% sand-silt 		 High number of deep pools 61 cm deep) 122 cm deep for large mainstem areas) Pool substrate composition <30% sand-silt 	
	 Streambed streak marks and/or "banana"-shaped sediment deposits common 	 Streambed streak marks and/or "banana"-shaped sediment deposits common 	Streambed streak marks and/or "banana"-shaped sediment deposits uncommon		 Streambed streak marks and/or "banana"-shaped sediment deposits absent 	
	 Fresh, large sand deposits very common in channel Moderate to heavy sand deposition along major portion of overbank area 	 Fresh large sand deposits common in channel Small localized areas of fresh sand deposits along top of low banks 	 Fresh, large sand deposits uncommon in channel Small localized areas of fresh sand deposits along top of low banks 		 Fresh, large sand deposits rare or absent from channel No evidence of fresh sediment deposition on overbank 	
	 Point bars present at most stream bends, moderate to large and unstable with high amount of fresh sand 	 Point bars common, moderate to large and unstable with high amount of fresh sand 	 Point bars small and stable, well-vegetated and/or armoured with little or no fresh sand 		 Point bars few, small and stable, well-vegetated and/or armoured with little or no fresh sand 	
Point range		□ 3 à 4	È	□ 5 □ 6	□ 7 □ 8	

Date:	(Jug. 23/2018	Reach: CCT-1	Project Code:	17101
Evaluation Category	Poor	Fair	Good	Excellent
	 Wetted perimeter < 40% of bottom channel width (< 45% for large mainstem areas) 	 Wetted perimeter 40- 60% of bottom channel width (45-65% for large mainstem areas) 	 Wetted perimeter 61-85% of bottom channel width (66-90% for large mainstem areas) 	 Wetted perimeter > 85% of bottom channel width (> 90% for large mainstem areas)
	• Dominated by one habitat type (usually runs) and by one velocity and depth condition (slow and shallow) (for large mainstem areas, few riffles present, runs and pools dominant, velocity and depth diversity low)	 Few pools present, riffles and runs dominant. Velocity and depth generally slow and shallow (for large mainstem areas, runs and pools dominant, velocity and depth diversity intermediate) 	 Good mix between riffles, runs and pools Relatively diverse velocity and depth of flow 	 Riffles, runs and pool habitat present Diverse velocity and depth of flow present (i.e., slow, fast, shallow and deep water)
Physical Instream Habitat	 Riffle substrate composition: predominantly gravel with high amount of sand < 5% cobble 	 Riffle substrate composition: predominantly small cobble, gravel and sand 5-24% cobble 	 Riffle substrate composition: good mix of gravel, cobble, and rubble material 25-49% cobble 	 Riffle substrate composition: cobble, gravel, rubble, boulder mix with little sand > 50% cobble
	• Riffle depth < 10 cm for large mainstem areas	• Riffle depth 10-15 cm for large mainstem areas	 Riffle depth 15-20 cm for large mainstem areas 	 Riffle depth > 20 cm for large mainstem areas
	Large pools generally < 30 cm deep (< 61 cm for large mainstem areas) and devoid of overhead cover/structure	Large pools generally 30- 46 cm deep (61-91 cm for large mainstem areas) with little or no overhead cover/structure	Large pools generally 46-61 cm deep (91-122 cm for large mainstem areas) with some overhead cover/structure	 Large pools generally > 61 cm deep (> 122 cm for large mainstem areas) with good overhead cover/structure
	• Extensive channel alteration and/or point bar formation/enlargement	Moderate amount of channel alteration and/or moderate increase in point bar formation/enlargement	Slight amount of channel alteration and/or slight increase in point bar formation/enlargement	No channel alteration or significant point bar formation/enlargement
	• Riffle/Pool ratio 0.49:1 ; ≥1.51:1	 Riffle/Pool ratio 0.5- 0.69:1 ; 1.31-1.5:1 	• Riffle/Pool ratio 0.7-0.89:1 ; 1.11-1.3:1	Riffle/Pool ratio 0.9-1.1:1
	 Summer afternoon water temperature > 27°C 	 Summer afternoon water temperature 24-27°C 	Summer afternoon water temperature 20-24°C	• Summer afternoon water temperature < 20°C
Point range	0 0 1 0 2	口 3 瓦 4	0506	0708
Water Quality	Substrate fouling level: High (> 50%)	Substrate fouling level: Moderate (21-50%)	• Substrate fouling level: Very light (11-20%)	Substrate fouling level: Rock underside (0-10%)
	 Brown colour TDS: > 150 mg/L 	• Grey colour • TDS: 101-150 mg/L	 Slightly grey colour TDS: 50-100 mg/L 	Clear flow TDS: < 50 mg/L
	Objects visible to depth < 0.15m below surface	• Objects visible to depth 0.15-0.5m below surface	• Objects visible to depth 0.5-1.0m below surface	Objects visible to depth > 1.0m below surface
	Moderate to strong organic odour	 Slight to moderate organic odour 	Slight organic odour	• No odour
Point range	□ 0 □ 1 □ 2	□ 3 □ 4	口 5 飞 6	□ 7 □ 8
Riparian Habitat Conditions	 Narrow riparian area of mostly non-woody vegetation 	 Riparian area predominantly wooded but with major localized gaps 	 Forested buffer generally > 31 m wide along major portion of both banks 	 Wide (> 60 m) mature forested buffer along both banks
	 Canopy coverage: <50% shading (30% for large mainstem areas) 	 Canopy coverage: 50- 60% shading (30-44% for large mainstem areas) 	 Canopy coverage: 60-79% shading (45-59% for large mainstem areas) 	• Canopy coverage: >80% shading (> 60% for) large mainstem areas)
Point range	0 1	□ 2 □ 3	□4 25	0607
Total overall s	score $(0-42) = 26$	Poor (<13) F	air (13-24) Good (25-	34) Excellent (>35)

GEO

Project Code: 17101 **General Site Characteristics** Date: 2101 Stream/Reach: 121 irport Rand; Olde Baseline Weather: 600 Location: Field Staff: redi Watershed/Subwatershed: CVM VOVI Features Site Sketch: Reach break -Cross-section Flow direction Ν Riffle Pool CHILD . Medial bar V Eroded bank 0 ----Undercut bank 1 **XXXXXX** Rip rap/stabilization/gabion Leaning tree × × × Fence Culvert/outfall 1 1 0 Swamp/wetland Grasses WWW C Tree Instream log/tree ₭ ₭ ₭ Woody debris 尺 Station location V Vegetated island Flow Type H1 Standing water H2 Scarcely perceptible flow H3 Smooth surface flow Upwelling H4 H5 Rippled H6 Unbroken standing wave H7 Broken standing wave H8 Chute H9 Free fall Driveway Substrate **S1** Silt **S6** Small boulder **S2** Sand **S7** Large boulder **S**3 Gravel **S8** Bimodal 11CH-1WA **S**4 Small cobble **S9** Bedrock/till **S5** Large cobble 105 Other d BM Benchmark Erosion pin EP BS Backsight RB Rebar DS Downstream US Upstream WDJ Woody debris jam TR Terrace vwc Valley wall contact FC Flood chute Scale: BOS Bottom of slope FP Flood plain Additional Notes: TOS Top of slope KP Knick point

m DE 0 44000 m SHO 0 44000 5 0.25 Rootlets Notes: Wet merdaw Ö КІХ Wtp, m NICH Turbidity (Table 17) Fedbur width Parent Odour (Table 16) ۵. R Checked by: Geomorphology Earth Science Observations 0 Boulder \mathcal{S} Σ Water Quality GEO Cobble 1200051 Evidence: **Bank Erosion** \Box 60 - 100% 09 - 02 0% □ 5 – 30% 5 N < 5% 15324 12 MOONT Gravel Coverage of Reach (%) $|q_{()}\rangle$ WDJ/50m: 0 (KIVEN Completed by: Crossing 7 □ Undercut Groundwater Sand **Bank Angle** D30-60 06 - 09 🗆 0 – 30 Density of WD: Moderate Clay/Silt MOTO 🗆 High YCd1 Project Code: 7101 0 Aquatic/Instream Vegetation **Riffle Substrate Pool Substrate** Flow Type (Table 5) \ll Present in Cutbank Present in Channel 4 **Bank Material** 1.86 Watershed/Subwatershed: 210 Woody Debris Z Not Present Meander Amplitude: Type (Table8) Comments: UTM (Downstream) 54.0 0.10 Stream/Reach: Number of Channels Wiffle ball / ADV / Estimated 3 4 Location: (Table 12) ct'o 0.09 Channel Zone (Table 4) (Table 7) Encroachment: 2 Undercuts (m) Wetted Width (m) Wetted Depth (m) Downs's Classification % Pools: Established (5-30) Immature (<5)</p> 19 Mature (>30) Age Class (yrs): (Table 11) (Table 15) Gradient Channel Type (Table 3) 5.0 23 Riffle Length (m) % Riffles: Type of Bank Failure 26°0 23/2018 3.35 Sup t.O Sinuosity (Degree) 4-10 □ > 10 1-4 Channel widths (Table 14) (Table 10) $\leq |$ N/N 0.6 Valley Type (Table 2) Fragmented Continuous **Reach Characteristics** 2.4 SUN AND. M Coverage: None Riffle/Pool Spacing (m) **Channel Characteristics Riparian Vegetation** £. Bankfull Width (m) Bankfull Depth (m) Top of bomk UTM (Upstream) Sinuosity (Type) Pool Depth (m) (Table 9) Entrenchment Velocity (m/s) Dominant Type: (Table 13) m) Land Use (Table 1) Field Staff: Weather: (Table 6) Species: Date:



Appendix D: Meander Belt Width Assessment

