

Appendix A

Safety Performance Functions

The Safety Performance Function was calculated based upon the Regional Municipality of Peel's functions, summarized below:

Peel Region's Safety Performance Functions for 4-legged Signalized Intersections

	Equation	Ln(α)	b	c	k	Pearson Chi-Square
All Collisions	$E(Y) = \alpha \times F_{maj}^b \times F_{min}^c$	-11.6363	0.6546	0.7817	0.2820	1.1146
Severe	$E(Y) = \alpha \times F_{maj}^b \times F_{min}^c$	-11.2613	0.5507	0.6644	0.2604	1.0973
PDO	$E(Y) = \alpha \times F_{maj}^b \times F_{min}^c$	-12.4550	0.6950	0.8049	0.2856	1.1131

The functions for "All Collisions", "Severe" and "PDO" were used to determine the predicted number of collisions. The traffic volumes were developed as follows:

- F_{maj} : The sum of the eastbound and westbound AADT's on Derry Road; the eastbound AADT came from a location west of the intersection and the westbound AADT came from a location to the east of the intersection
- F_{min} : The sum of the northbound and southbound AADT's on Argentia Road; the northbound AADT came from a location south of the intersection and the southbound AADT came from a location to the north of the intersection.

The AADT counts are shown in **Exhibit A-1**. The Derry Road AADT counts are from Peel Region's 2011 and 2012 AADT data, and the Argentia Road AADT counts are from the City of Mississauga's 2012 AADT data.

		5652			
		↓			
			←	14368	
					Fmajor
					27274
<hr/>					
		12906	→		
			↑		
				4580	
					Derry Road
					Argentia Road
					Fminor
					10232

Exhibit A-1: AADT Volumes on Derry Road and Argentia Road

The calculation of the predicted and expected number of collisions is shown below.

Collision Data

Years 5

All Collisions	
	Derry and Argentia
Observed Col	82
Fmaj (Derry)	27274
Fmin (Argentia)	10232
Step 1	
Predicted Col/yr	9.65
Predicted Col	48.26
Step 2	
Weights	0.07
Step 3	
Expected	79.69
Severe (Injury)	
	Derry and Argentia
Observed Col	12
Fmaj (Derry)	27274
Fmin (Argentia)	10232
Step 1	
Predicted Col/yr	1.65
Predicted Col	8.227
Step 2	
Weights	0.32
Step 3	
Expected	10.80
PDO	
Predicted Col	40.04
Expected	68.89

Appendix B

Existing Signal Timing Plans

REGIONAL MUNICIPALITY OF PEEL

Traffic Signal Timing Parameters

Database Date		October 1, 2012		Prepared Date:		September 9, 2013			
Database Rev		34		Completed By:		MF			
Timing Card / Field rev		8		Checked By:		MF			
Location:		Derry Rd @ Argentia Rd				TIME PERIOD (sec.) (Green+Amber+All Red)			
Phase #	Direction	Vehicle Minimum (sec.)	Pedestrian Minimum (sec.)		Amber (sec.)	All Red (sec.)	AM MAX	OFF MAX	PM MAX
			WALK	FDWALK					
1	WBLT - Derry	5.0			3.0		25.2	9.0	11.2
2	E/B - Derry	8.0	12.0	19.0	4.0	2.1	61.6	39.6	61.6
3	SBLT - Argentia	5.0			3.0		11.2	-	16.8
4	N/B - Argentia	8.0	12.0	20.0	4.0	3.0	42.0	41.4	50.4
5	Not in Use								
6	W/B - Derry	8.0	12.0	19.0	4.0	2.1	86.8	48.6	72.8
7	NBLT - Argentia	5.0			3.0		9.8	-	25.2
8	S/B -Argentia	8.0	12.0	20.0	4.0	3.0	43.4	41.4	42.0
System Control		YES							
Local Control		NO							
Semi-Actuated Mode		YES							
				TIME (M-F)		PEAK	CYCLE LENGTH (sec.)		OFFSET (sec.)
				06:00-09:30		AM	140		95.2
				09:30-15:00		OFF	90		15.3
				15:00-19:30		PM	140		81.2

REGIONAL MUNICIPALITY OF PEEL

Traffic Signal Timing Parameters

Database Date		September 14, 2011		Prepared Date:		September 9, 2013			
Database Rev		41		Completed By:		MF			
Timing Card / Field rev		21		Checked By:		MF			
Location:		Derry Rd @ Winston Churchill Blvd					TIME PERIOD		
Phase #	Direction	Vehicle Minimum (sec.)	Pedestrian Minimum (sec.)		Amber (sec.)	All Red (sec.)	(sec.)		
			WALK	FDWALK			(Green+Amber+All Red)		
							AM MAX	OFF MAX	PM MAX
1	WBLT - Derry	5.0			3.0		14.0	-	21.0
2	E/B - Derry	8.0	11.0	17.0	4.0	2.9	61.6	61.2	53.2
3	SB/ Prot LT - W.C.B.	6.0			3.0	2.0	23.8	16.8	21.0
4	N/B - W.C.B.	8.0	11.0	17.0	4.0	2.9	40.6	42.0	44.8
5	EB/ Prot LT - Derry	6.0			3.0	2.0	23.8	19.2	21.0
6	W/B - Derry	8.0	11.0	17.0	4.0	2.9	51.8	42.0	53.2
7	NBLT - W.C.B.	5.0			3.0		-	12.0	21.0
8	S/B - W.C.B.	8.0	11.0	17.0	4.0	2.9	64.4	46.8	44.8
System Control		YES							
Local Control		NO							
Semi-Actuated Mode		No (Fully Actuated)							
		TIME (M-F)		PEAK		CYCLE LENGTH (sec.)		OFFSET (sec.)	
		06:00-09:30		AM		140		8.4	
		09:30-15:00		OFF		120		18.0	
		15:00-19:30		PM		140		47.6	

REGIONAL MUNICIPALITY OF PEEL

Traffic Signal Timing Parameters

Database Date		June 24, 2011		Prepared Date:		September 9, 2013					
Database Rev		34		Completed By:		MF					
Timing Card / Field rev		27		Checked By:		MF					
Location:		Mississauga Rd @ Argentia Rd				TIME PERIOD					
Phase #	Direction	Vehicle Minimum (sec.)	Pedestrian Minimum (sec.)		Amber (sec.)	All Red (sec.)	(sec.) (Green+Amber+All Red)				
			WALK	FDWALK			AM MAX	OFF 1 MAX	OFF 2 MAX	PM MAX	
1	NBLT - EMP	5.0			3.0		-	-	-	12.6	
2	S/B - EMP	12.0	8.0	12.0	4.2	2.3	95.2	80.4	68.4	61.6	
3	EBLT - Argentia	5.0			3.0		-	-	12.0	26.6	
4	W/B - Argentia	8.0	11.0	17.0	4.0	3.8	44.8 (53)	39.6 (42)	39.6 (42)	39.2 (42)	
5	S/B Prot LT - EMP	8.0			3.0	2.0	28 (50)	24 (26)	24 (26)	21 (23)	
6	N/B - EMP	12.0	8.0	12.0	4.2	2.3	67.2	56.4	44.4	53.2	
7	WBLT - Argentia	5.0			3.0		11.2	-	-	22.4	
8	E/B - Argentia	8.0	11.0	17.0	4.0	3.8	33.6 (42)	39.6 (42)	51.6 (54)	43.4 (46)	
System Control		YES									
Local Control		NO									
Semi-Actuated Mode		YES									
				TIME (M-F)		PEAK		CYCLE LENGTH (sec.)		OFFSET (sec.)	
				06:00-09:30		AM		140		96.6	
				09:30-12:00		OFF 1		120		76.8	
				14:00-15:00		OFF 2		120		88.8	
				12:00-14:00		OFF 2		120		88.8	
				15:00-19:30		PM		140		57.4	

REGIONAL MUNICIPALITY OF PEEL

Traffic Signal Timing Parameters

Database Date		April 4, 2011				Prepared Date:		September 9, 2013	
Database Rev		63				Completed By:		MF	
Timing Card / Field rev		38				Checked By:		MF	
Location:		Mississauga Rd @ Derry Rd				TIME PERIOD			
Phase #	Direction	Vehicle Minimum (sec.)	Pedestrian Minimum (sec.)		Amber (sec.)	All Red (sec.)	(sec.)		
			WALK	FDWALK			(Green+Amber+All Red)		
							AM MAX	OFF MAX	PM MAX
1	N/B Prot LT - Miss	12.0			3.0	2.0	23.8 (26)	22.8 (25)	23.8 (26)
2	S/B - Miss	12.0	12.0	18.0	5.4	2.4	46.2	40.8	44.8
3	EBLT - Derry	5.0			3.0		23.8	9.6	21.0
4	W/B - Derry	12.0	12.0	18.0	5.4	3.2	46.2 (49)	46.8	50.4 (57)
5	S/B Prot LT - Miss	12.0			3.0	2.0	25.2 (28)	22.8 (25)	23.8 (26)
6	N/B - Miss	12.0	12.0	18.0	5.4	2.4	44.8	40.8	44.8
7	W/B Prot LT - Derry	12.0			3.0	2.0	22.4	24.0	29.4
8	E/B - Derry	12.0	12.0	18.0	5.4	3.2	47.6 (51)	32.4	42 (46)
System Control		YES							
Local Control		NO							
Semi-Actuated Mode		No (Fully Actuated)							
				TIME (M-F)	PEAK	CYCLE LENGTH (sec.)		OFFSET (sec.)	
				06:00-09:30	AM	140		16.8	
				09:30-15:00	OFF	120		24.0	
				15:00-19:30	PM	140		123.2	

10:59 USER 8/12 PRINT DAILY INT REP, INT 86, AS 1-3

DAILY INTERSECTION REPORT FOR ACT SCH 1 (MON TUE WED THU FRI)

INT TIME	SELECTION PLANS IN USE						ALTERNATES					
	MODE	CYC LEN	OFF NO.	SPLT NO.	SPEC FUNC	DUP ISEC	MODE	CYC LEN	OFF NO.	SPLT NO.	SPEC FUNC	DUP ISEC
86 00:00	/	/	/	/	/	/	LO	101	2	2	2	
86 06:00	1/1	/	/	/	1/1	/	CC	140	1	1	1	1023
86 09:30	1/1	/	/	/	1/1	/	CC	120	2	2	2	1023
86 15:00	1/1	/	/	/	1/1	/	CC	140	3	3	3	1023
86 19:30	1/1	/	/	/	1/1	/	CC	120	2	2	2	1023

DAILY INTERSECTION REPORT FOR ACT SCH 2 (SAT)

86 00:00	/	/	/	/	/	/	LO	101	2	2	2	
86 07:00	1/1	/	/	/	1/1	/	CC	120	2	2	2	1023

DAILY INTERSECTION REPORT FOR ACT SCH 3 (SUN HOL)

86 00:00	/	/	/	/	/	/	LO	101	2	2	2	
86 08:00	1/1	/	/	/	1/1	/	CC	120	2	2	2	1023
86 23:00	/	/	/	/	/	/	LO	101	2	2	2	1023

11:00 USER 8/12 PRINT SPF 1-3, INT 86

SPECIAL FUNCTIONS

INTERSECTION 86 WCB @ ARGENTIA

SPECIAL IN(Y)/OUT(N)

FUNCTION	#	1	2	3	PHASE OMIT BUT SP2 Y=ON
1		N	N	N	
2		N	N	N	
3		N	N	N	

11:00 USER 8/12 PRINT SPLIT 1-3, INT 86

SPLIT TABLE

INTERSECTION 86

WCB @ ARGENTIA

TABLE NO.	PHASE NUMBER								(MAX SPLIT) PHASE NUMBER							
	1	2	3	4	5	6	7	8	1	2	3	4	5	6	7	8
	NBL	SB	EBL	WB	SBL	NB	WBL	EB								
1	13	35	19	33	15	33	10	42	21	0	0	0	0	0	0	0
2	19	37	21	23	16	40	13	31	23	0	0	0	23	0	0	0
3	14	35	17	34	14	35	13	38	22	0	0	0	22	0	0	0

11:01 USER 8/12 PRINT OFFSET 1-3, INT 86

OFFSET TABLE

INTERSECTION 86 WCB @ ARGENTIA

OFFSET # OFFSET %

1 82

2 85

3 21

11:00 USER 8/12 PRINT CDT 86

CYCLE DEFINITION TABLE: 86

PHASE	DIR	VEH MIN	PED MIN	PED CLEAR	AMBER	ALL RED	COMM DELAY	SPECIAL FEATURE	STREET NAME
1	NBL	7			3	2	1		WCB
2	SB		15	22	4	3	1	C	WCB
3	EBL	7			3	2	1		ARGENTIA ROAD
4	WB	8	15	21	4	3	1	E	ARGENTIA ROAD
5	SBL	7			3	2	1		WCB
6	NB		15	22	4	3	1	C	WCB
7	WBL	5			3		1		ARGENTIA ROAD
8	EB	8	15	21	4	3	1	E	ARGENTIA ROAD

VALID SPECIAL FUNCTIONS (Y/N)

1	2	3	1&2	1&3	2&3	ALL
Y	Y	Y	Y	Y	Y	Y

INT No.:

86

LOCATION: **Winston Churchill @ Argentia**

SCHEDULED DATA

Mode	Cycle Length	OFF No.	Split No.	Spec Func	DUP ISEC
LO	101	2	2	2	1023

PHASING DATA

PHASE	MIN	MAX	WALK	DON'T WALK	AMBER	ALL RED
1. NBLT Protected Arrow	7	15	-	-	3	2
2. SB Winston Churchill	45	N/A	23	22	4	3
3. EBL Protected Arrow	7	15	-	-	3	2
4. WB Argentia	8	25	15	21	4	3
5. SBLT Protected Arrow	7	15	-	-	3	2
6. NB Winston Churchill	45	N/A	23	22	4	3
7. WBL Arrow	5	15	-	-	3	-
8. EB Argentia	8	25	15	21	4	3

* ALL VALUES IN SECONDS

Appendix C

Existing Turning Movement Counts

Ontario Traffic Inc.

Morning Peak Diagram

Specified Period

From: 7:00:00
To: 9:00:00

One Hour Peak

From: 8:00:00
To: 9:00:00

Municipality: Peel
Site #: 0000518131
Intersection: Derry Rd & Argentia Rd
TFR File #: 1
Count date: 15-Feb-11

Weather conditions:
Clear
Person(s) who counted:
Grigory Naftolin
Anna Naftolin

**** Signalized Intersection ****

Major Road: Derry Rd runs W/E

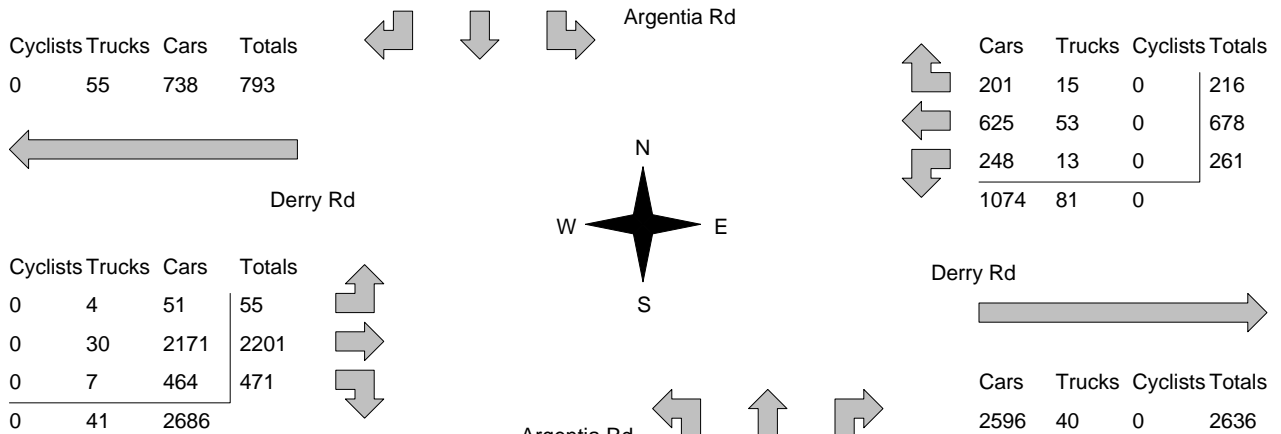
North Leg Total: 1259
North Entering: 849
North Peds: 13
Peds Cross: \times

Cyclists	0	0	0	0
Trucks	1	3	4	8
Cars	42	478	321	841
Totals	43	481	325	



Cyclists	0
Trucks	25
Cars	385
Totals	410

East Leg Total: 3791
East Entering: 1155
East Peds: 12
Peds Cross: \times



Peds Cross: \times
West Peds: 0
West Entering: 2727
West Leg Total: 3520

Cars	1190
Trucks	23
Cyclists	0
Totals	1213



Cars	71	133	104	308
Trucks	1	6	6	13
Cyclists	0	0	0	0
Totals	72	139	110	

Peds Cross: \times
South Peds: 0
South Entering: 321
South Leg Total: 1534

Comments

Ontario Traffic Inc.

Mid-day Peak Diagram

Specified Period

From: 11:00:00
To: 14:00:00

One Hour Peak

From: 12:30:00
To: 13:30:00

Municipality: Peel
Site #: 0000518131
Intersection: Derry Rd & Argentia Rd
TFR File #: 1
Count date: 15-Feb-11

Weather conditions:
Clear
Person(s) who counted:
Grigory Naftolin
Anna Naftolin

**** Signalized Intersection ****

Major Road: Derry Rd runs W/E

North Leg Total: 909
North Entering: 460
North Peds: 4
Peds Cross: \bowtie

Cyclists	0	0	3	3
Trucks	5	10	5	20
Cars	43	242	152	437
Totals	48	252	160	



Cyclists 0
Trucks 4
Cars 445
Totals 449

East Leg Total: 1588
East Entering: 779
East Peds: 3
Peds Cross: \bowtie

Cyclists	Trucks	Cars	Totals
0	32	664	696

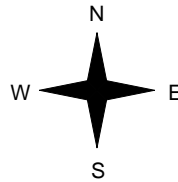


Argentia Rd

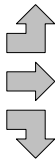
Cars	Trucks	Cyclists	Totals
151	1	0	152
502	25	0	527
95	5	0	100
748	31	0	



Derry Rd



Cyclists	Trucks	Cars	Totals
0	1	55	56
0	24	546	570
0	3	202	205
0	28	803	



Derry Rd



Cars	Trucks	Cyclists	Totals
773	33	3	809

Peds Cross: \bowtie
West Peds: 2
West Entering: 831
West Leg Total: 1527

Cars	539	Cars	119	239	75	433
Trucks	18	Trucks	2	2	4	8
Cyclists	0	Cyclists	0	0	0	0
Totals	557	Totals	121	241	79	



Argentia Rd



Peds Cross: \bowtie
South Peds: 0
South Entering: 441
South Leg Total: 998

Comments

Ontario Traffic Inc.

Afternoon Peak Diagram

Specified Period

From: 15:00:00
To: 18:00:00

One Hour Peak

From: 16:45:00
To: 17:45:00

Municipality: Peel
Site #: 0000518131
Intersection: Derry Rd & Argentia Rd
TFR File #: 1
Count date: 15-Feb-11

Weather conditions:
Clear
Person(s) who counted:
Grigory Naftolin
Anna Naftolin

**** Signalized Intersection ****

Major Road: Derry Rd runs W/E

North Leg Total: 1612
North Entering: 624
North Peds: 2
Peds Cross: \bowtie

Cyclists	0	0	0	0
Trucks	0	4	5	9
Cars	119	228	268	615
Totals	119	232	273	



Cyclists	0
Trucks	13
Cars	975
Totals	988

East Leg Total: 4266
East Entering: 2774
East Peds: 0
Peds Cross: \bowtie

Cyclists	Trucks	Cars	Totals
0	25	2703	2728

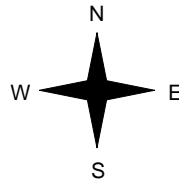


Argentia Rd

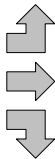
Cars	Trucks	Cyclists	Totals
414	7	0	421
2275	25	0	2300
52	1	0	53
2741	33	0	



Derry Rd



Cyclists	Trucks	Cars	Totals
0	0	46	46
0	42	863	905
0	6	161	167
0	48	1070	



Derry Rd



Peds Cross: \bowtie
West Peds: 0
West Entering: 1118
West Leg Total: 3846

Cars	441	Cars	309	515	307	1131
Trucks	11	Trucks	0	6	7	13
Cyclists	0	Cyclists	0	0	0	0
Totals	452	Totals	309	521	314	



Argentia Rd



Peds Cross: \bowtie
South Peds: 8
South Entering: 1144
South Leg Total: 1596

Comments

Ontario Traffic Inc.

Total Count Diagram

Municipality: Peel
Site #: 0000518131
Intersection: Derry Rd & Argentia Rd
TFR File #: 1
Count date: 15-Feb-11

Weather conditions:
 Clear
Person(s) who counted:
 Grigory Naftolin
 Anna Naftolin

**** Signalized Intersection ****

Major Road: Derry Rd runs W/E

North Leg Total: 8177
 North Entering: 4188
 North Peds: 32
 Peds Cross: \bowtie

Cyclists	0	0	3	3
Trucks	12	52	37	101
Cars	416	2037	1631	4084
Totals	428	2089	1671	



Cyclists 0
 Trucks 137
 Cars 3852
 Totals 3989

East Leg Total: 21738
 East Entering: 10839
 East Peds: 43
 Peds Cross: \bowtie

Cyclists	Trucks	Cars	Totals
0	318	9753	10071

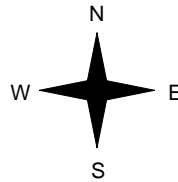


Argentia Rd

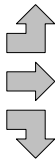
Cars	Trucks	Cyclists	Totals
1565	66	0	1631
7956	274	0	8230
934	44	0	978
10455	384	0	



Derry Rd



Cyclists	Trucks	Cars	Totals
0	14	400	414
0	247	7854	8101
0	57	1652	1709
0	318	9906	



Derry Rd



Peds Cross: \bowtie
 West Peds: 13
 West Entering: 10224
 West Leg Total: 20295

Cars	4623
Trucks	153
Cyclists	0
Totals	4776



Cars	1381	1887	1076	4344
Trucks	32	57	51	140
Cyclists	0	0	0	0
Totals	1413	1944	1127	

Peds Cross: \bowtie
 South Peds: 16
 South Entering: 4484
 South Leg Total: 9260

Comments

Ontario Traffic Inc. Traffic Count Summary

Intersection: Derry Rd & Argentia Rd

Count Date: 15-Feb-11

Municipality: Peel

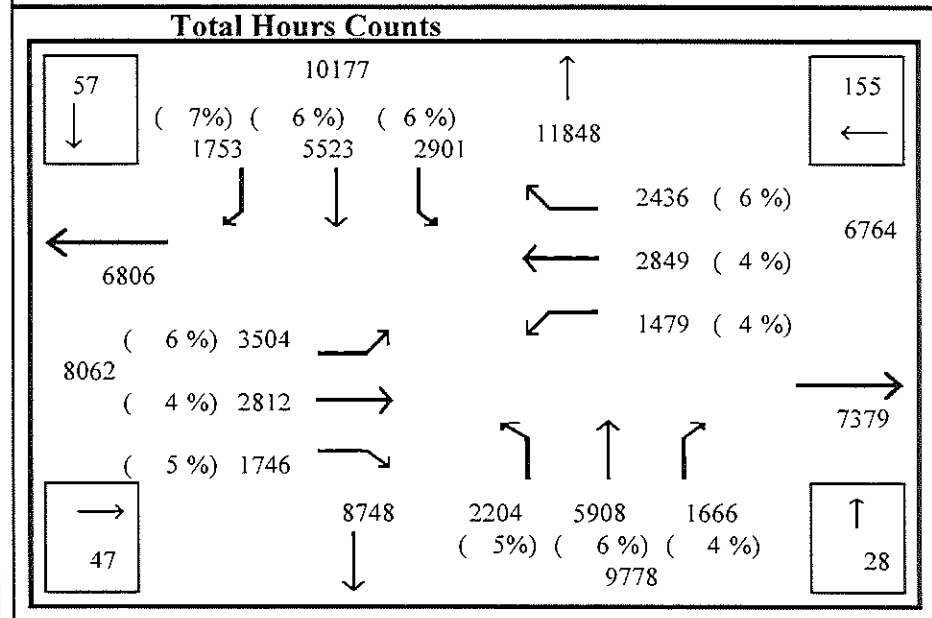
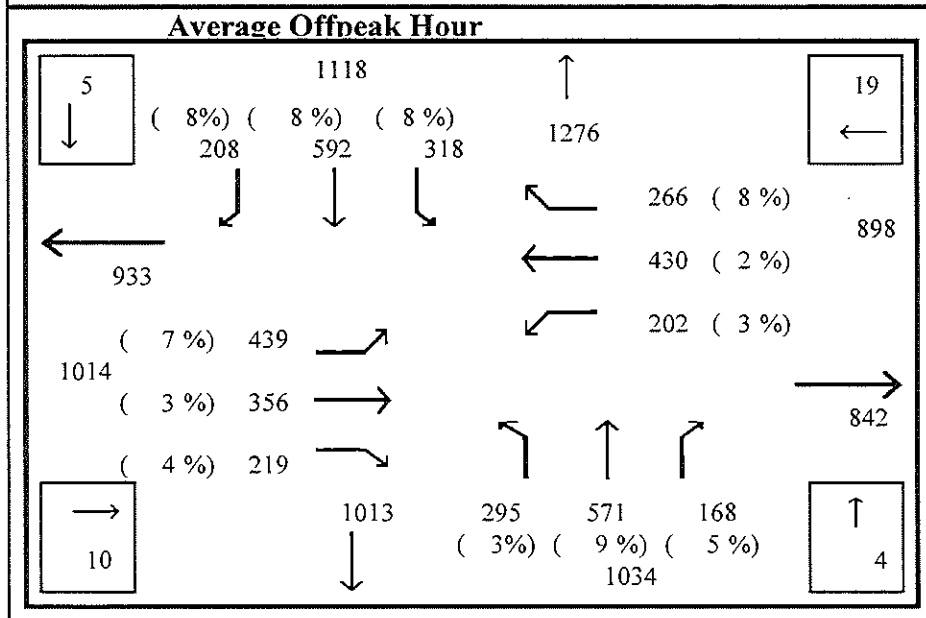
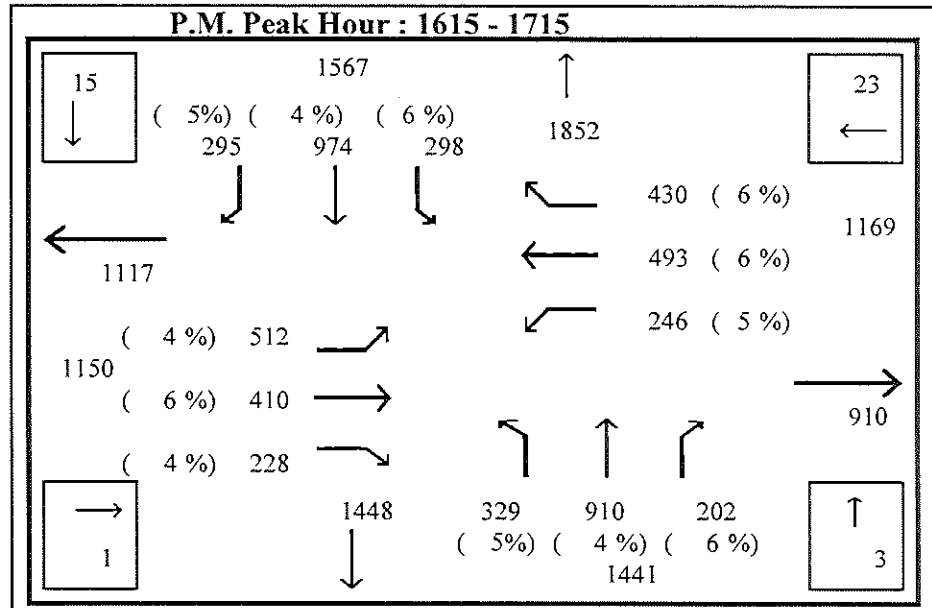
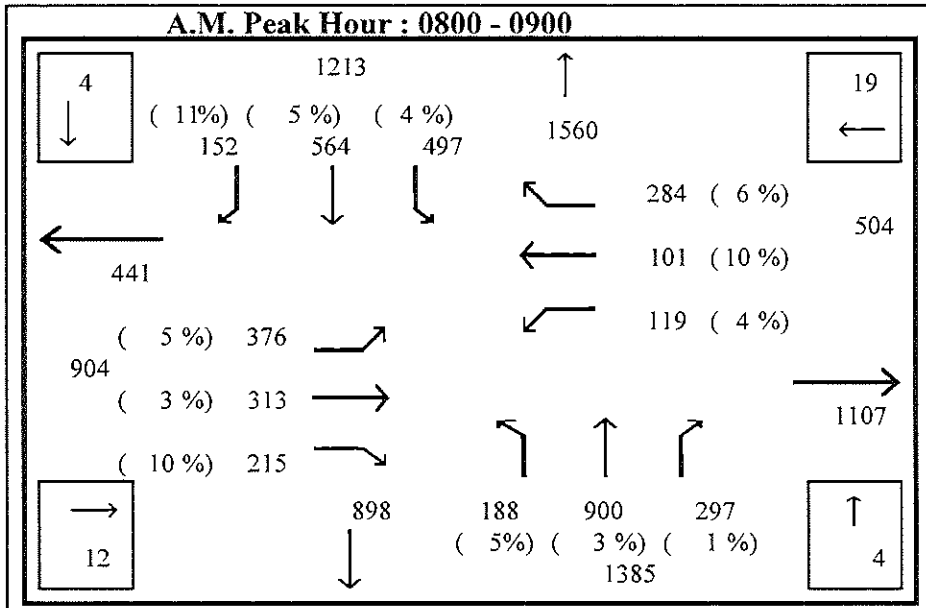
North Approach Totals						North/South Total Approaches	South Approach Totals					
Hour Ending	Includes Cars, Trucks, & Cyclists				Total Peds		Hour Ending	Includes Cars, Trucks, & Cyclists				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
7:00:00	7	10	2	19	0	21	7:00:00	0	1	1	2	0
8:00:00	338	411	19	768	5	951	8:00:00	39	68	76	183	0
9:00:00	325	481	43	849	13	1170	9:00:00	72	139	110	321	0
11:00:00	2	5	0	7	0	34	11:00:00	14	7	6	27	0
12:00:00	121	138	35	294	0	698	12:00:00	130	193	81	404	0
13:00:00	134	224	61	419	5	960	13:00:00	170	282	89	541	0
14:00:00	150	234	50	434	2	837	14:00:00	117	201	85	403	0
15:00:00	2	4	0	6	0	30	15:00:00	10	12	2	24	0
16:00:00	117	136	37	290	3	801	16:00:00	185	189	137	511	2
17:00:00	165	184	62	411	4	1376	17:00:00	378	319	268	965	10
18:00:00	310	262	119	691	0	1794	18:00:00	298	533	272	1103	4
Totals:	1671	2089	428	4188	32	8672		1413	1944	1127	4484	16
East Approach Totals						East/West Total Approaches	West Approach Totals					
Hour Ending	Includes Cars, Trucks, & Cyclists				Total Peds		Hour Ending	Includes Cars, Trucks, & Cyclists				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
7:00:00	5	11	1	17	0	55	7:00:00	0	34	4	38	0
8:00:00	209	590	151	950	0	3464	8:00:00	55	2101	358	2514	0
9:00:00	261	678	216	1155	12	3882	9:00:00	55	2201	471	2727	0
11:00:00	13	29	11	53	0	74	11:00:00	7	10	4	21	0
12:00:00	66	392	113	571	6	1207	12:00:00	27	498	111	636	0
13:00:00	101	543	175	819	3	1511	13:00:00	61	482	149	692	0
14:00:00	86	496	109	691	0	1515	14:00:00	52	583	189	824	2
15:00:00	3	63	2	68	0	85	15:00:00	0	14	3	17	0
16:00:00	89	1187	156	1432	11	2174	16:00:00	60	563	119	742	4
17:00:00	84	2045	282	2411	8	3322	17:00:00	52	704	155	911	7
18:00:00	61	2196	415	2672	3	3774	18:00:00	45	911	146	1102	0
Totals:	978	8230	1631	10839	43	21063		414	8101	1709	10224	13
Calculated Values for Traffic Crossing Major Street												
Hours Ending:	8:00	9:00	12:00	13:00		14:00	16:00	17:00	18:00			
Crossing Values:	788	890	450	589		503	506	877	1144			

Ontario Traffic Inc.

Count Date: 15-Feb-11 Site #: 000518131

Interval Time	Passenger Cars - North Approach						Trucks - North Approach						Cyclists - North Approach						Pedestrians	
	Left		Thru		Right		Left		Thru		Right		Left		Thru		Right		North Cross	
	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	7	7	10	10	2	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	55	48	85	75	7	5	0	0	1	1	0	0	0	0	0	0	0	0	0	0
7:30:00	149	94	183	98	11	4	0	0	1	0	0	0	0	0	0	0	0	0	0	0
7:45:00	247	98	281	98	11	0	1	1	2	1	0	0	0	0	0	0	0	0	2	2
8:00:00	343	96	416	135	21	10	2	1	5	3	0	0	0	0	0	0	0	0	5	3
8:15:00	416	73	543	127	37	16	2	0	5	0	0	0	0	0	0	0	0	0	5	0
8:30:00	499	83	655	112	45	8	3	1	7	2	0	0	0	0	0	0	0	0	13	8
8:45:00	593	94	793	138	55	10	6	3	7	0	1	1	0	0	0	0	0	0	13	0
9:00:00	664	71	894	101	63	8	6	0	8	1	1	0	0	0	0	0	0	0	18	5
9:02:17	664	0	894	0	63	0	6	0	8	0	1	0	0	0	0	0	0	0	18	0
11:00:00	666	2	898	4	63	0	6	0	9	1	1	0	0	0	0	0	0	0	18	0
11:15:00	715	49	914	16	67	4	9	3	10	1	4	3	0	0	0	0	0	0	18	0
11:30:00	741	26	939	25	77	10	9	0	11	1	5	1	0	0	0	0	0	0	18	0
11:45:00	762	21	979	40	82	5	11	2	12	1	6	1	0	0	0	0	0	0	18	0
12:00:00	781	19	1031	52	93	11	12	1	14	2	6	0	0	0	0	0	0	0	18	0
12:15:00	812	31	1076	45	112	19	13	1	18	4	6	0	0	0	0	0	0	0	18	0
12:30:00	832	20	1119	43	127	15	15	2	19	1	6	0	0	0	0	0	0	0	21	3
12:45:00	863	31	1171	52	142	15	17	2	27	8	9	3	3	3	0	0	0	0	23	2
13:00:00	907	44	1241	70	150	8	17	0	28	1	10	1	3	0	0	0	0	0	23	0
13:15:00	951	44	1302	61	158	8	18	1	28	0	10	0	3	0	0	0	0	0	25	2
13:30:00	984	33	1361	59	170	12	20	2	29	1	11	1	3	0	0	0	0	0	25	0
13:45:00	1022	38	1418	57	186	16	20	0	32	3	11	0	3	0	0	0	0	0	25	0
14:00:00	1053	31	1470	52	199	13	21	1	33	1	11	0	3	0	0	0	0	0	25	0
14:02:12	1053	0	1470	0	199	0	21	0	33	0	11	0	3	0	0	0	0	0	25	0
15:00:00	1055	2	1473	3	199	0	21	0	34	1	11	0	3	0	0	0	0	0	25	0
15:15:00	1076	21	1509	36	207	8	23	2	35	1	11	0	3	0	0	0	0	0	25	0
15:30:00	1097	21	1545	36	218	11	23	0	38	3	11	0	3	0	0	0	0	0	28	3
15:45:00	1139	42	1572	27	227	9	24	1	41	3	11	0	3	0	0	0	0	0	28	0
16:00:00	1167	28	1601	29	236	9	26	2	42	1	11	0	3	0	0	0	0	0	28	0
16:15:00	1208	41	1660	59	247	11	26	0	43	1	11	0	3	0	0	0	0	0	28	0
16:30:00	1243	35	1693	33	264	17	30	4	44	1	12	1	3	0	0	0	0	0	28	0
16:45:00	1296	53	1743	50	278	14	30	0	44	0	12	0	3	0	0	0	0	0	30	2
17:00:00	1326	30	1782	39	297	19	32	2	45	1	12	0	3	0	0	0	0	0	32	2
17:15:00	1418	92	1864	82	342	45	33	1	47	2	12	0	3	0	0	0	0	0	32	0
17:30:00	1498	80	1927	63	373	31	35	2	48	1	12	0	3	0	0	0	0	0	32	0
17:45:00	1564	66	1971	44	397	24	35	0	48	0	12	0	3	0	0	0	0	0	32	0
18:00:00	1631	67	2037	66	416	19	37	2	52	4	12	0	3	0	0	0	0	0	32	0
18:15:00	1631	0	2037	0	416	0	37	0	52	0	12	0	3	0	0	0	0	0	32	0
18:15:37	1631	0	2037	0	416	0	37	0	52	0	12	0	3	0	0	0	0	0	32	0

[ARG-WIN-01-S] ARGENTIA RD/WINSTON CHURCHILL BLVD



Note: North is at the top of the page

Value in (parenthesis) indicates truck/heavy vehicle percentages

Intersection count: 15-minute interval data

ARGENTIA RD/WINSTON CHURCHILL BLVD

ARG-WIN-01-S

ZArea: Z54W

Counted On: 2012/06/26

Traffic Control: Signal

Report Date: 2013/09/09

Time Ending	E - East					N - North					S - South					W - West					All Vehicles
	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	
715	38	38	22	6	0	20	135	41	6	0	100	135	21	13	0	10	18	18	1	0	622
730	79	53	57	7	0	30	179	47	7	0	164	149	47	18	0	11	12	36	0	0	896
745	78	68	42	5	0	19	229	77	9	0	106	132	43	16	1	23	16	60	4	0	927
800	82	76	47	17	2	23	218	33	20	0	117	151	44	21	0	26	19	55	9	0	958
815	90	64	46	6	2	32	237	82	12	6	95	119	36	16	6	10	15	59	4	0	923
830	94	82	54	14	0	30	221	77	12	0	119	136	29	16	4	40	32	77	8	0	1041
845	86	81	49	11	0	61	210	69	3	2	146	130	34	15	0	28	19	68	4	2	1014
900	87	77	44	19	2	56	204	65	14	4	119	148	37	18	9	36	25	64	15	2	1028
Total	634	539	361	85	6	271	1633	491	83	12	966	1100	291	133	20	184	156	437	45	4	

AM Peak	E - East					N - North					S - South					W - West					All Vehicles
	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	
815 - 900																					
Total	357	304	193	50	4	179	872	293	41	12	479	533	136	65	19	114	91	268	31	4	0

Offpeak	E - East					N - North					S - South					W - West					All Vehicles
	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	
1115	74	51	39	9	2	55	103	33	16	1	55	84	46	17	3	48	61	35	4	2	730
1130	73	54	36	16	5	45	119	38	18	0	58	95	44	24	8	47	65	42	12	0	786
1145	70	56	55	10	0	52	98	52	4	0	68	173	43	45	0	49	83	60	12	0	930
1200	83	60	42	15	1	72	118	29	18	6	99	133	52	30	4	38	92	53	6	6	940
1215	90	67	48	18	0	79	125	45	23	0	92	134	61	26	2	43	128	55	13	0	1047
1230	108	93	104	10	1	103	147	53	21	8	98	127	78	24	8	84	180	80	4	0	1314
1245	164	141	61	9	0	73	137	42	15	8	61	143	53	19	7	64	144	83	4	1	1213
1300	134	145	43	11	0	76	131	43	21	0	71	148	50	28	5	43	139	71	10	4	1164
1315	117	102	39	11	5	59	136	44	19	0	65	166	35	14	7	46	102	54	16	0	1025
1330	119	90	61	4	0	94	159	21	21	4	69	143	41	12	7	40	95	79	6	0	1054

Intersection count: 15-minute interval data

ARGENTIA RD/WINSTON CHURCHILL BLVD

ARG-WIN-01-S

ZArea: Z54W

Counted On: 2012/06/26

Traffic Control: Signal

Report Date: 2013/09/09

	1345	109	87	55	14	3	72	152	37	21	3	74	135	37	21	7	45	92	73	15	0	1039
	1400	92	91	52	15	0	77	134	41	15	0	66	145	39	17	0	41	85	48	10	0	968
Offpeak Total	1233	1037	635	142	17	857	1559	478	212	30	876	1626	579	277	58	588	1266	733	112	13		
Offpeak Hr Avg	411	346	212	47	6	286	520	159	71	10	292	542	193	92	19	196	422	244	37	4		

PM	E - East					N - North					S - South					W - West					All Vehicles
	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	
1515	117	92	52	15	4	88	157	53	20	3	74	151	50	37	10	48	99	67	23	0	1143
1530	123	97	48	18	6	77	165	54	31	0	83	117	62	27	9	55	107	87	17	0	1168
1545	138	109	56	19	0	85	175	48	26	0	67	188	73	20	11	53	138	93	18	0	1306
1600	118	87	55	13	4	86	167	57	23	1	82	188	51	23	9	49	97	77	13	0	1186
1615	122	84	62	19	3	85	219	53	13	0	84	194	63	19	6	58	121	96	6	0	1298
1630	120	84	54	17	2	75	228	53	23	0	86	236	74	24	6	56	105	107	14	3	1356
1645	122	97	52	10	0	77	192	48	11	0	104	234	75	23	2	53	123	84	17	0	1322
1700	112	121	49	17	6	94	204	42	14	1	40	194	54	8	10	62	109	95	18	0	1233
1715	135	82	65	13	7	68	248	47	17	0	50	268	76	21	5	62	124	118	22	0	1416
1730	117	94	64	13	2	81	227	67	15	0	67	249	72	15	4	57	103	105	12	3	1358
1745	116	95	66	9	0	75	198	49	10	0	76	232	52	18	2	49	108	98	9	3	1260
1800	96	75	45	12	0	81	198	54	16	0	64	192	58	14	3	42	82	84	3	2	1116
Total	1436	1117	668	175	34	972	2378	625	219	5	877	2443	760	249	77	644	1316	1111	172	11	

PM Peak	E - East					N - North					S - South					W - West					All Vehicles	
	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds	LT	Thru	RT	Trk	Peds		
1630 - 1715																						
Total	489	384	220	57	15	314	872	190	65	1	280	932	279	76	23	233	461	404	71	3	0	

Ontario Traffic Inc.

Morning Peak Diagram

Specified Period

From: 7:00:00
To: 9:00:00

One Hour Peak

From: 7:45:00
To: 8:45:00

Municipality: Peel
Site #: 0000520006
Intersection: Derry Rd & Winston Churchill Blvd
TFR File #: 1
Count date: 1-Mar-11

Weather conditions:
Sunny
Person(s) who counted:
Iory Mezentsev
Tatiana Mezentseva

**** Signalized Intersection ****

Major Road: Derry Rd runs W/E

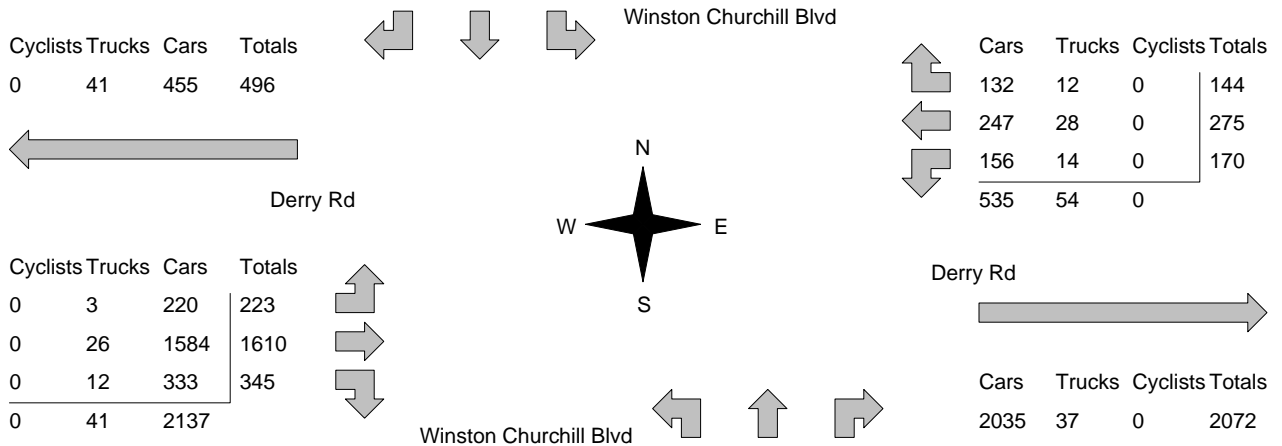
North Leg Total: 2000
North Entering: 899
North Peds: 6
Peds Cross: \bowtie

Cyclists	0	2	0	2
Trucks	1	45	8	54
Cars	74	623	146	843
Totals	75	670	154	



Cyclists	0
Trucks	38
Cars	1063
Totals	1101

East Leg Total: 2661
East Entering: 589
East Peds: 7
Peds Cross: \bowtie



Peds Cross: \bowtie
West Peds: 8
West Entering: 2178
West Leg Total: 2674

Cars	1112	Cars	134	711	305	1150
Trucks	71	Trucks	12	23	3	38
Cyclists	2	Cyclists	0	0	0	0
Totals	1185	Totals	146	734	308	

Peds Cross: \bowtie
South Peds: 5
South Entering: 1188
South Leg Total: 2373

Comments

Ontario Traffic Inc.

Mid-day Peak Diagram

Specified Period

From: 11:00:00

To: 14:00:00

One Hour Peak

From: 13:00:00

To: 14:00:00

Municipality: Peel
Site #: 0000520006
Intersection: Derry Rd & Winston Churchill Blvd
TFR File #: 1
Count date: 1-Mar-11

Weather conditions:
Sunny
Person(s) who counted:
Iory Mezentsev
Tatiana Mezentseva

**** Signalized Intersection ****

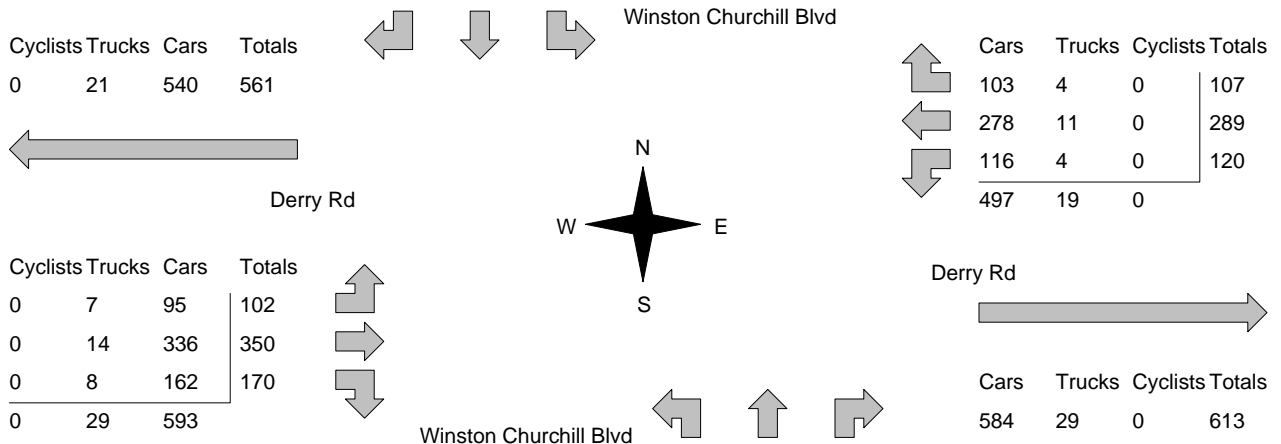
Major Road: Derry Rd runs W/E

North Leg Total: 1642
 North Entering: 847
 North Peds: 6
 Peds Cross: \bowtie

Cyclists	0	0	0	0
Trucks	3	18	6	27
Cars	97	601	122	820
Totals	100	619	128	

Cyclists	0
Trucks	41
Cars	754
Totals	795

East Leg Total: 1129
 East Entering: 516
 East Peds: 9
 Peds Cross: \bowtie



Peds Cross: \bowtie
 West Peds: 6
 West Entering: 622
 West Leg Total: 1183

Cars	879	Cars	165	556	126	847
Trucks	30	Trucks	7	30	9	46
Cyclists	0	Cyclists	0	0	0	0
Totals	909	Totals	172	586	135	

Peds Cross: \bowtie
 South Peds: 3
 South Entering: 893
 South Leg Total: 1802

Comments

Ontario Traffic Inc.

Afternoon Peak Diagram

Specified Period

From: 15:00:00

To: 18:00:00

One Hour Peak

From: 17:00:00

To: 18:00:00

Municipality: Peel
Site #: 0000520006
Intersection: Derry Rd & Winston Churchill Blvd
TFR File #: 1
Count date: 1-Mar-11

Weather conditions:
 Sunny
Person(s) who counted:
 Iory Mezentsev
 Tatiana Mezentseva

**** Signalized Intersection ****

Major Road: Derry Rd runs W/E

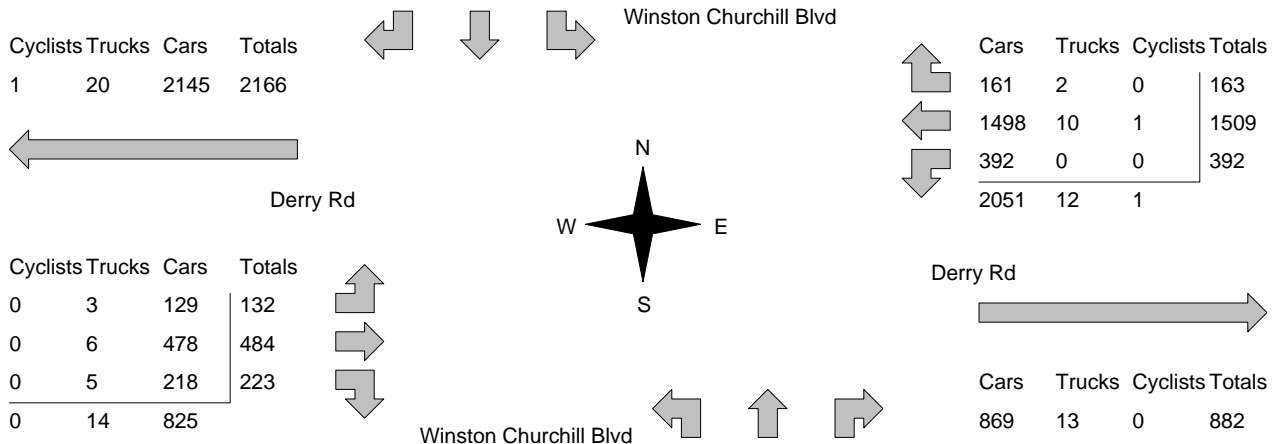
North Leg Total: 2815
 North Entering: 1707
 North Peds: 6
 Peds Cross: \bowtie

Cyclists	0	2	0	2
Trucks	2	15	1	18
Cars	324	1137	226	1687
Totals	326	1154	227	



Cyclists	2
Trucks	26
Cars	1080
Totals	1108

East Leg Total: 2946
 East Entering: 2064
 East Peds: 3
 Peds Cross: \bowtie



Peds Cross: \bowtie
 West Peds: 5
 West Entering: 839
 West Leg Total: 3005

Cars	1747	Cars	323	790	165	1278
Trucks	20	Trucks	8	21	6	35
Cyclists	2	Cyclists	0	2	0	2
Totals	1769	Totals	331	813	171	

Peds Cross: \bowtie
 South Peds: 1
 South Entering: 1315
 South Leg Total: 3084

Comments

Ontario Traffic Inc.

Total Count Diagram

Municipality: Peel
Site #: 0000520006
Intersection: Derry Rd & Winston Churchill Blvd
TFR File #: 1
Count date: 1-Mar-11

Weather conditions:
 Sunny
Person(s) who counted:
 Iory Mezentsev
 Tatiana Mezentseva

**** Signalized Intersection ****

Major Road: Derry Rd runs W/E

North Leg Total: 15064
 North Entering: 7918
 North Peds: 77
 Peds Cross: ⚡

Cyclists	0	7	0	7
Trucks	42	210	42	294
Cars	1143	5391	1083	7617
Totals	1185	5608	1125	



Cyclists	4
Trucks	313
Cars	6829
Totals	7146

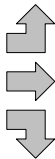
East Leg Total: 15692
 East Entering: 7832
 East Peds: 62
 Peds Cross: ⚡

Cyclists	Trucks	Cars	Totals
2	243	7854	8099

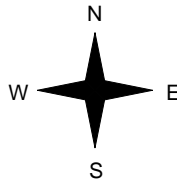


Derry Rd

Cyclists	Trucks	Cars	Totals
0	38	940	978
0	128	5200	5328
0	83	1669	1752
0	249	7809	



Winston Churchill Blvd



Winston Churchill Blvd



Cars	Trucks	Cyclists	Totals
1015	50	0	1065
5066	126	2	5194
1530	43	0	1573
7611	219	2	

Derry Rd



Cars	Trucks	Cyclists	Totals
7645	215	0	7860

Peds Cross: ⚡
 West Peds: 67
 West Entering: 8058
 West Leg Total: 16157

Cars	8590
Trucks	336
Cyclists	7
Totals	8933



Cars	1645	4874	1362	7881
Trucks	75	225	45	345
Cyclists	0	4	0	4
Totals	1720	5103	1407	

Peds Cross: ⚡
 South Peds: 35
 South Entering: 8230
 South Leg Total: 17163

Comments

Ontario Traffic Inc. Traffic Count Summary

Intersection: Derry Rd & Winston Churchill Blvd													Count Date: 1-Mar-11		Municipality: Peel	
North Approach Totals						North/South Total Approaches	South Approach Totals									
Includes Cars, Trucks, & Cyclists					Total Peds		Includes Cars, Trucks, & Cyclists					Total Peds				
Hour Ending	Left	Thru	Right	Grand Total			Hour Ending	Left	Thru	Right	Grand Total					
7:00:00	0	0	0	0	0	0	7:00:00	0	0	0	0	0				
8:00:00	115	519	113	747	11	1659	8:00:00	125	547	240	912	4				
9:00:00	147	635	70	852	6	2080	9:00:00	154	733	341	1228	1				
11:00:00	0	0	0	0	0	19	11:00:00	1	17	1	19	0				
12:00:00	106	522	78	706	3	1473	12:00:00	155	511	101	767	1				
13:00:00	99	596	109	804	3	1674	13:00:00	174	578	118	870	4				
14:00:00	128	619	100	847	6	1740	14:00:00	172	586	135	893	3				
15:00:00	0	3	2	5	0	5	15:00:00	0	0	0	0	0				
16:00:00	129	714	172	1015	32	2103	16:00:00	293	633	162	1088	17				
17:00:00	174	846	215	1235	10	2373	17:00:00	315	685	138	1138	4				
18:00:00	227	1154	326	1707	6	3022	18:00:00	331	813	171	1315	1				
Totals:	1125	5608	1185	7918	77	16148		1720	5103	1407	8230	35				
East Approach Totals						East/West Total Approaches	West Approach Totals									
Includes Cars, Trucks, & Cyclists					Total Peds		Includes Cars, Trucks, & Cyclists					Total Peds				
Hour Ending	Left	Thru	Right	Grand Total			Hour Ending	Left	Thru	Right	Grand Total					
7:00:00	0	0	0	0	0	0	7:00:00	0	0	0	0	0				
8:00:00	129	250	97	476	7	2485	8:00:00	188	1548	273	2009	12				
9:00:00	176	299	144	619	2	2672	9:00:00	201	1525	327	2053	6				
11:00:00	0	0	0	0	0	20	11:00:00	1	17	2	20	0				
12:00:00	78	242	105	425	6	967	12:00:00	67	302	173	542	0				
13:00:00	109	277	88	474	5	1002	13:00:00	84	272	172	528	5				
14:00:00	120	289	107	516	9	1138	14:00:00	102	350	170	622	6				
15:00:00	0	0	1	1	0	1	15:00:00	0	0	0	0	0				
16:00:00	232	842	156	1230	26	1949	16:00:00	97	422	200	719	31				
17:00:00	337	1486	204	2027	4	2753	17:00:00	106	408	212	726	2				
18:00:00	392	1509	163	2064	3	2903	18:00:00	132	484	223	839	5				
Totals:	1573	5194	1065	7832	62	15890		978	5328	1752	8058	67				
Calculated Values for Traffic Crossing Major Street																
Hours Ending:	8:00	9:00	12:00	13:00		14:00	16:00	17:00	18:00							
Crossing Values:	806	1042	789	879		934	1193	1341	1720							

Ontario Traffic Inc.

Count Date: 1-Mar-11 Site #: 000520006

Interval Time	Passenger Cars - North Approach						Trucks - North Approach						Cyclists - North Approach						Pedestrians	
	Left		Thru		Right		Left		Thru		Right		Left		Thru		Right		North Cross	
	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	32	32	83	83	21	21	2	2	6	6	9	9	0	0	0	0	0	0	2	2
7:30:00	58	26	217	134	45	24	3	1	16	10	12	3	0	0	0	0	0	0	4	2
7:45:00	80	22	350	133	71	26	5	2	22	6	12	0	0	0	0	0	0	0	8	4
8:00:00	108	28	488	138	101	30	7	2	30	8	12	0	0	0	1	1	0	0	11	3
8:15:00	145	37	659	171	112	11	9	2	47	17	12	0	0	0	2	1	0	0	13	2
8:30:00	184	39	826	167	125	13	12	3	56	9	12	0	0	0	2	0	0	0	14	1
8:45:00	226	42	973	147	145	20	13	1	67	11	13	1	0	0	2	0	0	0	14	0
9:00:00	248	22	1080	107	164	19	14	1	72	5	19	6	0	0	2	0	0	0	17	3
9:00:24	248	0	1080	0	164	0	14	0	72	0	19	0	0	0	2	0	0	0	17	0
11:00:00	248	0	1080	0	164	0	14	0	72	0	19	0	0	0	2	0	0	0	17	0
11:15:00	267	19	1166	86	184	20	14	0	76	4	21	2	0	0	2	0	0	0	18	1
11:30:00	299	32	1296	130	195	11	15	1	88	12	26	5	0	0	3	1	0	0	18	0
11:45:00	314	15	1414	118	211	16	16	1	97	9	27	1	0	0	3	0	0	0	18	0
12:00:00	350	36	1561	147	234	23	18	2	111	14	27	0	0	0	4	1	0	0	20	2
12:15:00	365	15	1689	128	265	31	20	2	120	9	28	1	0	0	4	0	0	0	22	2
12:30:00	397	32	1853	164	288	23	21	1	124	4	28	0	0	0	4	0	0	0	23	1
12:45:00	419	22	2003	150	309	21	21	0	132	8	28	0	0	0	4	0	0	0	23	0
13:00:00	445	26	2131	128	342	33	22	1	137	5	28	0	0	0	4	0	0	0	23	0
13:15:00	479	34	2272	141	354	12	22	0	140	3	29	1	0	0	4	0	0	0	26	3
13:30:00	514	35	2449	177	377	23	23	1	146	6	31	2	0	0	4	0	0	0	26	0
13:45:00	539	25	2578	129	399	22	24	1	149	3	31	0	0	0	4	0	0	0	27	1
14:00:00	567	28	2732	154	439	40	28	4	155	6	31	0	0	0	4	0	0	0	29	2
14:00:15	567	0	2732	0	439	0	28	0	155	0	31	0	0	0	4	0	0	0	29	0
15:00:00	567	0	2735	3	441	2	28	0	155	0	31	0	0	0	4	0	0	0	29	0
15:15:00	598	31	2898	163	482	41	31	3	161	6	31	0	0	0	4	0	0	0	36	7
15:30:00	625	27	3090	192	527	45	31	0	165	4	33	2	0	0	5	1	0	0	53	17
15:45:00	657	32	3234	144	564	37	32	1	170	5	35	2	0	0	5	0	0	0	58	5
16:00:00	690	33	3426	192	607	43	34	2	177	7	37	2	0	0	5	0	0	0	61	3
16:15:00	737	47	3628	202	656	49	35	1	183	6	37	0	0	0	5	0	0	0	62	1
16:30:00	777	40	3830	202	696	40	37	2	186	3	38	1	0	0	5	0	0	0	63	1
16:45:00	815	38	4059	229	755	59	40	3	188	2	38	0	0	0	5	0	0	0	66	3
17:00:00	857	42	4254	195	819	64	41	1	195	7	40	2	0	0	5	0	0	0	71	5
17:15:00	907	50	4547	293	897	78	41	0	197	2	42	2	0	0	7	2	0	0	74	3
17:30:00	974	67	4810	263	990	93	42	1	203	6	42	0	0	0	7	0	0	0	74	0
17:45:00	1036	62	5114	304	1088	98	42	0	207	4	42	0	0	0	7	0	0	0	77	3
18:00:00	1083	47	5391	277	1143	55	42	0	210	3	42	0	0	0	7	0	0	0	77	0
18:15:00	1083	0	5391	0	1143	0	42	0	210	0	42	0	0	0	7	0	0	0	77	0
18:15:26	1083	0	5391	0	1143	0	42	0	210	0	42	0	0	0	7	0	0	0	77	0

Ontario Traffic Inc.

Count Date: 1-Mar-11 Site #: 000520006

Interval Time	Passenger Cars - East Approach						Trucks - East Approach						Cyclists - East Approach						Pedestrians	
	Left		Thru		Right		Left		Thru		Right		Left		Thru		Right		East Cross	
	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	27	27	51	51	16	16	4	4	1	1	1	1	0	0	0	0	0	0	0	0
7:30:00	55	28	104	53	31	15	4	0	3	2	3	2	0	0	0	0	0	0	1	1
7:45:00	93	38	166	62	56	25	5	1	9	6	4	1	0	0	0	0	0	0	2	1
8:00:00	119	26	230	64	88	32	10	5	20	11	9	5	0	0	0	0	0	0	7	5
8:15:00	148	29	284	54	120	32	12	2	28	8	13	4	0	0	0	0	0	0	9	2
8:30:00	199	51	357	73	146	26	17	5	31	3	14	1	0	0	0	0	0	0	9	0
8:45:00	249	50	413	56	188	42	19	2	37	6	16	2	0	0	0	0	0	0	9	0
9:00:00	286	37	509	96	222	34	19	0	40	3	19	3	0	0	0	0	0	0	9	0
9:00:24	286	0	509	0	222	0	19	0	40	0	19	0	0	0	0	0	0	0	9	0
11:00:00	286	0	509	0	222	0	19	0	40	0	19	0	0	0	0	0	0	0	9	0
11:15:00	305	19	558	49	241	19	19	0	43	3	20	1	0	0	0	0	0	0	10	1
11:30:00	317	12	614	56	263	22	19	0	47	4	23	3	0	0	0	0	0	0	11	1
11:45:00	337	20	674	60	293	30	20	1	52	5	23	0	0	0	0	0	0	0	13	2
12:00:00	363	26	732	58	321	28	20	0	59	7	25	2	0	0	0	0	0	0	15	2
12:15:00	396	33	794	62	340	19	21	1	61	2	25	0	0	0	0	0	0	0	17	2
12:30:00	418	22	856	62	360	20	24	3	64	3	26	1	0	0	1	1	0	0	18	1
12:45:00	444	26	929	73	382	22	27	3	70	6	26	0	0	0	1	0	0	0	18	0
13:00:00	464	20	997	68	407	25	28	1	70	0	27	1	0	0	1	0	0	0	20	2
13:15:00	497	33	1074	77	441	34	29	1	75	5	28	1	0	0	1	0	0	0	25	5
13:30:00	518	21	1134	60	470	29	30	1	75	0	29	1	0	0	1	0	0	0	27	2
13:45:00	548	30	1203	69	486	16	31	1	81	6	30	1	0	0	1	0	0	0	27	0
14:00:00	580	32	1275	72	510	24	32	1	81	0	31	1	0	0	1	0	0	0	29	2
14:00:15	580	0	1275	0	511	1	32	0	81	0	31	0	0	0	1	0	0	0	29	0
15:00:00	580	0	1275	0	511	0	32	0	81	0	31	0	0	0	1	0	0	0	29	0
15:15:00	627	47	1438	163	539	28	36	4	82	1	31	0	0	0	1	0	0	0	35	6
15:30:00	674	47	1601	163	581	42	38	2	88	6	39	8	0	0	1	0	0	0	45	10
15:45:00	740	66	1851	250	623	42	39	1	95	7	42	3	0	0	1	0	0	0	46	1
16:00:00	804	64	2097	246	655	32	40	1	101	6	43	1	0	0	1	0	0	0	55	9
16:15:00	880	76	2430	333	699	44	43	3	105	4	45	2	0	0	1	0	0	0	55	0
16:30:00	959	79	2778	348	734	35	43	0	110	5	46	1	0	0	1	0	0	0	56	1
16:45:00	1048	89	3176	398	805	71	43	0	113	3	48	2	0	0	1	0	0	0	58	2
17:00:00	1138	90	3568	392	854	49	43	0	116	3	48	0	0	0	1	0	0	0	59	1
17:15:00	1250	112	3926	358	905	51	43	0	119	3	48	0	0	0	1	0	0	0	61	2
17:30:00	1336	86	4363	437	935	30	43	0	120	1	49	1	0	0	1	0	0	0	62	1
17:45:00	1427	91	4694	331	967	32	43	0	122	2	50	1	0	0	2	1	0	0	62	0
18:00:00	1530	103	5066	372	1015	48	43	0	126	4	50	0	0	0	2	0	0	0	62	0
18:15:00	1530	0	5066	0	1015	0	43	0	126	0	50	0	0	0	2	0	0	0	62	0
18:15:26	1530	0	5066	0	1015	0	43	0	126	0	50	0	0	0	2	0	0	0	62	0

Ontario Traffic Inc.

Count Date: 1-Mar-11 Site #: 000520006

Interval Time	Passenger Cars - South Approach						Trucks - South Approach						Cyclists - South Approach						Pedestrians	
	Left		Thru		Right		Left		Thru		Right		Left		Thru		Right		South Cross	
	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	22	22	96	96	50	50	4	4	3	3	2	2	0	0	0	0	0	0	0	0
7:30:00	45	23	197	101	100	50	6	2	9	6	3	1	0	0	0	0	0	0	0	0
7:45:00	84	39	346	149	170	70	8	2	14	5	4	1	0	0	0	0	0	0	0	0
8:00:00	113	29	528	182	234	64	12	4	19	5	6	2	0	0	0	0	0	0	4	4
8:15:00	151	38	726	198	321	87	16	4	23	4	6	0	0	0	0	0	0	0	5	1
8:30:00	187	36	900	174	406	85	17	1	34	11	6	0	0	0	0	0	0	0	5	0
8:45:00	218	31	1057	157	475	69	20	3	37	3	7	1	0	0	0	0	0	0	5	0
9:00:00	256	38	1227	170	573	98	23	3	53	16	8	1	0	0	0	0	0	0	5	0
9:00:24	257	1	1229	2	574	1	23	0	53	0	8	0	0	0	0	0	0	0	5	0
11:00:00	257	0	1243	14	574	0	23	0	54	1	8	0	0	0	0	0	0	0	5	0
11:15:00	290	33	1354	111	592	18	23	0	58	4	10	2	0	0	0	0	0	0	5	0
11:30:00	323	33	1482	128	612	20	27	4	66	8	11	1	0	0	0	0	0	0	6	1
11:45:00	365	42	1604	122	642	30	27	0	72	6	13	2	0	0	0	0	0	0	6	0
12:00:00	405	40	1728	124	669	27	30	3	80	8	14	1	0	0	0	0	0	0	6	0
12:15:00	443	38	1855	127	692	23	31	1	87	7	17	3	0	0	0	0	0	0	9	3
12:30:00	481	38	1980	125	712	20	34	3	94	7	17	0	0	0	0	0	0	0	10	1
12:45:00	525	44	2139	159	751	39	37	3	104	10	18	1	0	0	0	0	0	0	10	0
13:00:00	572	47	2269	130	782	31	37	0	117	13	19	1	0	0	0	0	0	0	10	0
13:15:00	617	45	2407	138	815	33	39	2	127	10	20	1	0	0	0	0	0	0	10	0
13:30:00	659	42	2550	143	841	26	40	1	134	7	23	3	0	0	0	0	0	0	11	1
13:45:00	693	34	2683	133	871	30	42	2	142	8	25	2	0	0	0	0	0	0	13	2
14:00:00	737	44	2825	142	908	37	44	2	147	5	28	3	0	0	0	0	0	0	13	0
14:00:15	737	0	2825	0	908	0	44	0	147	0	28	0	0	0	0	0	0	0	13	0
15:00:00	737	0	2825	0	908	0	44	0	147	0	28	0	0	0	0	0	0	0	13	0
15:15:00	798	61	2982	157	949	41	46	2	153	6	33	5	0	0	0	0	0	0	15	2
15:30:00	869	71	3136	154	985	36	54	8	161	8	36	3	0	0	0	0	0	0	24	9
15:45:00	935	66	3273	137	1022	37	57	3	166	5	37	1	0	0	0	0	0	0	28	4
16:00:00	1014	79	3431	158	1061	39	60	3	172	6	37	0	0	0	2	2	0	0	30	2
16:15:00	1091	77	3600	169	1090	29	62	2	184	12	37	0	0	0	2	0	0	0	30	0
16:30:00	1160	69	3771	171	1119	29	65	3	190	6	37	0	0	0	2	0	0	0	32	2
16:45:00	1244	84	3928	157	1155	36	65	0	196	6	37	0	0	0	2	0	0	0	32	0
17:00:00	1322	78	4084	156	1197	42	67	2	204	8	39	2	0	0	2	0	0	0	34	2
17:15:00	1410	88	4300	216	1234	37	69	2	212	8	42	3	0	0	2	0	0	0	35	1
17:30:00	1490	80	4491	191	1276	42	71	2	217	5	43	1	0	0	3	1	0	0	35	0
17:45:00	1579	89	4689	198	1330	54	74	3	219	2	45	2	0	0	4	1	0	0	35	0
18:00:00	1645	66	4874	185	1362	32	75	1	225	6	45	0	0	0	4	0	0	0	35	0
18:15:00	1645	0	4874	0	1362	0	75	0	225	0	45	0	0	0	4	0	0	0	35	0
18:15:26	1645	0	4874	0	1362	0	75	0	225	0	45	0	0	0	4	0	0	0	35	0

Ontario Traffic Inc.

Morning Peak Diagram

Specified Period

From: 7:00:00
To: 9:00:00

One Hour Peak

From: 8:00:00
To: 9:00:00

Municipality: Peel
Site #: 0000111131
Intersection: Mississauga Road & Argentia Road
TFR File #: 1
Count date: 20-Dec-11

Weather conditions:
Clear
Person(s) who counted:
Sergei Bulyma
Bronislava Bulyma

**** Signalized Intersection ****

Major Road: Mississauga Road runs N/S

North Leg Total: 5319
North Entering: 2523
North Peds: 1
Peds Cross: \times

Cyclists	0	0	0	0
Trucks	6	92	25	123
Cars	494	1205	701	2400
Totals	500	1297	726	



Cyclists 0
Trucks 94
Cars 2702
Totals 2796

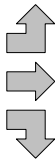
East Leg Total: 2320
East Entering: 782
East Peds: 13
Peds Cross: \times

Cyclists	0	Trucks	19	Cars	946	Totals	965
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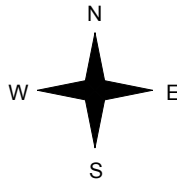


Argentia Road

Cyclists	0	Trucks	8	Cars	67	Totals	75
	0		13		344		357
	0		6		22		28
	0		27		433		



Mississauga Road



Cars	254	Trucks	21	Cyclists	0	Totals	275
	325		13		0		338
	163		6		0		169
	742		40		0		

Argentia Road



Cars	1491	Trucks	47	Cyclists	0	Totals	1538
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Peds Cross: \times
West Peds: 3
West Entering: 460
West Leg Total: 1425

Cars	1390	Cars	127	2381	446	2954
Trucks	104	Trucks	0	65	9	74
Cyclists	0	Cyclists	0	0	0	0
Totals	1494	Totals	127	2446	455	



Peds Cross: \times
South Peds: 28
South Entering: 3028
South Leg Total: 4522

Comments

Ontario Traffic Inc.

Mid-day Peak Diagram

Specified Period

From: 11:00:00
To: 14:00:00

One Hour Peak

From: 12:00:00
To: 13:00:00

Municipality: Peel
Site #: 0000111131
Intersection: Mississauga Road & Argentia Road
TFR File #: 1
Count date: 20-Dec-11

Weather conditions:
Clear
Person(s) who counted:
Sergei Bulyma
Bronislava Bulyma

**** Signalized Intersection ****

Major Road: Mississauga Road runs N/S

North Leg Total: 3307
North Entering: 1471
North Peds: 10
Peds Cross: \bowtie

Cyclists	0	0	0	0
Trucks	8	88	23	119
Cars	110	988	254	1352
Totals	118	1076	277	



Cyclists	0
Trucks	138
Cars	1698
Totals	1836

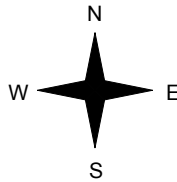
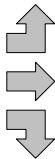
East Leg Total: 1673
East Entering: 970
East Peds: 14
Peds Cross: \bowtie

Cyclists	0
Trucks	17
Cars	471
Totals	488



Argentia Road

Cyclists	0
Trucks	11
Cars	174
Totals	185
Cyclists	1
Trucks	9
Cars	202
Totals	212
Cyclists	0
Trucks	2
Cars	83
Totals	85
Cyclists	1
Trucks	22
Cars	459
Totals	482



Mississauga Road

Cars	342	35	0	377
Trucks	282	7	0	289
Cyclists	291	12	1	304
Totals	915	54	1	



Argentia Road



Cars	662	40	1	703
Trucks				
Cyclists				
Totals	662	40	1	703

Peds Cross: \bowtie
West Peds: 0
West Entering: 482
West Leg Total: 970

Cars	1362	79	1182	206	1467
Trucks	102	2	92	8	102
Cyclists	1	0	0	0	0
Totals	1465	81	1274	214	



Peds Cross: \bowtie
South Peds: 3
South Entering: 1569
South Leg Total: 3034

Comments

Ontario Traffic Inc.

Afternoon Peak Diagram

Specified Period

From: 15:00:00

To: 18:00:00

One Hour Peak

From: 16:30:00

To: 17:30:00

Municipality: Peel
Site #: 0000111131
Intersection: Mississauga Road & Argentia Road
TFR File #: 1
Count date: 20-Dec-11

Weather conditions:
 Clear
Person(s) who counted:
 Sergei Bulyma
 Bronislava Bulyma

**** Signalized Intersection ****

Major Road: Mississauga Road runs N/S

North Leg Total: 5487
 North Entering: 2342
 North Peds: 5
 Peds Cross: \bowtie

Cyclists	0	0	0	0
Trucks	4	56	15	75
Cars	60	1976	231	2267
Totals	64	2032	246	



Cyclists 0
 Trucks 97
 Cars 3048
 Totals 3145

East Leg Total: 2616
 East Entering: 2024
 East Peds: 6
 Peds Cross: \bowtie

Cyclists	Trucks	Cars	Totals
0	17	498	515

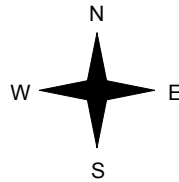


Mississauga Road

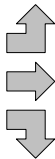
Cars	Trucks	Cyclists	Totals
1126	21	0	1147
396	13	0	409
460	7	1	468
1982	41	1	



Argentia Road



Cyclists	Trucks	Cars	Totals
0	6	363	369
0	7	205	212
0	2	87	89
0	15	655	



Argentia Road



Cars	Trucks	Cyclists	Totals
570	22	0	592

Peds Cross: \bowtie
 West Peds: 0
 West Entering: 670
 West Leg Total: 1185

Cars	2523	Cars	42	1559	134	1735
Trucks	65	Trucks	0	70	0	70
Cyclists	1	Cyclists	0	0	0	0
Totals	2589	Totals	42	1629	134	



Peds Cross: \bowtie
 South Peds: 0
 South Entering: 1805
 South Leg Total: 4394

Comments

Ontario Traffic Inc.

Total Count Diagram

Municipality: Peel
Site #: 0000111131
Intersection: Mississauga Road & Argentia Road
TFR File #: 1
Count date: 20-Dec-11

Weather conditions:
 Clear
Person(s) who counted:
 Sergei Bulyma
 Bronislava Bulyma

**** Signalized Intersection ****

Major Road: Mississauga Road runs N/S

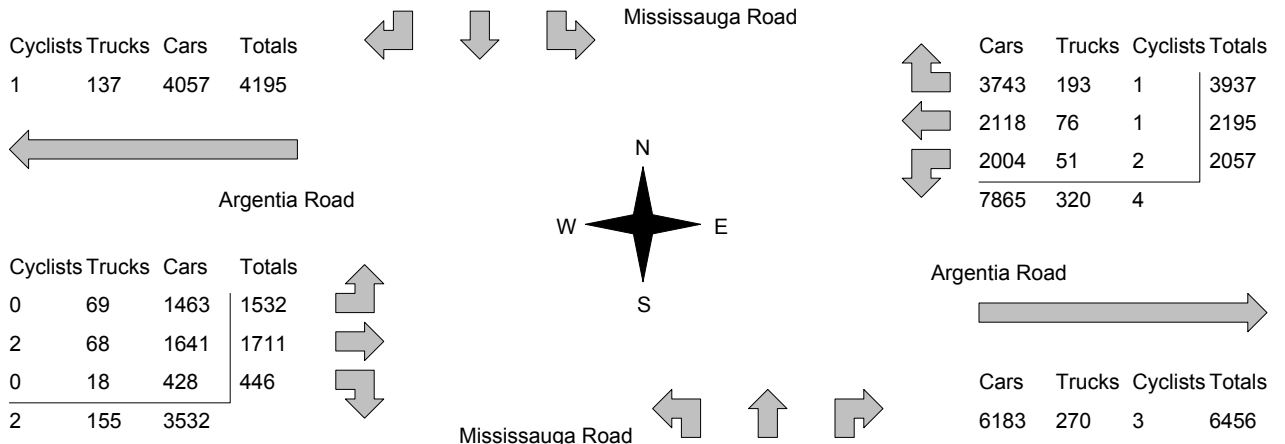
North Leg Total: 33708
 North Entering: 14976
 North Peds: 41
 Peds Cross: ⚡

Cyclists	0	0	0	0
Trucks	52	590	152	794
Cars	1363	9979	2840	14182
Totals	1415	10569	2992	



Cyclists 1
 Trucks 880
 Cars 17851
 Totals 18732

East Leg Total: 14645
 East Entering: 8189
 East Peds: 59
 Peds Cross: ⚡



Peds Cross: ⚡
 West Peds: 12
 West Entering: 3689
 West Leg Total: 7884

Cars	12411	Cars	576	12645	1702	14923
Trucks	659	Trucks	9	618	50	677
Cyclists	2	Cyclists	0	0	1	1
Totals	13072	Totals	585	13263	1753	

Peds Cross: ⚡
 South Peds: 57
 South Entering: 15601
 South Leg Total: 28673

Comments

Ontario Traffic Inc. Traffic Count Summary

Intersection: Mississauga Road & Argentia Road Count Date: 20-Dec-11 Municipality: Peel

North Approach Totals						North/South Total Approaches	South Approach Totals					
Hour Ending	Includes Cars, Trucks, & Cyclists				Total Peds		Hour Ending	Includes Cars, Trucks, & Cyclists				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
7:00:00	0	0	0	0	0	20	7:00:00	0	14	6	20	0
8:00:00	719	1179	337	2235	3	4680	8:00:00	83	2031	331	2445	17
9:00:00	726	1297	500	2523	1	5551	9:00:00	127	2446	455	3028	28
11:00:00	0	1	0	1	0	15	11:00:00	2	9	3	14	0
12:00:00	300	1022	131	1453	3	2710	12:00:00	54	1098	105	1257	2
13:00:00	277	1076	118	1471	10	3040	13:00:00	81	1274	214	1569	3
14:00:00	303	841	106	1250	12	3059	14:00:00	101	1481	227	1809	3
15:00:00	0	0	0	0	0	18	15:00:00	3	14	1	18	0
16:00:00	221	1280	100	1601	4	3480	16:00:00	56	1671	152	1879	0
17:00:00	209	1750	69	2028	7	3926	17:00:00	39	1697	162	1898	4
18:00:00	237	2123	54	2414	1	4068	18:00:00	38	1520	96	1654	0
Totals:	2992	10569	1415	14976	41	30567		584	13255	1752	15591	57
East Approach Totals						East/West Total Approaches	West Approach Totals					
Hour Ending	Includes Cars, Trucks, & Cyclists				Total Peds		Hour Ending	Includes Cars, Trucks, & Cyclists				Total Peds
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total	
7:00:00	1	2	3	6	0	7	7:00:00	0	1	0	1	0
8:00:00	113	187	176	476	2	882	8:00:00	62	332	12	406	4
9:00:00	169	338	275	782	13	1242	9:00:00	75	357	28	460	3
11:00:00	2	9	6	17	0	20	11:00:00	1	2	0	3	0
12:00:00	201	182	368	751	2	1058	12:00:00	155	97	55	307	0
13:00:00	304	289	377	970	14	1452	13:00:00	185	212	85	482	0
14:00:00	175	211	271	657	3	1004	14:00:00	120	175	52	347	4
15:00:00	4	8	5	17	0	21	15:00:00	0	4	0	4	0
16:00:00	223	209	542	974	4	1445	16:00:00	259	144	68	471	1
17:00:00	379	343	957	1679	14	2296	17:00:00	353	179	85	617	0
18:00:00	485	411	956	1852	7	2443	18:00:00	322	208	61	591	0
Totals:	2056	2189	3936	8181	59	11870		1532	1711	446	3689	12
Calculated Values for Traffic Crossing Major Street												
Hours Ending:	8:00	9:00	12:00	13:00		14:00	16:00	17:00	18:00			
Crossing Values:	527	630	543	791		521	695	1086	1219			

Ontario Traffic Inc.

Count Date: 20-Dec-11 Site #: 0000111131

Interval Time	Passenger Cars - East Approach						Trucks - East Approach						Cyclists - East Approach						Pedestrians	
	Left		Thru		Right		Left		Thru		Right		Left		Thru		Right		East Cross	
	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	1	1	2	2	3	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	21	20	32	30	48	45	0	0	2	2	0	0	0	0	0	0	0	0	1	1
7:30:00	44	23	59	27	91	43	0	0	4	2	3	3	0	0	0	0	0	0	2	1
7:45:00	70	26	109	50	129	38	1	1	5	1	5	2	0	0	0	0	0	0	2	0
8:00:00	112	42	182	73	167	38	2	1	7	2	12	7	0	0	0	0	0	0	2	0
8:15:00	165	53	255	73	224	57	2	0	11	4	17	5	0	0	0	0	0	0	11	9
8:30:00	210	45	342	87	302	78	6	4	12	1	25	8	0	0	0	0	0	0	15	4
8:45:00	239	29	417	75	373	71	7	1	15	3	27	2	0	0	0	0	0	0	15	0
9:00:00	275	36	507	90	421	48	8	1	20	5	33	6	0	0	0	0	0	0	15	0
9:00:25	277	2	516	9	424	3	8	0	20	0	33	0	0	0	0	0	0	0	15	0
11:00:00	277	0	516	0	426	2	8	0	20	0	34	1	0	0	0	0	0	0	15	0
11:15:00	299	22	545	29	497	71	10	2	24	4	38	4	0	0	0	0	0	0	15	0
11:30:00	329	30	578	33	573	76	11	1	24	0	47	9	0	0	0	0	0	0	15	0
11:45:00	385	56	618	40	666	93	13	2	25	1	65	18	0	0	0	0	0	0	15	0
12:00:00	471	86	689	71	758	92	15	2	29	4	70	5	0	0	0	0	0	0	17	2
12:15:00	574	103	793	104	854	96	20	5	30	1	80	10	0	0	0	0	0	0	17	0
12:30:00	650	76	861	68	946	92	23	3	32	2	92	12	1	1	0	0	0	0	25	8
12:45:00	716	66	902	41	1043	97	25	2	34	2	99	7	1	0	0	0	0	0	26	1
13:00:00	762	46	971	69	1100	57	27	2	36	2	105	6	1	0	0	0	0	0	31	5
13:15:00	819	57	1035	64	1161	61	29	2	39	3	115	10	1	0	0	0	0	0	32	1
13:30:00	856	37	1077	42	1221	60	30	1	41	2	118	3	1	0	0	0	0	0	33	1
13:45:00	900	44	1139	62	1272	51	30	0	44	3	125	7	1	0	0	0	0	0	34	1
14:00:00	933	33	1173	34	1342	70	31	1	45	1	134	9	1	0	0	0	0	0	34	0
14:00:58	937	4	1181	8	1345	3	31	0	45	0	134	0	1	0	0	0	0	0	34	0
15:00:00	937	0	1181	0	1347	2	31	0	45	0	134	0	1	0	0	0	0	0	34	0
15:15:00	991	54	1229	48	1462	115	32	1	48	3	137	3	1	0	0	0	0	0	34	0
15:30:00	1029	38	1272	43	1571	109	35	3	52	4	148	11	1	0	0	0	0	0	35	1
15:45:00	1076	47	1322	50	1736	165	38	3	52	0	153	5	1	0	0	0	0	0	36	1
16:00:00	1150	74	1381	59	1864	128	41	3	54	2	159	6	1	0	0	0	0	0	38	2
16:15:00	1235	85	1455	74	2067	203	42	1	57	3	162	3	1	0	0	0	0	0	44	6
16:30:00	1314	79	1533	78	2234	167	42	0	59	2	168	6	1	0	0	0	0	0	47	3
16:45:00	1415	101	1615	82	2545	311	43	1	61	2	176	8	1	0	0	0	0	0	47	0
17:00:00	1522	107	1712	97	2796	251	47	4	66	5	184	8	2	1	0	0	0	0	52	5
17:15:00	1665	143	1823	111	3112	316	49	2	69	3	187	3	2	0	0	0	0	0	53	1
17:30:00	1774	109	1929	106	3360	248	49	0	72	3	189	2	2	0	0	0	0	0	53	0
17:45:00	1902	128	2030	101	3581	221	49	0	74	2	191	2	2	0	1	1	1	1	59	6
18:00:00	2003	101	2112	82	3742	161	51	2	76	2	193	2	2	0	1	0	1	0	59	0
18:00:22	2004	1	2118	6	3743	1	51	0	76	0	193	0	2	0	1	0	1	0	59	0

Ontario Traffic Inc.

Count Date: 20-Dec-11 Site #: 0000111131

Interval Time	Passenger Cars - South Approach						Trucks - South Approach						Cyclists - South Approach						Pedestrians	
	Left		Thru		Right		Left		Thru		Right		Left		Thru		Right		South Cross	
	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	14	14	6	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	16	16	397	383	70	64	0	0	12	12	1	1	0	0	0	0	0	0	0	0
7:30:00	32	16	848	451	143	73	0	0	27	15	3	2	0	0	0	0	0	0	16	16
7:45:00	52	20	1456	608	228	85	0	0	41	14	5	2	0	0	0	0	0	0	17	1
8:00:00	82	30	1977	521	328	100	1	1	68	27	9	4	0	0	0	0	0	0	17	0
8:15:00	95	13	2620	643	427	99	1	0	82	14	10	1	0	0	0	0	0	0	31	14
8:30:00	121	26	3222	602	556	129	1	0	91	9	16	6	0	0	0	0	0	0	45	14
8:45:00	163	42	3779	557	662	106	1	0	112	21	17	1	0	0	0	0	0	0	45	0
9:00:00	209	46	4358	579	774	112	1	0	133	21	18	1	0	0	0	0	0	0	45	0
9:00:25	211	2	4365	7	777	3	1	0	133	0	18	0	0	0	0	0	0	0	45	0
11:00:00	211	0	4367	2	777	0	1	0	133	0	18	0	0	0	0	0	0	0	45	0
11:15:00	224	13	4575	208	797	20	2	1	151	18	19	1	0	0	0	0	0	0	45	0
11:30:00	237	13	4848	273	817	20	2	0	175	24	20	1	0	0	0	0	0	0	45	0
11:45:00	252	15	5102	254	841	24	3	1	191	16	21	1	0	0	0	0	0	0	45	0
12:00:00	262	10	5380	278	877	36	4	1	218	27	23	2	0	0	0	0	0	0	47	2
12:15:00	284	22	5619	239	913	36	5	1	247	29	24	1	0	0	0	0	0	0	47	0
12:30:00	293	9	5923	304	943	30	5	0	269	22	26	2	0	0	0	0	0	0	49	2
12:45:00	316	23	6197	274	1004	61	6	1	287	18	29	3	0	0	0	0	0	0	50	1
13:00:00	341	25	6562	365	1083	79	6	0	310	23	31	2	0	0	0	0	0	0	50	0
13:15:00	369	28	6881	319	1144	61	6	0	333	23	33	2	0	0	0	0	0	0	51	1
13:30:00	397	28	7220	339	1208	64	7	1	359	26	35	2	0	0	0	0	0	0	51	0
13:45:00	418	21	7581	361	1250	42	8	1	376	17	36	1	0	0	0	0	0	0	53	2
14:00:00	440	22	7948	367	1304	54	8	0	405	29	37	1	0	0	0	0	0	0	53	0
14:00:58	440	0	7952	4	1305	1	8	0	405	0	37	0	0	0	0	0	0	0	53	0
15:00:00	443	3	7962	10	1305	0	8	0	405	0	37	0	0	0	0	0	0	0	53	0
15:15:00	448	5	8329	367	1337	32	9	1	428	23	41	4	0	0	0	0	0	0	53	0
15:30:00	462	14	8770	441	1382	45	9	0	459	31	43	2	0	0	0	0	1	1	53	0
15:45:00	487	25	9217	447	1417	35	9	0	480	21	45	2	0	0	0	0	1	0	53	0
16:00:00	498	11	9541	324	1446	29	9	0	497	17	47	2	0	0	0	0	1	0	53	0
16:15:00	511	13	10016	475	1489	43	9	0	512	15	47	0	0	0	0	0	1	0	53	0
16:30:00	514	3	10377	361	1522	33	9	0	529	17	48	1	0	0	0	0	1	0	57	4
16:45:00	526	12	10767	390	1566	44	9	0	543	14	48	0	0	0	0	0	1	0	57	0
17:00:00	537	11	11163	396	1607	41	9	0	572	29	48	0	0	0	0	0	1	0	57	0
17:15:00	539	2	11529	366	1631	24	9	0	582	10	48	0	0	0	0	0	1	0	57	0
17:30:00	556	17	11936	407	1656	25	9	0	599	17	48	0	0	0	0	0	1	0	57	0
17:45:00	561	5	12294	358	1683	27	9	0	607	8	49	1	0	0	0	0	1	0	57	0
18:00:00	575	14	12637	343	1701	18	9	0	618	11	50	1	0	0	0	0	1	0	57	0
18:00:22	576	1	12645	8	1702	1	9	0	618	0	50	0	0	0	0	0	1	0	57	0

Ontario Traffic Inc.

Count Date: 20-Dec-11 Site #: 0000111131

Interval Time	Passenger Cars - West Approach						Trucks - West Approach						Cyclists - West Approach						Pedestrians	
	Left		Thru		Right		Left		Thru		Right		Left		Thru		Right		West Cross	
	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0
7:15:00	14	14	66	66	1	1	0	0	2	1	1	1	0	0	0	0	0	0	0	0
7:30:00	21	7	129	63	3	2	1	1	6	4	1	0	0	0	0	0	0	0	3	3
7:45:00	44	23	213	84	6	3	2	1	9	3	1	0	0	0	0	0	0	0	3	0
8:00:00	60	16	321	108	11	5	2	0	12	3	1	0	0	0	0	0	0	0	4	1
8:15:00	75	15	414	93	15	4	3	1	15	3	2	1	0	0	0	0	0	0	4	0
8:30:00	95	20	519	105	20	5	5	2	19	4	4	2	0	0	0	0	0	0	6	2
8:45:00	110	15	596	77	28	8	5	0	21	2	7	3	0	0	0	0	0	0	7	1
9:00:00	127	17	665	69	33	5	10	5	25	4	7	0	0	0	0	0	0	0	7	0
9:00:25	127	0	665	0	33	0	10	0	25	0	7	0	0	0	0	0	0	0	7	0
11:00:00	128	1	667	2	33	0	10	0	25	0	7	0	0	0	0	0	0	0	7	0
11:15:00	159	31	682	15	44	11	13	3	26	1	7	0	0	0	0	0	0	0	7	0
11:30:00	195	36	711	29	60	16	15	2	28	2	8	1	0	0	0	0	0	0	7	0
11:45:00	238	43	737	26	73	13	19	4	29	1	9	1	0	0	0	0	0	0	7	0
12:00:00	274	36	759	22	85	12	19	0	30	1	10	1	0	0	0	0	0	0	7	0
12:15:00	338	64	803	44	108	23	23	4	35	5	10	0	0	0	0	0	0	0	7	0
12:30:00	383	45	845	42	133	25	24	1	36	1	10	0	0	0	1	1	0	0	7	0
12:45:00	411	28	890	45	151	18	27	3	37	1	11	1	0	0	1	0	0	0	7	0
13:00:00	448	37	961	71	168	17	30	3	39	2	12	1	0	0	1	0	0	0	7	0
13:15:00	474	26	1005	44	177	9	36	6	41	2	12	0	0	0	1	0	0	0	8	1
13:30:00	505	31	1060	55	194	17	36	0	42	1	13	1	0	0	1	0	0	0	8	0
13:45:00	536	31	1091	31	208	14	42	6	45	3	15	2	0	0	2	1	0	0	9	1
14:00:00	554	18	1128	37	217	9	44	2	46	1	15	0	0	0	2	0	0	0	11	2
14:00:58	554	0	1128	0	217	0	44	0	46	0	15	0	0	0	2	0	0	0	11	0
15:00:00	554	0	1132	4	217	0	44	0	46	0	15	0	0	0	2	0	0	0	11	0
15:15:00	612	58	1166	34	238	21	50	6	46	0	15	0	0	0	2	0	0	0	11	0
15:30:00	653	41	1186	20	249	11	51	1	49	3	15	0	0	0	2	0	0	0	11	0
15:45:00	730	77	1229	43	268	19	52	1	51	2	16	1	0	0	2	0	0	0	12	1
16:00:00	804	74	1268	39	284	16	53	1	54	3	16	0	0	0	2	0	0	0	12	0
16:15:00	895	91	1303	35	304	20	55	2	56	2	16	0	0	0	2	0	0	0	12	0
16:30:00	979	84	1336	33	322	18	59	4	57	1	16	0	0	0	2	0	0	0	12	0
16:45:00	1071	92	1392	56	344	22	60	1	60	3	16	0	0	0	2	0	0	0	12	0
17:00:00	1147	76	1439	47	369	25	63	3	62	2	16	0	0	0	2	0	0	0	12	0
17:15:00	1261	114	1496	57	397	28	63	0	63	1	18	2	0	0	2	0	0	0	12	0
17:30:00	1342	81	1541	45	409	12	65	2	64	1	18	0	0	0	2	0	0	0	12	0
17:45:00	1416	74	1585	44	416	7	67	2	66	2	18	0	0	0	2	0	0	0	12	0
18:00:00	1463	47	1641	56	428	12	69	2	68	2	18	0	0	0	2	0	0	0	12	0
18:00:22	1463	0	1641	0	428	0	69	0	68	0	18	0	0	0	2	0	0	0	12	0

Ontario Traffic Inc.

Morning Peak Diagram

Specified Period

From: 7:00:00
To: 9:00:00

One Hour Peak

From: 7:45:00
To: 8:45:00

Municipality: Peel
Site #: 0000112422
Intersection: Mississauga Road & Derry Road
TFR File #: 3
Count date: 21-Dec-11

Weather conditions:
Cloudy, Rain
Person(s) who counted:
Sergei Bulyma
Bronislava Bulyma

**** Signalized Intersection ****

Major Road: Mississauga Road runs N/S

North Leg Total: 3801
North Entering: 1671
North Peds: 2
Peds Cross: \bowtie

Cyclists	0	0	0	0
Trucks	11	58	13	82
Cars	242	1109	238	1589
Totals	253	1167	251	



Cyclists	0
Trucks	79
Cars	2051
Totals	2130

East Leg Total: 3162
East Entering: 901
East Peds: 12
Peds Cross: \bowtie

Cyclists	Trucks	Cars	Totals
0	55	1131	1186

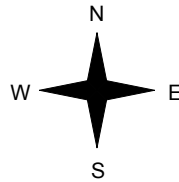


Mississauga Road

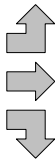
Cars	Trucks	Cyclists	Totals
46	12	0	58
466	36	0	502
293	48	0	341
805	96	0	



Derry Road



Cyclists	Trucks	Cars	Totals
0	12	553	565
0	24	1445	1469
0	5	125	130
0	41	2123	



Mississauga Road



Derry Road



Cars	Trucks	Cyclists	Totals
2200	61	0	2261

Peds Cross: \bowtie
West Peds: 1
West Entering: 2164
West Leg Total: 3350

Cars	1527	Cars	423	1452	517	2392
Trucks	111	Trucks	8	55	24	87
Cyclists	0	Cyclists	0	0	0	0
Totals	1638	Totals	431	1507	541	



Peds Cross: \bowtie
South Peds: 25
South Entering: 2479
South Leg Total: 4117

Comments

Ontario Traffic Inc.

Mid-day Peak Diagram

Specified Period

From: 11:00:00
To: 14:00:00

One Hour Peak

From: 11:45:00
To: 12:45:00

Municipality: Peel
Site #: 0000112422
Intersection: Mississauga Road & Derry Road
TFR File #: 3
Count date: 21-Dec-11

Weather conditions:
Cloudy, Rain
Person(s) who counted:
Sergei Bulyma
Bronislava Bulyma

**** Signalized Intersection ****

Major Road: Mississauga Road runs N/S

North Leg Total: 2172
North Entering: 1125
North Peds: 0
Peds Cross: \bowtie

Cyclists	0	0	0	0
Trucks	6	66	18	90
Cars	146	684	205	1035
Totals	152	750	223	



Cyclists	0
Trucks	102
Cars	945
Totals	1047

East Leg Total: 1689
East Entering: 754
East Peds: 5
Peds Cross: \bowtie

Cyclists	0
Trucks	39
Cars	715
Totals	754

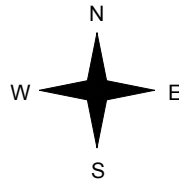


Mississauga Road

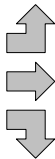
Cars	68	19	0	87
Trucks	368	19	0	387
Cyclists	249	31	0	280
Totals	685	69	0	



Derry Road



Cyclists	0
Trucks	8
Cars	154
Totals	162
Cyclists	0
Trucks	24
Cars	417
Totals	441
Cyclists	0
Trucks	9
Cars	135
Totals	144
Cyclists	0
Trucks	41
Cars	706
Totals	747



Mississauga Road

Derry Road



Cars	856	79	0	935
Trucks				
Cyclists				
Totals	935			

Peds Cross: \bowtie
West Peds: 1
West Entering: 747
West Leg Total: 1501

Cars	1068	201	723	234	1158
Trucks	106	14	75	37	126
Cyclists	0	0	0	0	0
Totals	1174	215	798	271	



Peds Cross: \bowtie
South Peds: 21
South Entering: 1284
South Leg Total: 2458

Comments

Ontario Traffic Inc.

Afternoon Peak Diagram

Specified Period

From: 15:00:00

To: 18:00:00

One Hour Peak

From: 16:15:00

To: 17:15:00

Municipality: Peel
Site #: 0000112422
Intersection: Mississauga Road & Derry Road
TFR File #: 3
Count date: 21-Dec-11

Weather conditions:
 Cloudy, Rain
Person(s) who counted:
 Sergei Bulyma
 Bronislava Bulyma

**** Signalized Intersection ****

Major Road: Mississauga Road runs N/S

North Leg Total: 4136
 North Entering: 1727
 North Peds: 1
 Peds Cross: \bowtie

Cyclists	0	0	0	0
Trucks	9	46	9	64
Cars	226	1302	135	1663
Totals	235	1348	144	



Cyclists	0
Trucks	72
Cars	2337
Totals	2409

East Leg Total: 3117
 East Entering: 1937
 East Peds: 0
 Peds Cross: \bowtie

Cyclists	Trucks	Cars	Totals
0	29	1757	1786

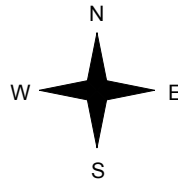


Mississauga Road

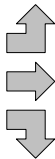
Cars	Trucks	Cyclists	Totals
126	16	0	142
1262	13	0	1275
493	27	0	520
1881	56	0	



Derry Road



Cyclists	Trucks	Cars	Totals
0	3	490	493
0	18	741	759
0	2	232	234
0	23	1463	



Derry Road



Cars	Trucks	Cyclists	Totals
1126	54	0	1180

Mississauga Road



Peds Cross: \bowtie
 West Peds: 0
 West Entering: 1486
 West Leg Total: 3272

Cars	2027	Cars	269	1721	250	2240
Trucks	75	Trucks	7	53	27	87
Cyclists	0	Cyclists	0	0	0	0
Totals	2102	Totals	276	1774	277	



Peds Cross: \bowtie
 South Peds: 1
 South Entering: 2327
 South Leg Total: 4429

Comments

Ontario Traffic Inc.

Total Count Diagram

Municipality: Peel
Site #: 0000112422
Intersection: Mississauga Road & Derry Road
TFR File #: 3
Count date: 21-Dec-11

Weather conditions:
 Cloudy, Rain
Person(s) who counted:
 Sergei Bulyma
 Bronislava Bulyma

**** Signalized Intersection ****

Major Road: Mississauga Road runs N/S

North Leg Total: 23671
 North Entering: 10354
 North Peds: 16
 Peds Cross: \bowtie

Cyclists	1	0	0	1
Trucks	59	426	103	588
Cars	1330	7147	1288	9765
Totals	1390	7573	1391	



Cyclists 0
 Trucks 680
 Cars 12637
 Totals 13317

East Leg Total: 19071
 East Entering: 8683
 East Peds: 45
 Peds Cross: \bowtie

Cyclists	Trucks	Cars	Totals
1	330	8642	8973

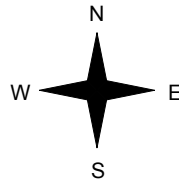


Mississauga Road

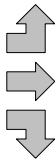
Cars	Trucks	Cyclists	Totals
599	125	0	724
5159	180	0	5339
2387	233	0	2620
8145	538	0	



Derry Road



Cyclists	Trucks	Cars	Totals
0	63	2834	2897
0	170	6422	6592
0	55	1201	1256
0	288	10457	



Derry Road



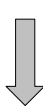
Mississauga Road



Cars	Trucks	Cyclists	Totals
9886	502	0	10388

Peds Cross: \bowtie
 West Peds: 4
 West Entering: 10745
 West Leg Total: 19718

Cars	10735	Cars	2153	9204	2176	13533
Trucks	714	Trucks	91	492	229	812
Cyclists	0	Cyclists	0	0	0	0
Totals	11449	Totals	2244	9696	2405	



Peds Cross: \bowtie
 South Peds: 91
 South Entering: 14345
 South Leg Total: 25794

Comments

Ontario Traffic Inc. Traffic Count Summary

Intersection: Mississauga Road & Derry Road													Count Date: 21-Dec-11		Municipality: Peel	
North Approach Totals						South Approach Totals										
Hour Ending	Includes Cars, Trucks, & Cyclists				Total Peds	North/South Total Approaches	Hour Ending	Includes Cars, Trucks, & Cyclists				Total Peds				
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total					
7:00:00	0	1	0	1	0	1	7:00:00	0	0	0	0	0				
8:00:00	149	980	163	1292	2	3361	8:00:00	311	1365	393	2069	5				
9:00:00	264	1192	257	1713	3	4233	9:00:00	442	1528	550	2520	27				
11:00:00	1	7	4	12	0	34	11:00:00	4	10	8	22	0				
12:00:00	191	700	147	1038	3	2168	12:00:00	203	660	267	1130	12				
13:00:00	190	696	135	1021	3	2259	13:00:00	204	803	231	1238	17				
14:00:00	125	651	111	887	1	2145	14:00:00	224	846	188	1258	23				
15:00:00	0	0	1	1	0	19	15:00:00	2	16	0	18	0				
16:00:00	150	923	142	1215	2	3017	16:00:00	263	1277	262	1802	4				
17:00:00	139	1275	227	1641	1	3983	17:00:00	302	1746	294	2342	1				
18:00:00	182	1148	203	1533	1	3466	18:00:00	280	1441	212	1933	2				
Totals:	1391	7573	1390	10354	16	24686		2235	9692	2405	14332	91				
East Approach Totals						West Approach Totals										
Hour Ending	Includes Cars, Trucks, & Cyclists				Total Peds	East/West Total Approaches	Hour Ending	Includes Cars, Trucks, & Cyclists				Total Peds				
	Left	Thru	Right	Grand Total				Left	Thru	Right	Grand Total					
7:00:00	0	0	0	0	0	0	7:00:00	0	0	0	0	0				
8:00:00	225	442	46	713	7	3077	8:00:00	490	1702	172	2364	1				
9:00:00	332	461	62	855	8	2857	9:00:00	558	1347	97	2002	0				
11:00:00	0	6	2	8	0	38	11:00:00	18	6	6	30	0				
12:00:00	231	307	65	603	7	1389	12:00:00	159	434	193	786	0				
13:00:00	280	391	88	759	5	1482	13:00:00	168	440	115	723	1				
14:00:00	273	418	82	773	10	1565	14:00:00	220	435	137	792	2				
15:00:00	0	7	2	9	0	19	15:00:00	2	2	6	10	0				
16:00:00	350	868	104	1322	2	2344	16:00:00	327	566	129	1022	0				
17:00:00	492	1166	140	1798	1	3228	17:00:00	452	770	208	1430	0				
18:00:00	437	1273	133	1843	5	3417	18:00:00	503	880	191	1574	0				
Totals:	2620	5339	724	8683	45	19416		2897	6582	1254	10733	4				
Calculated Values for Traffic Crossing Major Street																
Hours Ending:	8:00	9:00	12:00	13:00			14:00	16:00	17:00	18:00						
Crossing Values:	2424	2267	839	908			952	1551	2112	2216						

Ontario Traffic Inc.

Count Date: 21-Dec-11 Site #: 0000112422

Interval Time	Passenger Cars - North Approach						Trucks - North Approach						Cyclists - North Approach						Pedestrians	
	Left		Thru		Right		Left		Thru		Right		Left		Thru		Right		North Cross	
	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr	Cum	Incr
7:00:00	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15:00	28	28	209	208	28	28	1	1	5	5	1	1	0	0	0	0	0	0	1	1
7:30:00	72	44	391	182	69	41	4	3	16	11	2	1	0	0	0	0	0	0	2	1
7:45:00	107	35	671	280	112	43	6	2	26	10	3	1	0	0	0	0	0	0	2	0
8:00:00	141	34	944	273	156	44	8	2	37	11	7	4	0	0	0	0	0	0	2	0
8:15:00	203	62	1242	298	221	65	13	5	52	15	11	4	0	0	0	0	0	0	2	0
8:30:00	268	65	1528	286	281	60	15	2	70	18	12	1	0	0	0	0	0	0	4	2
8:45:00	345	77	1780	252	354	73	19	4	84	14	14	2	0	0	0	0	0	0	4	0
9:00:00	391	46	2067	287	404	50	22	3	106	22	16	2	0	0	0	0	0	0	5	1
9:00:37	391	0	2067	0	404	0	22	0	106	0	16	0	0	0	0	0	0	0	5	0
11:00:00	392	1	2074	7	408	4	22	0	106	0	16	0	0	0	0	0	0	0	5	0
11:15:00	416	24	2219	145	441	33	24	2	120	14	16	0	0	0	0	0	0	0	5	0
11:30:00	453	37	2362	143	472	31	27	3	128	8	16	0	0	0	0	0	0	0	5	0
11:45:00	499	46	2539	177	509	37	30	3	145	17	18	2	0	0	0	0	0	0	8	3
12:00:00	570	71	2725	186	551	42	35	5	155	10	20	2	0	0	0	0	0	0	8	0
12:15:00	626	56	2918	193	599	48	37	2	177	22	24	4	0	0	0	0	0	0	8	0
12:30:00	666	40	3064	146	637	38	40	3	195	18	24	0	0	0	0	0	0	0	8	0
12:45:00	704	38	3223	159	655	18	48	8	211	16	24	0	0	0	0	0	0	0	8	0
13:00:00	741	37	3354	131	680	25	54	6	222	11	26	2	0	0	0	0	0	0	11	3
13:15:00	774	33	3496	142	715	35	54	0	242	20	26	0	0	0	0	0	0	0	12	1
13:30:00	798	24	3624	128	736	21	56	2	257	15	28	2	0	0	0	0	0	0	12	0
13:45:00	823	25	3795	171	757	21	59	3	274	17	33	5	0	0	0	0	0	0	12	0
14:00:00	856	33	3928	133	782	25	64	5	299	25	35	2	0	0	0	0	0	0	12	0
14:00:52	856	0	3928	0	782	0	64	0	299	0	35	0	0	0	0	0	0	0	12	0
15:00:00	856	0	3928	0	783	1	64	0	299	0	35	0	0	0	0	0	0	0	12	0
15:15:00	891	35	4141	213	806	23	67	3	310	11	40	5	0	0	0	0	0	0	12	0
15:30:00	919	28	4334	193	846	40	73	6	327	17	41	1	0	0	0	0	0	0	12	0
15:45:00	959	40	4583	249	885	39	78	5	341	14	41	0	0	0	0	0	0	0	13	1
16:00:00	992	33	4798	215	919	34	78	0	352	11	41	0	0	0	0	0	0	0	14	1
16:15:00	1030	38	5063	265	969	50	84	6	360	8	45	4	0	0	0	0	0	0	14	0
16:30:00	1055	25	5389	326	1028	59	87	3	375	15	48	3	0	0	0	0	0	0	14	0
16:45:00	1090	35	5664	275	1089	61	90	3	389	14	49	1	0	0	0	0	0	0	14	0
17:00:00	1119	29	6024	360	1138	49	90	0	401	12	49	0	0	0	0	0	0	0	15	1
17:15:00	1165	46	6365	341	1195	57	93	3	406	5	54	5	0	0	0	0	0	0	15	0
17:30:00	1208	43	6687	322	1249	54	95	2	414	8	56	2	0	0	0	0	1	1	15	0
17:45:00	1249	41	6940	253	1282	33	101	6	421	7	56	0	0	0	0	0	1	0	15	0
18:00:00	1288	39	7147	207	1330	48	103	2	426	5	59	3	0	0	0	0	1	0	16	1
18:00:23	1288	0	7147	0	1330	0	103	0	426	0	59	0	0	0	0	0	1	0	16	0

Appendix D

Synchro HCM Reports – Derry Road and Argentia Road

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘	↑↑↑	↗	↘	↑	↗	↘	↗	↘
Volume (vph)	55	2201	471	261	678	216	72	139	110	325	481	43
Ideal Flow (vphpl)	1900	2050	1900	1900	1900	1900	1900	1900	1900	1900	1975	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.1	6.1	4.0	3.0	6.1	4.0	3.0	7.0	7.0	3.0	7.0	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	1.00	1.00	1.00	1.00	1.00	1.00	
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00	0.97	1.00	1.00	
Flpb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1657	5603	1543	1700	4856	1434	1767	1847	1420	1755	1951	
Flt Permitted	0.38	1.00	1.00	0.07	1.00	1.00	0.11	1.00	1.00	0.59	1.00	
Satd. Flow (perm)	667	5603	1543	120	4856	1434	209	1847	1420	1089	1951	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	55	2201	471	261	678	216	72	139	110	325	481	43
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	82	0	2	0
Lane Group Flow (vph)	55	2201	471	261	678	216	72	139	28	325	522	0
Confl. Peds. (#/hr)	13					13			12	12		
Heavy Vehicles (%)	7%	1%	1%	5%	8%	7%	1%	4%	5%	1%	1%	2%
Bus Blockages (#/hr)	0	0	6	0	0	6	0	0	10	0	0	0
Turn Type	Perm		Free	pm+pt		Free	pm+pt		Perm	pm+pt		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free	6		Free	8		8	4		
Actuated Green, G (s)	56.4	56.4	140.0	80.1	80.1	140.0	41.0	35.6	35.6	46.6	38.4	
Effective Green, g (s)	56.4	56.4	140.0	80.1	80.1	140.0	41.0	35.6	35.6	46.6	38.4	
Actuated g/C Ratio	0.40	0.40	1.00	0.57	0.57	1.00	0.29	0.25	0.25	0.33	0.27	
Clearance Time (s)	6.1	6.1		3.0	6.1		3.0	7.0	7.0	3.0	7.0	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	269	2257	1543	302	2778	1434	121	470	361	401	535	
v/s Ratio Prot		c0.39		c0.13	0.14		0.02	0.08		c0.05	c0.27	
v/s Ratio Perm	0.08		0.31	0.36		0.15	0.15		0.02	0.22		
v/c Ratio	0.20	0.98	0.31	0.86	0.24	0.15	0.60	0.30	0.08	0.81	0.98	
Uniform Delay, d1	27.2	41.1	0.0	44.2	14.9	0.0	40.2	42.1	39.7	42.4	50.3	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	1.7	13.9	0.5	23.4	0.2	0.2	11.3	0.7	0.2	13.2	32.7	
Delay (s)	28.9	55.1	0.5	67.6	15.1	0.2	51.5	42.8	39.9	55.6	83.0	
Level of Service	C	E	A	E	B	A	D	D	D	E	F	
Approach Delay (s)		45.1			24.2			43.8			72.5	
Approach LOS		D			C			D			E	

Intersection Summary

HCM Average Control Delay	44.8	HCM Level of Service	D
HCM Volume to Capacity ratio	0.94		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	15.1
Intersection Capacity Utilization	103.2%	ICU Level of Service	G
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘	↑↑↑	↗	↘	↑	↗	↘	↗	↘
Volume (vph)	46	905	167	53	2300	421	309	521	314	273	232	119
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	2250	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.1	6.1	4.0	3.0	6.1	4.0	3.0	7.0	7.0	3.0	7.0	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	1.00	1.00	1.00	1.00	1.00	1.00	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.95	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1785	4995	1477	1749	5193	1515	1785	1902	1547	2072	1800	
Flt Permitted	0.07	1.00	1.00	0.23	1.00	1.00	0.24	1.00	1.00	0.12	1.00	
Satd. Flow (perm)	129	4995	1477	418	5193	1515	449	1902	1547	271	1800	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	46	905	167	53	2300	421	309	521	314	273	232	119
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	114	0	14	0
Lane Group Flow (vph)	46	905	167	53	2300	421	309	521	200	273	337	0
Confl. Peds. (#/hr)	2		8	8		2						
Heavy Vehicles (%)	0%	5%	4%	2%	1%	2%	0%	1%	2%	2%	2%	0%
Bus Blockages (#/hr)	0	0	6	0	0	5	0	0	3	0	0	0
Turn Type	Perm		Free	pm+pt		Free	pm+pt		Perm	pm+pt		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free	6		Free	8		8	4		
Actuated Green, G (s)	58.4	58.4	140.0	68.0	68.0	140.0	58.9	42.1	42.1	48.7	34.9	
Effective Green, g (s)	58.4	58.4	140.0	68.0	68.0	140.0	58.9	42.1	42.1	48.7	34.9	
Actuated g/C Ratio	0.42	0.42	1.00	0.49	0.49	1.00	0.42	0.30	0.30	0.35	0.25	
Clearance Time (s)	6.1	6.1		3.0	6.1		3.0	7.0	7.0	3.0	7.0	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	54	2084	1477	266	2522	1515	389	572	465	272	449	
v/s Ratio Prot		0.18		0.01	c0.44		c0.12	0.27		c0.10	0.19	
v/s Ratio Perm	0.36		0.11	0.09		0.28	0.21		0.13	c0.25		
v/c Ratio	0.85	0.43	0.11	0.20	0.91	0.28	0.79	0.91	0.43	1.00	0.75	
Uniform Delay, d1	36.9	29.0	0.0	20.0	33.2	0.0	30.8	47.1	39.3	37.5	48.5	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	84.5	0.7	0.2	0.8	6.4	0.5	12.2	19.6	1.3	55.5	8.2	
Delay (s)	121.4	29.7	0.2	20.8	39.6	0.5	42.9	66.8	40.7	93.0	56.8	
Level of Service	F	C	A	C	D	A	D	E	D	F	E	
Approach Delay (s)		29.1			33.3			53.2			72.6	
Approach LOS		C			C			D			E	

Intersection Summary

HCM Average Control Delay	40.8	HCM Level of Service	D
HCM Volume to Capacity ratio	0.91		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	12.1
Intersection Capacity Utilization	98.9%	ICU Level of Service	F
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘	↑↑↑	↗	↘	↑	↗	↘	↗	↘
Volume (vph)	61	2431	545	313	749	239	80	154	127	359	546	47
Ideal Flow (vphpl)	1900	2050	1900	1900	1900	1900	1900	1900	1900	1900	1975	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.1	6.1	4.0	3.0	6.1	4.0	3.0	7.0	7.0	3.0	7.0	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	1.00	1.00	1.00	1.00	1.00	1.00	
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00	0.97	1.00	1.00	
Flpb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1658	5603	1543	1700	4856	1434	1767	1847	1420	1757	1952	
Flt Permitted	0.36	1.00	1.00	0.06	1.00	1.00	0.12	1.00	1.00	0.54	1.00	
Satd. Flow (perm)	621	5603	1543	114	4856	1434	233	1847	1420	1006	1952	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	61	2431	545	313	749	239	80	154	127	359	546	47
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	98	0	2	0
Lane Group Flow (vph)	61	2431	545	313	749	239	80	154	29	359	591	0
Confl. Peds. (#/hr)	13					13			12	12		
Heavy Vehicles (%)	7%	1%	1%	5%	8%	7%	1%	4%	5%	1%	1%	2%
Bus Blockages (#/hr)	0	0	6	0	0	6	0	0	10	0	0	0
Turn Type	Perm		Free	pm+pt		Free	pm+pt		Perm	pm+pt		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free	6		Free	8		8	4		
Actuated Green, G (s)	59.9	59.9	140.0	80.9	80.9	140.0	37.0	32.0	32.0	46.0	38.0	
Effective Green, g (s)	59.9	59.9	140.0	80.9	80.9	140.0	37.0	32.0	32.0	46.0	38.0	
Actuated g/C Ratio	0.43	0.43	1.00	0.58	0.58	1.00	0.26	0.23	0.23	0.33	0.27	
Clearance Time (s)	6.1	6.1		3.0	6.1		3.0	7.0	7.0	3.0	7.0	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	266	2397	1543	270	2806	1434	116	422	325	390	530	
v/s Ratio Prot		0.43		c0.15	0.15		0.02	0.08		c0.07	c0.30	
v/s Ratio Perm	0.10		0.35	c0.52		0.17	0.16		0.02	0.23		
v/c Ratio	0.23	1.01	0.35	1.16	0.27	0.17	0.69	0.36	0.09	0.92	1.11	
Uniform Delay, d1	25.4	40.0	0.0	47.7	14.7	0.0	42.6	45.4	42.5	44.5	51.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	2.0	22.0	0.6	104.8	0.2	0.3	19.8	1.1	0.2	27.7	74.5	
Delay (s)	27.4	62.0	0.6	152.6	15.0	0.3	62.4	46.6	42.8	72.2	125.5	
Level of Service	C	E	A	F	B	A	E	D	D	E	F	
Approach Delay (s)		50.3			45.4			48.7			105.4	
Approach LOS		D			D			D			F	

Intersection Summary

HCM Average Control Delay	58.4	HCM Level of Service	E
HCM Volume to Capacity ratio	1.06		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	6.0
Intersection Capacity Utilization	113.3%	ICU Level of Service	H
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↑↑↑	↗	↖	↑↑↑	↗	↖	↑	↗	↖	↗	↖
Volume (vph)	51	1000	189	64	2541	465	346	581	362	302	261	131
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	2250	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.1	6.1	4.0	3.0	6.1	4.0	3.0	7.0	7.0	3.0	7.0	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	1.00	1.00	1.00	1.00	1.00	1.00	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.95	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1785	4995	1477	1749	5193	1515	1785	1902	1547	2072	1801	
Flt Permitted	0.07	1.00	1.00	0.20	1.00	1.00	0.19	1.00	1.00	0.11	1.00	
Satd. Flow (perm)	128	4995	1477	360	5193	1515	357	1902	1547	243	1801	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	51	1000	189	64	2541	465	346	581	362	302	261	131
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	77	0	13	0
Lane Group Flow (vph)	51	1000	189	64	2541	465	346	581	285	302	379	0
Confl. Peds. (#/hr)	2		8	8		2						
Heavy Vehicles (%)	0%	5%	4%	2%	1%	2%	0%	1%	2%	2%	2%	0%
Bus Blockages (#/hr)	0	0	6	0	0	5	0	0	3	0	0	0
Turn Type	Perm		Free	pm+pt		Free	pm+pt		Perm	pm+pt		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free	6		Free	8		8	4		
Actuated Green, G (s)	58.5	58.5	140.0	65.5	65.5	140.0	61.4	43.4	43.4	50.9	35.9	
Effective Green, g (s)	58.5	58.5	140.0	65.5	65.5	140.0	61.4	43.4	43.4	50.9	35.9	
Actuated g/C Ratio	0.42	0.42	1.00	0.47	0.47	1.00	0.44	0.31	0.31	0.36	0.26	
Clearance Time (s)	6.1	6.1		3.0	6.1		3.0	7.0	7.0	3.0	7.0	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	53	2087	1477	208	2430	1515	386	590	480	284	462	
v/s Ratio Prot		0.20		0.01	c0.49		c0.14	0.31		c0.11	0.21	
v/s Ratio Perm	0.40		0.13	0.14		0.31	0.25		0.18	c0.27		
v/c Ratio	0.96	0.48	0.13	0.31	1.05	0.31	0.90	0.98	0.59	1.06	0.82	
Uniform Delay, d1	39.7	29.7	0.0	21.9	37.2	0.0	30.8	48.0	40.9	40.8	49.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	113.1	0.8	0.2	1.8	31.6	0.5	23.7	33.1	3.0	71.1	12.5	
Delay (s)	152.8	30.5	0.2	23.6	68.8	0.5	54.5	81.1	43.8	112.0	61.5	
Level of Service	F	C	A	C	E	A	D	F	D	F	E	
Approach Delay (s)		30.9			57.5			63.5			83.4	
Approach LOS		C			E			E			F	

Intersection Summary

HCM Average Control Delay	56.4	HCM Level of Service	E
HCM Volume to Capacity ratio	1.01		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	12.1
Intersection Capacity Utilization	112.1%	ICU Level of Service	H
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘	↑↑↑	↗	↘	↑	↗	↘	↗	↘
Volume (vph)	67	2686	600	343	827	264	88	170	139	397	602	52
Ideal Flow (vphpl)	1900	2050	1900	1900	1900	1900	1900	1900	1900	1900	1975	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.1	6.1	4.0	3.0	6.1	4.0	3.0	7.0	7.0	3.0	7.0	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	1.00	1.00	1.00	1.00	1.00	1.00	
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00	0.97	1.00	1.00	
Flpb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1659	5603	1543	1700	4856	1434	1767	1847	1420	1758	1952	
Flt Permitted	0.33	1.00	1.00	0.06	1.00	1.00	0.12	1.00	1.00	0.51	1.00	
Satd. Flow (perm)	573	5603	1543	114	4856	1434	233	1847	1420	953	1952	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	67	2686	600	343	827	264	88	170	139	397	602	52
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	107	0	2	0
Lane Group Flow (vph)	67	2686	600	343	827	264	88	170	32	397	652	0
Confl. Peds. (#/hr)	13					13			12	12		
Heavy Vehicles (%)	7%	1%	1%	5%	8%	7%	1%	4%	5%	1%	1%	2%
Bus Blockages (#/hr)	0	0	6	0	0	6	0	0	10	0	0	0
Turn Type	Perm		Free	pm+pt		Free	pm+pt		Perm	pm+pt		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free	6		Free	8		8	4		
Actuated Green, G (s)	59.9	59.9	140.0	78.9	78.9	140.0	37.0	32.0	32.0	48.0	40.0	
Effective Green, g (s)	59.9	59.9	140.0	78.9	78.9	140.0	37.0	32.0	32.0	48.0	40.0	
Actuated g/C Ratio	0.43	0.43	1.00	0.56	0.56	1.00	0.26	0.23	0.23	0.34	0.29	
Clearance Time (s)	6.1	6.1		3.0	6.1		3.0	7.0	7.0	3.0	7.0	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	245	2397	1543	246	2737	1434	116	422	325	401	558	
v/s Ratio Prot		0.48		c0.16	0.17		0.03	0.09		c0.09	c0.33	
v/s Ratio Perm	0.12		0.39	c0.63		0.18	0.17		0.02	0.25		
v/c Ratio	0.27	1.12	0.39	1.39	0.30	0.18	0.76	0.40	0.10	0.99	1.17	
Uniform Delay, d1	26.0	40.0	0.0	47.2	16.1	0.0	44.4	45.9	42.6	44.6	50.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	2.7	60.5	0.7	200.4	0.3	0.3	28.3	1.3	0.3	42.3	93.8	
Delay (s)	28.7	100.6	0.7	247.6	16.4	0.3	72.7	47.2	42.9	86.9	143.8	
Level of Service	C	F	A	F	B	A	E	D	D	F	F	
Approach Delay (s)		81.3			68.7			51.3			122.3	
Approach LOS		F			E			D			F	

Intersection Summary

HCM Average Control Delay	83.4	HCM Level of Service	F
HCM Volume to Capacity ratio	1.22		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	6.0
Intersection Capacity Utilization	123.1%	ICU Level of Service	H
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘	↑↑↑	↗	↘	↑	↗	↘	↗	↘
Volume (vph)	56	1104	209	70	2806	514	382	641	398	333	288	145
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	2250	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.1	6.1	4.0	3.0	6.1	4.0	3.0	7.0	7.0	3.0	7.0	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	1.00	1.00	1.00	1.00	1.00	1.00	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.95	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1785	4995	1477	1750	5193	1515	1785	1902	1547	2072	1801	
Flt Permitted	0.07	1.00	1.00	0.17	1.00	1.00	0.11	1.00	1.00	0.12	1.00	
Satd. Flow (perm)	126	4995	1477	307	5193	1515	206	1902	1547	261	1801	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	56	1104	209	70	2806	514	382	641	398	333	288	145
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	68	0	13	0
Lane Group Flow (vph)	56	1104	209	70	2806	514	382	641	330	333	420	0
Confl. Peds. (#/hr)	2		8	8		2						
Heavy Vehicles (%)	0%	5%	4%	2%	1%	2%	0%	1%	2%	2%	2%	0%
Bus Blockages (#/hr)	0	0	6	0	0	5	0	0	3	0	0	0
Turn Type	Perm		Free	pm+pt		Free	pm+pt		Perm	pm+pt		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free	6		Free	8		8	4		
Actuated Green, G (s)	59.5	59.5	140.0	66.5	66.5	140.0	60.4	43.4	43.4	47.4	33.4	
Effective Green, g (s)	59.5	59.5	140.0	66.5	66.5	140.0	60.4	43.4	43.4	47.4	33.4	
Actuated g/C Ratio	0.42	0.42	1.00	0.48	0.48	1.00	0.43	0.31	0.31	0.34	0.24	
Clearance Time (s)	6.1	6.1		3.0	6.1		3.0	7.0	7.0	3.0	7.0	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	54	2123	1477	187	2467	1515	360	590	480	269	430	
v/s Ratio Prot		0.22		0.01	c0.54		c0.18	0.34		c0.12	0.23	
v/s Ratio Perm	0.44		0.14	0.17		0.34	0.28		0.21	c0.29		
v/c Ratio	1.04	0.52	0.14	0.37	1.14	0.34	1.06	1.09	0.69	1.24	0.98	
Uniform Delay, d1	40.2	29.7	0.0	21.8	36.8	0.0	43.9	48.3	42.3	39.4	52.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	133.3	0.9	0.2	2.6	67.4	0.6	64.5	62.7	5.2	134.7	37.4	
Delay (s)	173.6	30.6	0.2	24.5	104.1	0.6	108.4	111.0	47.5	174.2	90.3	
Level of Service	F	C	A	C	F	A	F	F	D	F	F	
Approach Delay (s)		31.8			86.8			92.5			126.8	
Approach LOS		C			F			F			F	

Intersection Summary

HCM Average Control Delay	81.5	HCM Level of Service	F
HCM Volume to Capacity ratio	1.13		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	12.1
Intersection Capacity Utilization	121.7%	ICU Level of Service	H
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

10/17/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘	↑↑	↗	↘↗	↑↗	
Volume (vph)	61	2431	545	313	749	239	80	154	127	359	546	47
Ideal Flow (vphpl)	1900	2050	1900	1900	1900	1900	1900	1900	1900	1900	1975	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.7	6.7	4.0	5.0	6.7	4.0	3.0	7.2	7.2	5.0	7.2	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	1.00	0.95	1.00	0.97	0.95	
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00	0.97	1.00	1.00	
Flpb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1657	5603	1543	3298	4856	1434	1767	3510	1423	3429	3709	
Flt Permitted	0.36	1.00	1.00	0.95	1.00	1.00	0.30	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	620	5603	1543	3298	4856	1434	554	3510	1423	3429	3709	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	61	2431	545	313	749	239	80	154	127	359	546	47
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	106	0	5	0
Lane Group Flow (vph)	61	2431	545	313	749	239	80	154	21	359	588	0
Confl. Peds. (#/hr)	13					13			12	12		
Heavy Vehicles (%)	7%	1%	1%	5%	8%	7%	1%	4%	5%	1%	1%	2%
Bus Blockages (#/hr)	0	0	6	0	0	6	0	0	10	0	0	0
Turn Type	Perm		Free	Prot		Free	pm+pt		Perm	Prot		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free			Free	8		8			
Actuated Green, G (s)	61.5	61.5	145.0	22.6	89.1	145.0	30.0	24.0	24.0	13.0	33.0	
Effective Green, g (s)	61.5	61.5	145.0	22.6	89.1	145.0	30.0	24.0	24.0	13.0	33.0	
Actuated g/C Ratio	0.42	0.42	1.00	0.16	0.61	1.00	0.21	0.17	0.17	0.09	0.23	
Clearance Time (s)	6.7	6.7		5.0	6.7		3.0	7.2	7.2	5.0	7.2	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	263	2376	1543	514	2984	1434	165	581	236	307	844	
v/s Ratio Prot		c0.43		c0.09	0.15		0.02	0.04		c0.10	c0.16	
v/s Ratio Perm	0.10		0.35			0.17	0.08		0.01			
v/c Ratio	0.23	1.02	0.35	0.61	0.25	0.17	0.48	0.27	0.09	1.17	0.70	
Uniform Delay, d1	26.7	41.8	0.0	57.1	12.7	0.0	48.0	52.8	51.2	66.0	51.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	2.1	24.6	0.6	3.0	0.2	0.3	4.6	0.5	0.3	105.5	3.2	
Delay (s)	28.7	66.3	0.6	60.1	12.9	0.3	52.6	53.3	51.6	171.5	54.6	
Level of Service	C	E	A	E	B	A	D	D	D	F	D	
Approach Delay (s)		53.8			22.0			52.5			98.7	
Approach LOS		D			C			D			F	

Intersection Summary

HCM Average Control Delay	53.9	HCM Level of Service	D
HCM Volume to Capacity ratio	0.86		
Actuated Cycle Length (s)	145.0	Sum of lost time (s)	16.7
Intersection Capacity Utilization	97.5%	ICU Level of Service	F
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

10/17/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘	↑↑	↗	↘↗	↑↗	
Volume (vph)	51	1000	189	64	2541	465	346	581	362	302	261	131
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	2250	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.7	6.7	4.0	5.0	6.7	4.0	3.0	7.2	7.2	5.0	7.2	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	1.00	0.95	1.00	0.97	0.95	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.95	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1785	4995	1477	3395	5193	1515	1785	3614	1547	4020	3422	
Flt Permitted	0.06	1.00	1.00	0.95	1.00	1.00	0.39	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	112	4995	1477	3395	5193	1515	726	3614	1547	4020	3422	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	51	1000	189	64	2541	465	346	581	362	302	261	131
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	70	0	20	0
Lane Group Flow (vph)	51	1000	189	64	2541	465	346	581	292	302	372	0
Confl. Peds. (#/hr)	2		8	8		2						
Heavy Vehicles (%)	0%	5%	4%	2%	1%	2%	0%	1%	2%	2%	2%	0%
Bus Blockages (#/hr)	0	0	6	0	0	5	0	0	3	0	0	0
Turn Type	Perm		Free	Prot		Free	pm+pt		Perm	Prot		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free			Free	8		8			
Actuated Green, G (s)	66.9	66.9	145.0	6.1	78.0	145.0	52.3	36.3	36.3	11.8	34.1	
Effective Green, g (s)	66.9	66.9	145.0	6.1	78.0	145.0	52.3	36.3	36.3	11.8	34.1	
Actuated g/C Ratio	0.46	0.46	1.00	0.04	0.54	1.00	0.36	0.25	0.25	0.08	0.24	
Clearance Time (s)	6.7	6.7		5.0	6.7		3.0	7.2	7.2	5.0	7.2	
Vehicle Extension (s)	5.0	5.0		3.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	52	2305	1477	143	2793	1515	379	905	387	327	805	
v/s Ratio Prot		0.20		0.02	c0.49		c0.10	0.16		0.08	0.11	
v/s Ratio Perm	c0.45		0.13			0.31	c0.23		0.19			
v/c Ratio	0.98	0.43	0.13	0.45	0.91	0.31	0.91	0.64	0.76	0.92	0.46	
Uniform Delay, d1	38.4	26.3	0.0	67.8	30.3	0.0	40.6	48.5	50.2	66.2	47.6	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	119.4	0.6	0.2	2.2	5.7	0.5	26.9	2.1	9.6	31.8	0.9	
Delay (s)	157.8	26.9	0.2	70.0	36.0	0.5	67.4	50.7	59.9	97.9	48.5	
Level of Service	F	C	A	E	D	A	E	D	E	F	D	
Approach Delay (s)		28.2			31.3			57.8			70.0	
Approach LOS		C			C			E			E	

Intersection Summary

HCM Average Control Delay	40.4	HCM Level of Service	D
HCM Volume to Capacity ratio	0.97		
Actuated Cycle Length (s)	145.0	Sum of lost time (s)	19.4
Intersection Capacity Utilization	94.6%	ICU Level of Service	F
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

10/17/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘	↑↑	↗	↘↗	↑↗	
Volume (vph)	67	2686	600	343	827	264	88	170	139	397	602	52
Ideal Flow (vphpl)	1900	2050	1900	1900	1900	1900	1900	1900	1900	1900	1975	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.7	6.7	4.0	5.0	6.7	4.0	3.0	7.2	7.2	5.0	7.2	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	1.00	0.95	1.00	0.97	0.95	
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00	0.97	1.00	1.00	
Flpb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1659	5603	1543	3298	4856	1434	1767	3510	1423	3429	3709	
Flt Permitted	0.33	1.00	1.00	0.95	1.00	1.00	0.25	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	573	5603	1543	3298	4856	1434	471	3510	1423	3429	3709	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	67	2686	600	343	827	264	88	170	139	397	602	52
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	114	0	5	0
Lane Group Flow (vph)	67	2686	600	343	827	264	88	170	25	397	649	0
Confl. Peds. (#/hr)	13					13			12	12		
Heavy Vehicles (%)	7%	1%	1%	5%	8%	7%	1%	4%	5%	1%	1%	2%
Bus Blockages (#/hr)	0	0	6	0	0	6	0	0	10	0	0	0
Turn Type	Perm		Free	Prot		Free	pm+pt		Perm	Prot		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free			Free	8		8			
Actuated Green, G (s)	58.1	58.1	145.0	23.8	86.9	145.0	32.2	26.2	26.2	13.0	35.2	
Effective Green, g (s)	58.1	58.1	145.0	23.8	86.9	145.0	32.2	26.2	26.2	13.0	35.2	
Actuated g/C Ratio	0.40	0.40	1.00	0.16	0.60	1.00	0.22	0.18	0.18	0.09	0.24	
Clearance Time (s)	6.7	6.7		5.0	6.7		3.0	7.2	7.2	5.0	7.2	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	230	2245	1543	541	2910	1434	158	634	257	307	900	
v/s Ratio Prot		c0.48		c0.10	0.17		0.02	0.05		c0.12	c0.17	
v/s Ratio Perm	0.12		0.39			0.18	0.10		0.02			
v/c Ratio	0.29	1.20	0.39	0.63	0.28	0.18	0.56	0.27	0.10	1.29	0.72	
Uniform Delay, d1	29.5	43.5	0.0	56.5	14.0	0.0	46.5	51.1	49.5	66.0	50.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	3.2	93.0	0.7	3.4	0.2	0.3	7.1	0.5	0.3	154.1	3.5	
Delay (s)	32.7	136.5	0.7	59.9	14.3	0.3	53.6	51.6	49.9	220.1	53.9	
Level of Service	C	F	A	E	B	A	D	D	D	F	D	
Approach Delay (s)		110.1			22.6			51.5			116.7	
Approach LOS		F			C			D			F	

Intersection Summary

HCM Average Control Delay	87.4	HCM Level of Service	F
HCM Volume to Capacity ratio	0.95		
Actuated Cycle Length (s)	145.0	Sum of lost time (s)	16.7
Intersection Capacity Utilization	104.0%	ICU Level of Service	G
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

10/17/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘	↑↑	↗	↘↗	↑↗	
Volume (vph)	56	1104	209	70	2806	514	382	641	398	333	288	145
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	2250	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.7	6.7	4.0	5.0	6.7	4.0	3.0	7.2	7.2	5.0	7.2	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	1.00	0.95	1.00	0.97	0.95	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.95	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1785	4995	1477	3395	5193	1515	1785	3614	1547	4020	3421	
Flt Permitted	0.06	1.00	1.00	0.95	1.00	1.00	0.40	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	117	4995	1477	3395	5193	1515	753	3614	1547	4020	3421	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	56	1104	209	70	2806	514	382	641	398	333	288	145
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	72	0	13	0
Lane Group Flow (vph)	56	1104	209	70	2806	514	382	641	326	333	420	0
Confl. Peds. (#/hr)	2		8	8		2						
Heavy Vehicles (%)	0%	5%	4%	2%	1%	2%	0%	1%	2%	2%	2%	0%
Bus Blockages (#/hr)	0	0	6	0	0	5	0	0	3	0	0	0
Turn Type	Perm		Free	Prot		Free	pm+pt		Perm	Prot		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free			Free	8		8			
Actuated Green, G (s)	64.3	64.3	140.0	4.6	73.9	140.0	48.4	35.4	35.4	11.8	36.2	
Effective Green, g (s)	64.3	64.3	140.0	4.6	73.9	140.0	48.4	35.4	35.4	11.8	36.2	
Actuated g/C Ratio	0.46	0.46	1.00	0.03	0.53	1.00	0.35	0.25	0.25	0.08	0.26	
Clearance Time (s)	6.7	6.7		5.0	6.7		3.0	7.2	7.2	5.0	7.2	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	54	2294	1477	112	2741	1515	356	914	391	339	885	
v/s Ratio Prot		0.22		0.02	c0.54		c0.10	0.18		0.08	0.12	
v/s Ratio Perm	0.48		0.14			c0.34	c0.27		0.21			
v/c Ratio	1.04	0.48	0.14	0.62	1.02	0.34	1.07	0.70	0.83	0.98	0.47	
Uniform Delay, d1	37.9	26.3	0.0	66.8	33.0	0.0	42.9	47.5	49.5	64.0	43.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	133.3	0.7	0.2	14.4	23.5	0.6	68.5	3.1	15.6	44.1	0.8	
Delay (s)	171.2	27.0	0.2	81.3	56.6	0.6	111.4	50.6	65.2	108.1	44.7	
Level of Service	F	C	A	F	E	A	F	D	E	F	D	
Approach Delay (s)		28.8			48.6			71.0			72.3	
Approach LOS		C			D			E			E	

Intersection Summary

HCM Average Control Delay	51.9	HCM Level of Service	D
HCM Volume to Capacity ratio	1.00		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	12.7
Intersection Capacity Utilization	102.9%	ICU Level of Service	G
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

10/17/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘	↑↑↑	↗	↘↗	↑↗		↘↗	↗	
Volume (vph)	61	2431	545	313	749	239	80	154	127	359	546	47
Ideal Flow (vphpl)	1900	2050	1900	1900	1900	1900	1900	1900	1900	1900	1975	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.6	6.6	4.0	3.0	6.6	4.0	5.0	7.0		5.0	7.0	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	1.00	0.97	0.95		0.97	1.00	
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98	1.00	0.99		1.00	1.00	
Flpb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.93		1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1657	5603	1543	1700	4856	1434	3429	3219		3429	1952	
Flt Permitted	0.36	1.00	1.00	0.07	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	620	5603	1543	119	4856	1434	3429	3219		3429	1952	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	61	2431	545	313	749	239	80	154	127	359	546	47
RTOR Reduction (vph)	0	0	0	0	0	0	0	94	0	0	2	0
Lane Group Flow (vph)	61	2431	545	313	749	239	80	187	0	359	591	0
Confl. Peds. (#/hr)	13					13			12	12		
Heavy Vehicles (%)	7%	1%	1%	5%	8%	7%	1%	4%	5%	1%	1%	2%
Bus Blockages (#/hr)	0	0	6	0	0	6	0	0	10	0	0	0
Turn Type	Perm		Free	pm+pt		Free	Prot			Prot		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free	6		Free						
Actuated Green, G (s)	57.2	57.2	145.0	76.3	76.3	145.0	5.0	38.1		12.0	45.1	
Effective Green, g (s)	57.2	57.2	145.0	76.3	76.3	145.0	5.0	38.1		12.0	45.1	
Actuated g/C Ratio	0.39	0.39	1.00	0.53	0.53	1.00	0.03	0.26		0.08	0.31	
Clearance Time (s)	6.6	6.6		3.0	6.6		5.0	7.0		5.0	7.0	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	245	2210	1543	238	2555	1434	118	846		284	607	
v/s Ratio Prot		0.43		c0.15	0.15		0.02	0.06		c0.10	c0.30	
v/s Ratio Perm	0.10		0.35	c0.55		0.17						
v/c Ratio	0.25	1.10	0.35	1.32	0.29	0.17	0.68	0.22		1.26	0.97	
Uniform Delay, d1	29.5	43.9	0.0	48.1	19.2	0.0	69.2	41.8		66.5	49.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2	2.4	52.7	0.6	168.4	0.3	0.3	18.4	0.3		143.9	30.0	
Delay (s)	31.9	96.6	0.6	216.5	19.5	0.3	87.6	42.1		210.4	79.4	
Level of Service	C	F	A	F	B	A	F	D		F	E	
Approach Delay (s)		78.0			63.4			52.2			128.8	
Approach LOS		E			E			D			F	

Intersection Summary

HCM Average Control Delay	81.6	HCM Level of Service	F
HCM Volume to Capacity ratio	1.13		
Actuated Cycle Length (s)	145.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	114.3%	ICU Level of Service	H
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

10/17/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘	↑↑↑	↗	↘↗	↑↗		↘↗	↗	
Volume (vph)	51	1000	189	64	2541	465	346	581	362	302	261	131
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	2250	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.6	6.6	4.0	5.0	6.6	4.0	5.0	7.0		5.0	7.0	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	1.00	0.97	0.95		0.97	1.00	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00		1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.94		1.00	0.95	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1785	4995	1477	1749	5193	1515	3463	3393		4020	1801	
Flt Permitted	0.06	1.00	1.00	0.20	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	117	4995	1477	365	5193	1515	3463	3393		4020	1801	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	51	1000	189	64	2541	465	346	581	362	302	261	131
RTOR Reduction (vph)	0	0	0	0	0	0	0	66	0	0	3	0
Lane Group Flow (vph)	51	1000	189	64	2541	465	346	877	0	302	389	0
Confl. Peds. (#/hr)	2		8	8		2						
Heavy Vehicles (%)	0%	5%	4%	2%	1%	2%	0%	1%	2%	2%	2%	0%
Bus Blockages (#/hr)	0	0	6	0	0	5	0	0	3	0	0	0
Turn Type	Perm		Free	pm+pt		Free	Prot			Prot		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free	6		Free						
Actuated Green, G (s)	64.0	64.0	145.0	72.2	72.2	145.0	17.0	42.4		11.8	37.2	
Effective Green, g (s)	64.0	64.0	145.0	72.2	72.2	145.0	17.0	42.4		11.8	37.2	
Actuated g/C Ratio	0.44	0.44	1.00	0.50	0.50	1.00	0.12	0.29		0.08	0.26	
Clearance Time (s)	6.6	6.6		5.0	6.6		5.0	7.0		5.0	7.0	
Vehicle Extension (s)	5.0	5.0		3.0	5.0		5.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	52	2205	1477	212	2586	1515	406	992		327	462	
v/s Ratio Prot		0.20		0.01	c0.49		c0.10	c0.26		0.08	0.22	
v/s Ratio Perm	0.43		0.13	0.14		0.31						
v/c Ratio	0.98	0.45	0.13	0.30	0.98	0.31	0.85	0.88		0.92	0.84	
Uniform Delay, d1	39.9	28.3	0.0	20.4	35.8	0.0	62.8	49.0		66.2	51.1	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2	119.4	0.7	0.2	0.8	14.1	0.5	17.1	10.2		31.8	14.3	
Delay (s)	159.2	29.0	0.2	21.2	49.8	0.5	79.9	59.1		97.9	65.4	
Level of Service	F	C	A	C	D	A	E	E		F	E	
Approach Delay (s)		29.9			41.8			64.7			79.6	
Approach LOS		C			D			E			E	

Intersection Summary

HCM Average Control Delay	48.3	HCM Level of Service	D
HCM Volume to Capacity ratio	0.95		
Actuated Cycle Length (s)	145.0	Sum of lost time (s)	18.6
Intersection Capacity Utilization	103.6%	ICU Level of Service	G
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

10/17/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘	↑↑↑	↗	↘↗	↑↗		↘↗	↗	
Volume (vph)	67	2686	600	343	827	264	88	170	139	397	602	52
Ideal Flow (vphpl)	1900	2050	1900	1900	1900	1900	1900	1900	1900	1900	1975	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.6	6.6	4.0	3.0	6.6	4.0	5.0	7.0		5.0	7.0	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	1.00	0.97	0.95		0.97	1.00	
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98	1.00	0.99		1.00	1.00	
Flpb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.93		1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1659	5603	1543	1700	4856	1434	3429	3220		3429	1952	
Flt Permitted	0.33	1.00	1.00	0.07	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	573	5603	1543	119	4856	1434	3429	3220		3429	1952	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	67	2686	600	343	827	264	88	170	139	397	602	52
RTOR Reduction (vph)	0	0	0	0	0	0	0	102	0	0	2	0
Lane Group Flow (vph)	67	2686	600	343	827	264	88	207	0	397	652	0
Confl. Peds. (#/hr)	13					13			12	12		
Heavy Vehicles (%)	7%	1%	1%	5%	8%	7%	1%	4%	5%	1%	1%	2%
Bus Blockages (#/hr)	0	0	6	0	0	6	0	0	10	0	0	0
Turn Type	Perm		Free	pm+pt		Free	Prot			Prot		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free	6		Free						
Actuated Green, G (s)	57.2	57.2	145.0	76.2	76.2	145.0	5.0	38.2		12.0	45.2	
Effective Green, g (s)	57.2	57.2	145.0	76.2	76.2	145.0	5.0	38.2		12.0	45.2	
Actuated g/C Ratio	0.39	0.39	1.00	0.53	0.53	1.00	0.03	0.26		0.08	0.31	
Clearance Time (s)	6.6	6.6		3.0	6.6		5.0	7.0		5.0	7.0	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	226	2210	1543	237	2552	1434	118	848		284	608	
v/s Ratio Prot		0.48		c0.16	0.17		0.03	0.06		c0.12	c0.33	
v/s Ratio Perm	0.12		0.39	c0.60		0.18						
v/c Ratio	0.30	1.22	0.39	1.45	0.32	0.18	0.75	0.24		1.40	1.07	
Uniform Delay, d1	30.1	43.9	0.0	48.1	19.7	0.0	69.4	42.0		66.5	49.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2	3.3	101.3	0.7	223.4	0.3	0.3	26.2	0.3		199.1	57.4	
Delay (s)	33.4	145.2	0.7	271.5	20.0	0.3	95.6	42.4		265.6	107.3	
Level of Service	C	F	A	F	C	A	F	D		F	F	
Approach Delay (s)		117.1			76.5			54.2			167.1	
Approach LOS		F			E			D			F	

Intersection Summary

HCM Average Control Delay	112.2	HCM Level of Service	F
HCM Volume to Capacity ratio	1.24		
Actuated Cycle Length (s)	145.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	123.6%	ICU Level of Service	H
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

10/17/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘	↑↑↑	↗	↘↗	↑↗		↘↗	↗	
Volume (vph)	56	1104	209	70	2806	514	382	641	398	333	288	145
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	2250	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.6	6.6	4.0	3.0	6.6	4.0	5.0	7.0		5.0	7.0	
Lane Util. Factor	1.00	0.91	1.00	1.00	0.91	1.00	0.97	0.95		0.97	1.00	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00		1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.94		1.00	0.95	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1785	4995	1477	1750	5193	1515	3463	3393		4020	1801	
Flt Permitted	0.06	1.00	1.00	0.18	1.00	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (perm)	106	4995	1477	334	5193	1515	3463	3393		4020	1801	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	56	1104	209	70	2806	514	382	641	398	333	288	145
RTOR Reduction (vph)	0	0	0	0	0	0	0	64	0	0	1	0
Lane Group Flow (vph)	56	1104	209	70	2806	514	382	975	0	333	432	0
Confl. Peds. (#/hr)	2		8	8		2						
Heavy Vehicles (%)	0%	5%	4%	2%	1%	2%	0%	1%	2%	2%	2%	0%
Bus Blockages (#/hr)	0	0	6	0	0	5	0	0	3	0	0	0
Turn Type	Perm		Free	pm+pt		Free	Prot			Prot		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free	6		Free						
Actuated Green, G (s)	70.6	70.6	150.0	79.2	79.2	150.0	14.0	40.0		12.2	38.2	
Effective Green, g (s)	70.6	70.6	150.0	79.2	79.2	150.0	14.0	40.0		12.2	38.2	
Actuated g/C Ratio	0.47	0.47	1.00	0.53	0.53	1.00	0.09	0.27		0.08	0.25	
Clearance Time (s)	6.6	6.6		3.0	6.6		5.0	7.0		5.0	7.0	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	50	2351	1477	229	2742	1515	323	905		327	459	
v/s Ratio Prot		0.22		0.01	c0.54		c0.11	c0.29		0.08	0.24	
v/s Ratio Perm	c0.53		0.14	0.15		c0.34						
v/c Ratio	1.12	0.47	0.14	0.31	1.02	0.34	1.18	1.08		1.02	0.94	
Uniform Delay, d1	39.7	27.0	0.0	18.9	35.4	0.0	68.0	55.0		68.9	54.8	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2	164.4	0.7	0.2	1.6	23.4	0.6	109.3	53.1		54.5	28.5	
Delay (s)	204.1	27.7	0.2	20.5	58.8	0.6	177.3	108.1		123.4	83.3	
Level of Service	F	C	A	C	E	A	F	F		F	F	
Approach Delay (s)		30.7			49.2			126.7			100.8	
Approach LOS		C			D			F			F	

Intersection Summary

HCM Average Control Delay	67.1	HCM Level of Service	E
HCM Volume to Capacity ratio	1.11		
Actuated Cycle Length (s)	150.0	Sum of lost time (s)	18.2
Intersection Capacity Utilization	112.2%	ICU Level of Service	H
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘	↑↑	↗	↘	↑↗	
Volume (vph)	55	2201	471	261	678	216	72	139	110	325	481	43
Ideal Flow (vphpl)	1900	2050	1900	1900	1900	1900	1900	1900	1900	1900	1975	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.6	6.6	4.0	5.0	6.6	4.0	3.0	7.2	7.2	3.0	7.2	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	1.00	0.95	1.00	1.00	0.95	
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00	0.98	1.00	1.00	
Flpb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1657	5603	1543	3298	4856	1434	1767	3510	1424	1757	3707	
Flt Permitted	0.38	1.00	1.00	0.95	1.00	1.00	0.30	1.00	1.00	0.59	1.00	
Satd. Flow (perm)	667	5603	1543	3298	4856	1434	563	3510	1424	1096	3707	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	55	2201	471	261	678	216	72	139	110	325	481	43
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	90	0	6	0
Lane Group Flow (vph)	55	2201	471	261	678	216	72	139	20	325	518	0
Confl. Peds. (#/hr)	13					13			12	12		
Heavy Vehicles (%)	7%	1%	1%	5%	8%	7%	1%	4%	5%	1%	1%	2%
Bus Blockages (#/hr)	0	0	6	0	0	6	0	0	10	0	0	0
Turn Type	Perm		Free	Prot		Free	pm+pt		Perm	pm+pt		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free			Free	8		8	4		
Actuated Green, G (s)	66.8	66.8	140.0	18.1	89.9	140.0	28.9	24.9	24.9	36.3	29.3	
Effective Green, g (s)	66.8	66.8	140.0	18.1	89.9	140.0	28.9	24.9	24.9	36.3	29.3	
Actuated g/C Ratio	0.48	0.48	1.00	0.13	0.64	1.00	0.21	0.18	0.18	0.26	0.21	
Clearance Time (s)	6.6	6.6		5.0	6.6		3.0	7.2	7.2	3.0	7.2	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	318	2673	1543	426	3118	1434	151	624	253	324	776	
v/s Ratio Prot		c0.39		c0.08	0.14		0.01	0.04		c0.06	0.14	
v/s Ratio Perm	0.08		0.31			0.15	0.08		0.01	c0.20		
v/c Ratio	0.17	0.82	0.31	0.61	0.22	0.15	0.48	0.22	0.08	1.00	0.67	
Uniform Delay, d1	20.9	31.5	0.0	57.6	10.4	0.0	47.4	49.3	48.0	51.2	50.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	1.2	3.0	0.5	3.7	0.2	0.2	4.9	0.4	0.3	50.8	2.9	
Delay (s)	22.0	34.5	0.5	61.4	10.6	0.2	52.3	49.6	48.2	101.9	53.8	
Level of Service	C	C	A	E	B	A	D	D	D	F	D	
Approach Delay (s)		28.4			20.1			49.8			72.2	
Approach LOS		C			C			D			E	

Intersection Summary

HCM Average Control Delay	35.2	HCM Level of Service	D
HCM Volume to Capacity ratio	0.82		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	14.6
Intersection Capacity Utilization	98.8%	ICU Level of Service	F
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘	↑↑	↗	↘	↑↗	
Volume (vph)	46	905	167	53	2300	421	309	521	314	273	232	119
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	2250	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.6	6.6	4.0	5.0	6.6	4.0	3.0	7.2	7.2	3.0	7.2	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	1.00	0.95	1.00	1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.95	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1785	4995	1477	3395	5193	1515	1785	3614	1547	2072	3419	
Flt Permitted	0.06	1.00	1.00	0.95	1.00	1.00	0.39	1.00	1.00	0.30	1.00	
Satd. Flow (perm)	114	4995	1477	3395	5193	1515	727	3614	1547	663	3419	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	46	905	167	53	2300	421	309	521	314	273	232	119
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	113	0	35	0
Lane Group Flow (vph)	46	905	167	53	2300	421	309	521	201	273	316	0
Confl. Peds. (#/hr)	2		8	8		2						
Heavy Vehicles (%)	0%	5%	4%	2%	1%	2%	0%	1%	2%	2%	2%	0%
Bus Blockages (#/hr)	0	0	6	0	0	5	0	0	3	0	0	0
Turn Type	Perm		Free	Prot		Free	pm+pt		Perm	pm+pt		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free			Free	8		8	4		
Actuated Green, G (s)	66.0	66.0	140.0	7.4	78.4	140.0	47.8	30.6	30.6	41.6	27.4	
Effective Green, g (s)	66.0	66.0	140.0	7.4	78.4	140.0	47.8	30.6	30.6	41.6	27.4	
Actuated g/C Ratio	0.47	0.47	1.00	0.05	0.56	1.00	0.34	0.22	0.22	0.30	0.20	
Clearance Time (s)	6.6	6.6		5.0	6.6		3.0	7.2	7.2	3.0	7.2	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	54	2355	1477	179	2908	1515	380	790	338	340	669	
v/s Ratio Prot		0.18		0.02	c0.44		c0.10	0.14		0.08	0.09	
v/s Ratio Perm	c0.40		0.11			0.28	c0.18		0.13	0.16		
v/c Ratio	0.85	0.38	0.11	0.30	0.79	0.28	0.81	0.66	0.60	0.80	0.47	
Uniform Delay, d1	32.7	23.9	0.0	63.8	24.3	0.0	38.0	49.9	49.1	40.7	49.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	84.5	0.5	0.2	1.9	2.3	0.5	14.0	2.7	4.2	14.5	1.1	
Delay (s)	117.2	24.4	0.2	65.7	26.6	0.5	52.0	52.6	53.4	55.2	51.0	
Level of Service	F	C	A	E	C	A	D	D	D	E	D	
Approach Delay (s)		24.6			23.4			52.7			52.8	
Approach LOS		C			C			D			D	

Intersection Summary

HCM Average Control Delay	32.8	HCM Level of Service	C
HCM Volume to Capacity ratio	0.83		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	16.2
Intersection Capacity Utilization	86.6%	ICU Level of Service	E
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

10/17/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘	↑↑	↗	↘	↑↑	↘↗
Volume (vph)	61	2431	545	313	749	239	80	154	127	359	546	47
Ideal Flow (vphpl)	1900	2050	1900	1900	1900	1900	1900	1900	1900	1900	1975	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.6	6.6	4.0	5.0	6.6	4.0	3.0	7.2	7.2	3.0	7.2	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	1.00	0.95	1.00	1.00	0.95	
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00	0.97	1.00	1.00	
Flpb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1657	5603	1543	3298	4856	1434	1767	3510	1422	1756	3709	
Flt Permitted	0.36	1.00	1.00	0.95	1.00	1.00	0.21	1.00	1.00	0.61	1.00	
Satd. Flow (perm)	620	5603	1543	3298	4856	1434	383	3510	1422	1134	3709	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	61	2431	545	313	749	239	80	154	127	359	546	47
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	102	0	5	0
Lane Group Flow (vph)	61	2431	545	313	749	239	80	154	25	359	588	0
Confl. Peds. (#/hr)	13					13			12	12		
Heavy Vehicles (%)	7%	1%	1%	5%	8%	7%	1%	4%	5%	1%	1%	2%
Bus Blockages (#/hr)	0	0	6	0	0	6	0	0	10	0	0	0
Turn Type	Perm		Free	Prot		Free	pm+pt		Perm	pm+pt		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free			Free	8		8	4		
Actuated Green, G (s)	67.2	67.2	150.0	20.2	92.4	150.0	38.8	30.0	30.0	42.8	32.0	
Effective Green, g (s)	67.2	67.2	150.0	20.2	92.4	150.0	38.8	30.0	30.0	42.8	32.0	
Actuated g/C Ratio	0.45	0.45	1.00	0.13	0.62	1.00	0.26	0.20	0.20	0.29	0.21	
Clearance Time (s)	6.6	6.6		5.0	6.6		3.0	7.2	7.2	3.0	7.2	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	278	2510	1543	444	2991	1434	180	702	284	368	791	
v/s Ratio Prot		c0.43		c0.09	0.15		0.03	0.04		c0.07	0.16	
v/s Ratio Perm	0.10		0.35			0.17	0.09		0.02	c0.21		
v/c Ratio	0.22	0.97	0.35	0.70	0.25	0.17	0.44	0.22	0.09	0.98	0.74	
Uniform Delay, d1	25.3	40.4	0.0	62.1	13.1	0.0	44.1	50.2	48.9	51.7	55.2	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	1.8	12.0	0.6	6.3	0.2	0.3	3.6	0.3	0.3	40.5	4.6	
Delay (s)	27.2	52.3	0.6	68.3	13.3	0.3	47.7	50.5	49.2	92.2	59.7	
Level of Service	C	D	A	E	B	A	D	D	D	F	E	
Approach Delay (s)		42.5			24.1			49.4			72.0	
Approach LOS		D			C			D			E	

Intersection Summary

HCM Average Control Delay	43.7	HCM Level of Service	D
HCM Volume to Capacity ratio	0.91		
Actuated Cycle Length (s)	150.0	Sum of lost time (s)	17.6
Intersection Capacity Utilization	106.3%	ICU Level of Service	G
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

10/17/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘	↑↑	↗	↘	↑↗	
Volume (vph)	51	1000	189	64	2541	465	346	581	362	302	261	131
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	2250	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.6	6.6	4.0	5.0	6.6	4.0	3.0	7.2	7.2	3.0	7.2	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	1.00	0.95	1.00	1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.95	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1785	4995	1477	3395	5193	1515	1785	3614	1547	2072	3422	
Flt Permitted	0.06	1.00	1.00	0.95	1.00	1.00	0.39	1.00	1.00	0.27	1.00	
Satd. Flow (perm)	117	4995	1477	3395	5193	1515	729	3614	1547	595	3422	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	51	1000	189	64	2541	465	346	581	362	302	261	131
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	84	0	26	0
Lane Group Flow (vph)	51	1000	189	64	2541	465	346	581	278	302	366	0
Confl. Peds. (#/hr)	2		8	8		2						
Heavy Vehicles (%)	0%	5%	4%	2%	1%	2%	0%	1%	2%	2%	2%	0%
Bus Blockages (#/hr)	0	0	6	0	0	5	0	0	3	0	0	0
Turn Type	Perm		Free	Prot		Free	pm+pt		Perm	pm+pt		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free			Free	8		8	4		
Actuated Green, G (s)	64.3	64.3	140.0	5.2	74.5	140.0	50.9	34.9	34.9	46.5	32.7	
Effective Green, g (s)	64.3	64.3	140.0	5.2	74.5	140.0	50.9	34.9	34.9	46.5	32.7	
Actuated g/C Ratio	0.46	0.46	1.00	0.04	0.53	1.00	0.36	0.25	0.25	0.33	0.23	
Clearance Time (s)	6.6	6.6		5.0	6.6		3.0	7.2	7.2	3.0	7.2	
Vehicle Extension (s)	5.0	5.0		3.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	54	2294	1477	126	2763	1515	386	901	386	343	799	
v/s Ratio Prot		0.20		0.02	c0.49		c0.10	0.16		0.09	0.11	
v/s Ratio Perm	0.44		0.13			0.31	c0.22		0.18	0.21		
v/c Ratio	0.94	0.44	0.13	0.51	0.92	0.31	0.90	0.64	0.72	0.88	0.46	
Uniform Delay, d1	36.1	25.6	0.0	66.1	30.0	0.0	38.4	47.0	48.1	39.1	46.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	107.2	0.6	0.2	3.2	6.3	0.5	23.7	2.2	7.9	23.6	0.9	
Delay (s)	143.3	26.2	0.2	69.3	36.4	0.5	62.1	49.2	56.0	62.6	46.9	
Level of Service	F	C	A	E	D	A	E	D	E	E	D	
Approach Delay (s)		27.0			31.6			54.6			53.8	
Approach LOS		C			C			D			D	

Intersection Summary

HCM Average Control Delay	37.9	HCM Level of Service	D
HCM Volume to Capacity ratio	0.90		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	12.6
Intersection Capacity Utilization	94.5%	ICU Level of Service	F
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

10/17/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘	↑↑	↗	↘	↑↗	
Volume (vph)	67	2686	600	343	827	264	88	170	139	397	602	52
Ideal Flow (vphpl)	1900	2050	1900	1900	1900	1900	1900	1900	1900	1900	1975	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.6	6.6	4.0	5.0	6.6	4.0	3.0	7.2	7.2	3.0	7.2	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	1.00	0.95	1.00	1.00	0.95	
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00	0.97	1.00	1.00	
Flpb, ped/bikes	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1659	5603	1543	3298	4856	1434	1767	3510	1423	1756	3709	
Flt Permitted	0.33	1.00	1.00	0.95	1.00	1.00	0.18	1.00	1.00	0.63	1.00	
Satd. Flow (perm)	573	5603	1543	3298	4856	1434	327	3510	1423	1155	3709	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	67	2686	600	343	827	264	88	170	139	397	602	52
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	108	0	5	0
Lane Group Flow (vph)	67	2686	600	343	827	264	88	170	31	397	649	0
Confl. Peds. (#/hr)	13					13			12	12		
Heavy Vehicles (%)	7%	1%	1%	5%	8%	7%	1%	4%	5%	1%	1%	2%
Bus Blockages (#/hr)	0	0	6	0	0	6	0	0	10	0	0	0
Turn Type	Perm		Free	Prot		Free	pm+pt		Perm	pm+pt		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free			Free	8		8	4		
Actuated Green, G (s)	65.2	65.2	145.0	17.7	87.9	145.0	39.3	32.3	32.3	41.3	33.3	
Effective Green, g (s)	65.2	65.2	145.0	17.7	87.9	145.0	39.3	32.3	32.3	41.3	33.3	
Actuated g/C Ratio	0.45	0.45	1.00	0.12	0.61	1.00	0.27	0.22	0.22	0.28	0.23	
Clearance Time (s)	6.6	6.6		5.0	6.6		3.0	7.2	7.2	3.0	7.2	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	258	2519	1543	403	2944	1434	158	782	317	362	852	
v/s Ratio Prot		c0.48		c0.10	0.17		0.03	0.05		c0.06	0.18	
v/s Ratio Perm	0.12		0.39			0.18	0.12		0.02	c0.25		
v/c Ratio	0.26	1.07	0.39	0.85	0.28	0.18	0.56	0.22	0.10	1.10	0.76	
Uniform Delay, d1	24.9	39.9	0.0	62.4	13.6	0.0	41.8	46.0	44.8	50.7	52.2	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	2.4	38.7	0.7	17.1	0.2	0.3	7.1	0.3	0.3	75.9	4.8	
Delay (s)	27.3	78.6	0.7	79.4	13.8	0.3	48.9	46.3	45.1	126.5	56.9	
Level of Service	C	E	A	E	B	A	D	D	D	F	E	
Approach Delay (s)		63.6			27.0			46.4			83.2	
Approach LOS		E			C			D			F	

Intersection Summary

HCM Average Control Delay	57.4	HCM Level of Service	E
HCM Volume to Capacity ratio	1.02		
Actuated Cycle Length (s)	145.0	Sum of lost time (s)	17.6
Intersection Capacity Utilization	113.8%	ICU Level of Service	H
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Argentia Road

10/17/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘	↑↑	↗	↘	↑↗	
Volume (vph)	56	1104	209	70	2806	514	382	641	398	333	288	145
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	2250	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.7
Total Lost time (s)	6.6	6.6	4.0	5.0	6.6	4.0	3.0	7.2	7.2	3.0	7.2	
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	1.00	0.95	1.00	1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.95	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1785	4995	1477	3395	5193	1515	1785	3614	1547	2072	3421	
Flt Permitted	0.07	1.00	1.00	0.95	1.00	1.00	0.40	1.00	1.00	0.21	1.00	
Satd. Flow (perm)	122	4995	1477	3395	5193	1515	750	3614	1547	449	3421	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	56	1104	209	70	2806	514	382	641	398	333	288	145
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	83	0	20	0
Lane Group Flow (vph)	56	1104	209	70	2806	514	382	641	315	333	413	0
Confl. Peds. (#/hr)	2		8	8		2						
Heavy Vehicles (%)	0%	5%	4%	2%	1%	2%	0%	1%	2%	2%	2%	0%
Bus Blockages (#/hr)	0	0	6	0	0	5	0	0	3	0	0	0
Turn Type	Perm		Free	Prot		Free	pm+pt		Perm	pm+pt		
Protected Phases		2		1	6		3	8		7	4	
Permitted Phases	2		Free			Free	8		8	4		
Actuated Green, G (s)	61.4	61.4	140.0	4.9	71.3	140.0	51.1	35.1	35.1	52.7	35.9	
Effective Green, g (s)	61.4	61.4	140.0	4.9	71.3	140.0	51.1	35.1	35.1	52.7	35.9	
Actuated g/C Ratio	0.44	0.44	1.00	0.04	0.51	1.00	0.37	0.25	0.25	0.38	0.26	
Clearance Time (s)	6.6	6.6		5.0	6.6		3.0	7.2	7.2	3.0	7.2	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	
Lane Grp Cap (vph)	54	2191	1477	119	2645	1515	392	906	388	364	877	
v/s Ratio Prot		0.22		0.02	c0.54		c0.11	0.18		c0.11	0.12	
v/s Ratio Perm	0.46		0.14			0.34	c0.24		0.20	0.23		
v/c Ratio	1.04	0.50	0.14	0.59	1.06	0.34	0.97	0.71	0.81	0.91	0.47	
Uniform Delay, d1	39.3	28.3	0.0	66.6	34.4	0.0	40.1	47.8	49.3	34.2	44.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	133.3	0.8	0.2	11.0	36.3	0.6	38.8	3.2	13.6	28.0	0.8	
Delay (s)	172.6	29.2	0.2	77.6	70.7	0.6	79.0	51.0	63.0	62.2	44.9	
Level of Service	F	C	A	E	E	A	E	D	E	E	D	
Approach Delay (s)		30.6			60.2			61.8			52.4	
Approach LOS		C			E			E			D	

Intersection Summary

HCM Average Control Delay	53.8	HCM Level of Service	D
HCM Volume to Capacity ratio	1.01		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	15.6
Intersection Capacity Utilization	102.8%	ICU Level of Service	G
Analysis Period (min)	15		
c Critical Lane Group			

Appendix E

Synchro HCM Reports – Study Area Intersections

HCM Signalized Intersection Capacity Analysis

3: Argentia Road & Mississauga Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑	↗	↘	↑↑	↗	↘	↑↑↑	↗	↘↗	↑↑↑	↗
Volume (vph)	75	357	28	169	338	275	127	2446	455	726	1297	500
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	2150	1900	2100	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	7.8	7.8	4.0	3.0	7.8	4.0	6.5	6.5	4.0	5.0	6.5	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00	0.99	1.00	1.00	0.98	1.00	1.00	0.98
Flpb, ped/bikes	1.00	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1607	3510	1293	1705	3510	1461	1782	5762	1541	3716	4902	1547
Flt Permitted	0.55	1.00	1.00	0.35	1.00	1.00	0.20	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	927	3510	1293	623	3510	1461	377	5762	1541	3716	4902	1547
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	75	357	28	169	338	275	127	2446	455	726	1297	500
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	75	357	28	169	338	275	127	2446	455	726	1297	500
Confl. Peds. (#/hr)	1		28	28		1	3		13	13		3
Heavy Vehicles (%)	11%	4%	21%	4%	4%	8%	0%	3%	2%	3%	7%	1%
Turn Type	Perm		Free	pm+pt		Free	Perm		Free	Prot		Free
Protected Phases		4		3	8			2		1	6	
Permitted Phases	4		Free	8		Free	2		Free			Free
Actuated Green, G (s)	22.7	22.7	142.2	33.9	33.9	142.2	60.7	60.7	142.2	28.3	94.0	142.2
Effective Green, g (s)	22.7	22.7	142.2	33.9	33.9	142.2	60.7	60.7	142.2	28.3	94.0	142.2
Actuated g/C Ratio	0.16	0.16	1.00	0.24	0.24	1.00	0.43	0.43	1.00	0.20	0.66	1.00
Clearance Time (s)	7.8	7.8		3.0	7.8		6.5	6.5		5.0	6.5	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	148	560	1293	211	837	1461	161	2460	1541	740	3240	1547
v/s Ratio Prot		0.10		c0.05	0.10			c0.42		c0.20	0.26	
v/s Ratio Perm	0.08		0.02	c0.14		0.19	0.34		0.30			0.32
v/c Ratio	0.51	0.64	0.02	0.80	0.40	0.19	0.79	0.99	0.30	0.98	0.40	0.32
Uniform Delay, d1	54.6	55.9	0.0	49.2	45.6	0.0	35.2	40.6	0.0	56.7	11.1	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	5.6	3.3	0.0	21.6	0.7	0.3	31.4	16.9	0.5	28.5	0.4	0.6
Delay (s)	60.2	59.2	0.0	70.8	46.3	0.3	66.6	57.4	0.5	85.1	11.5	0.6
Level of Service	E	E	A	E	D	A	E	E	A	F	B	A
Approach Delay (s)		55.8			35.4			49.3			30.5	
Approach LOS		E			D			D			C	

Intersection Summary

HCM Average Control Delay	41.1	HCM Level of Service	D
HCM Volume to Capacity ratio	0.90		
Actuated Cycle Length (s)	142.2	Sum of lost time (s)	14.5
Intersection Capacity Utilization	107.3%	ICU Level of Service	G
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Argentia Road & Mississauga Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑	↗	↘	↑↑	↗	↘	↑↑↑	↗	↘↗	↑↑↑	↗
Volume (vph)	369	212	89	468	409	1147	42	1629	134	246	2032	64
Ideal Flow (vphpl)	1900	1900	1900	2050	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	3.0	7.8	4.0	3.0	7.8	4.0	3.0	6.5	4.0	5.0	6.5	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1748	3544	1566	1907	3544	1545	1785	5043	1575	3267	5092	1507
Flt Permitted	0.30	1.00	1.00	0.62	1.00	1.00	0.07	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	550	3544	1566	1242	3544	1545	139	5043	1575	3267	5092	1507
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	369	212	89	468	409	1147	42	1629	134	246	2032	64
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	369	212	89	468	409	1147	42	1629	134	246	2032	64
Confl. Peds. (#/hr)	5					5			6	6		
Heavy Vehicles (%)	2%	3%	2%	1%	3%	2%	0%	4%	0%	6%	3%	6%
Turn Type	pm+pt		Free	pm+pt		Free	pm+pt		Free	Prot		Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free	8		Free	2		Free			Free
Actuated Green, G (s)	49.8	27.4	140.0	42.9	23.5	140.0	59.9	54.0	140.0	16.9	67.0	140.0
Effective Green, g (s)	49.8	27.4	140.0	42.9	23.5	140.0	59.9	54.0	140.0	16.9	67.0	140.0
Actuated g/C Ratio	0.36	0.20	1.00	0.31	0.17	1.00	0.43	0.39	1.00	0.12	0.48	1.00
Clearance Time (s)	3.0	7.8		3.0	7.8		3.0	6.5		5.0	6.5	
Vehicle Extension (s)	3.0	5.0		5.0	5.0		3.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	395	694	1566	473	595	1545	129	1945	1575	394	2437	1507
v/s Ratio Prot	c0.16	0.06		c0.14	0.12		0.01	0.32		0.08	c0.40	
v/s Ratio Perm	c0.18		0.06	0.17		c0.74	0.13		0.09			0.04
v/c Ratio	0.93	0.31	0.06	0.99	0.69	0.74	0.33	0.84	0.09	0.62	0.83	0.04
Uniform Delay, d1	38.0	48.2	0.0	46.0	54.8	0.0	27.6	39.0	0.0	58.5	31.7	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	29.0	0.5	0.1	38.4	4.2	3.3	1.5	4.5	0.1	4.3	3.5	0.1
Delay (s)	67.0	48.7	0.1	84.5	59.0	3.3	29.1	43.5	0.1	62.9	35.2	0.1
Level of Service	E	D	A	F	E	A	C	D	A	E	D	A
Approach Delay (s)		52.3			33.3			40.0			37.2	
Approach LOS		D			C			D			D	

Intersection Summary		
HCM Average Control Delay	38.2	HCM Level of Service
HCM Volume to Capacity ratio	0.80	D
Actuated Cycle Length (s)	140.0	Sum of lost time (s)
Intersection Capacity Utilization	95.6%	3.0
Analysis Period (min)	15	ICU Level of Service
		F

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Argentia Road & Mississauga Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑	↗	↘	↑↑	↗	↘	↑↑↑	↗	↘↗	↑↑↑	↗
Volume (vph)	83	394	31	187	373	304	140	2702	503	802	1433	552
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	2150	1900	2100	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	7.8	7.8	4.0	3.0	7.8	4.0	6.5	6.5	4.0	5.0	6.5	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00	0.99	1.00	1.00	0.98	1.00	1.00	0.98
Flpb, ped/bikes	1.00	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1607	3510	1293	1707	3510	1461	1782	5762	1541	3716	4902	1547
Flt Permitted	0.53	1.00	1.00	0.32	1.00	1.00	0.17	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	896	3510	1293	570	3510	1461	327	5762	1541	3716	4902	1547
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	83	394	31	187	373	304	140	2702	503	802	1433	552
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	83	394	31	187	373	304	140	2702	503	802	1433	552
Confl. Peds. (#/hr)	1		28	28		1	3		13	13		3
Heavy Vehicles (%)	11%	4%	21%	4%	4%	8%	0%	3%	2%	3%	7%	1%
Turn Type	Perm		Free	pm+pt		Free	Perm		Free	Prot		Free
Protected Phases		4		3	8			2		1	6	
Permitted Phases	4		Free	8		Free	2		Free			Free
Actuated Green, G (s)	23.6	23.6	140.0	31.6	31.6	140.0	59.7	59.7	140.0	29.4	94.1	140.0
Effective Green, g (s)	23.6	23.6	140.0	31.6	31.6	140.0	59.7	59.7	140.0	29.4	94.1	140.0
Actuated g/C Ratio	0.17	0.17	1.00	0.23	0.23	1.00	0.43	0.43	1.00	0.21	0.67	1.00
Clearance Time (s)	7.8	7.8		3.0	7.8		6.5	6.5		5.0	6.5	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	151	592	1293	169	792	1461	139	2457	1541	780	3295	1547
v/s Ratio Prot		0.11		c0.04	0.11			c0.47		c0.22	0.29	
v/s Ratio Perm	0.09		0.02	c0.21		0.21	0.43		0.33			0.36
v/c Ratio	0.55	0.67	0.02	1.11	0.47	0.21	1.01	1.10	0.33	1.03	0.43	0.36
Uniform Delay, d1	53.3	54.5	0.0	54.3	47.0	0.0	40.1	40.1	0.0	55.3	10.6	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	7.0	3.7	0.0	100.6	0.9	0.3	78.2	51.9	0.6	39.6	0.4	0.6
Delay (s)	60.4	58.2	0.0	154.9	47.9	0.3	118.4	92.0	0.6	94.9	11.1	0.6
Level of Service	E	E	A	F	D	A	F	F	A	F	B	A
Approach Delay (s)		55.0			54.3			79.4			33.1	
Approach LOS		E			D			E			C	

Intersection Summary

HCM Average Control Delay	57.7	HCM Level of Service	E
HCM Volume to Capacity ratio	1.04		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	14.5
Intersection Capacity Utilization	115.1%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Argentia Road & Mississauga Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑	↗	↘	↑↑	↗	↘	↑↑↑	↗	↘↗	↑↑↑	↗
Volume (vph)	408	234	98	517	452	1267	46	1799	148	272	2245	71
Ideal Flow (vphpl)	1900	1900	1900	2050	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	3.0	7.8	4.0	3.0	7.8	4.0	3.0	6.5	4.0	5.0	6.5	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1749	3544	1566	1907	3544	1545	1785	5043	1575	3267	5092	1507
Flt Permitted	0.29	1.00	1.00	0.61	1.00	1.00	0.07	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	525	3544	1566	1216	3544	1545	131	5043	1575	3267	5092	1507
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	408	234	98	517	452	1267	46	1799	148	272	2245	71
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	408	234	98	517	452	1267	46	1799	148	272	2245	71
Confl. Peds. (#/hr)	5					5			6	6		
Heavy Vehicles (%)	2%	3%	2%	1%	3%	2%	0%	4%	0%	6%	3%	6%
Turn Type	pm+pt		Free	pm+pt		Free	pm+pt		Free	Prot		Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free	8		Free	2		Free			Free
Actuated Green, G (s)	46.7	25.7	140.0	44.7	24.7	140.0	62.0	57.3	140.0	14.7	69.3	140.0
Effective Green, g (s)	46.7	25.7	140.0	44.7	24.7	140.0	62.0	57.3	140.0	14.7	69.3	140.0
Actuated g/C Ratio	0.33	0.18	1.00	0.32	0.18	1.00	0.44	0.41	1.00	0.10	0.49	1.00
Clearance Time (s)	3.0	7.8		3.0	7.8		3.0	6.5		5.0	6.5	
Vehicle Extension (s)	3.0	5.0		5.0	5.0		3.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	359	651	1566	487	625	1545	114	2064	1575	343	2521	1507
v/s Ratio Prot	c0.17	0.07		0.15	0.13		0.01	0.36		0.08	c0.44	
v/s Ratio Perm	c0.21		0.06	0.19		c0.82	0.17		0.09			0.05
v/c Ratio	1.14	0.36	0.06	1.06	0.72	0.82	0.40	0.87	0.09	0.79	0.89	0.05
Uniform Delay, d1	41.5	50.0	0.0	45.1	54.4	0.0	29.0	38.0	0.0	61.2	31.9	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	89.9	0.7	0.1	58.1	5.1	5.0	2.3	5.4	0.1	13.5	5.2	0.1
Delay (s)	131.5	50.7	0.1	103.3	59.5	5.0	31.3	43.4	0.1	74.7	37.2	0.1
Level of Service	F	D	A	F	E	A	C	D	A	E	D	A
Approach Delay (s)		88.5			38.7			39.9			40.1	
Approach LOS		F			D			D			D	

Intersection Summary

HCM Average Control Delay	44.4	HCM Level of Service	D
HCM Volume to Capacity ratio	0.90		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	3.0
Intersection Capacity Utilization	102.9%	ICU Level of Service	G
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Argentia Road & Mississauga Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑	↗	↘	↑↑	↗	↘	↑↑↑	↗	↘↗	↑↑↑	↗
Volume (vph)	92	436	34	206	412	336	155	2985	555	886	1583	610
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	2150	1900	2100	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	7.8	7.8	4.0	3.0	7.8	4.0	6.5	6.5	4.0	5.0	6.5	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00	0.99	1.00	1.00	0.98	1.00	1.00	0.98
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1607	3510	1293	1709	3510	1461	1783	5762	1541	3716	4902	1547
Flt Permitted	0.51	1.00	1.00	0.28	1.00	1.00	0.15	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	863	3510	1293	507	3510	1461	279	5762	1541	3716	4902	1547
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	92	436	34	206	412	336	155	2985	555	886	1583	610
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	92	436	34	206	412	336	155	2985	555	886	1583	610
Confl. Peds. (#/hr)	1		28	28		1	3		13	13		3
Heavy Vehicles (%)	11%	4%	21%	4%	4%	8%	0%	3%	2%	3%	7%	1%
Turn Type	Perm		Free	pm+pt		Free	Perm		Free	Prot		Free
Protected Phases		4		3	8			2		1	6	
Permitted Phases	4		Free	8		Free	2		Free			Free
Actuated Green, G (s)	24.7	24.7	140.0	32.7	32.7	140.0	60.7	60.7	140.0	27.3	93.0	140.0
Effective Green, g (s)	24.7	24.7	140.0	32.7	32.7	140.0	60.7	60.7	140.0	27.3	93.0	140.0
Actuated g/C Ratio	0.18	0.18	1.00	0.23	0.23	1.00	0.43	0.43	1.00	0.20	0.66	1.00
Clearance Time (s)	7.8	7.8		3.0	7.8		6.5	6.5		5.0	6.5	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	152	619	1293	161	820	1461	121	2498	1541	725	3256	1547
v/s Ratio Prot		0.12		c0.05	0.12			0.52		c0.24	0.32	
v/s Ratio Perm	0.11		0.03	c0.25		0.23	c0.56		0.36			0.39
v/c Ratio	0.61	0.70	0.03	1.28	0.50	0.23	1.28	1.19	0.36	1.22	0.49	0.39
Uniform Delay, d1	53.2	54.2	0.0	53.7	46.6	0.0	39.6	39.6	0.0	56.4	11.7	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	9.6	4.6	0.0	164.8	1.0	0.4	175.3	91.9	0.7	112.1	0.5	0.8
Delay (s)	62.8	58.8	0.0	218.5	47.6	0.4	215.0	131.6	0.7	168.5	12.2	0.8
Level of Service	E	E	A	F	D	A	F	F	A	F	B	A
Approach Delay (s)		55.9			67.9			115.4			54.9	
Approach LOS		E			E			F			D	

Intersection Summary

HCM Average Control Delay	83.4	HCM Level of Service	F
HCM Volume to Capacity ratio	1.21		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	14.5
Intersection Capacity Utilization	123.6%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Argentia Road & Mississauga Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	450	259	109	571	499	1400	51	1988	164	300	2479	78
Ideal Flow (vphpl)	1900	1900	1900	2050	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	3.0	7.8	4.0	3.0	7.8	4.0	3.0	6.5	4.0	5.0	6.5	4.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1749	3544	1566	1907	3544	1545	1785	5043	1575	3267	5092	1507
Flt Permitted	0.25	1.00	1.00	0.58	1.00	1.00	0.07	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	452	3544	1566	1165	3544	1545	135	5043	1575	3267	5092	1507
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	450	259	109	571	499	1400	51	1988	164	300	2479	78
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	450	259	109	571	499	1400	51	1988	164	300	2479	78
Confl. Peds. (#/hr)	5					5			6	6		
Heavy Vehicles (%)	2%	3%	2%	1%	3%	2%	0%	4%	0%	6%	3%	6%
Turn Type	pm+pt		Free	pm+pt		Free	pm+pt		Free	Prot		Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free	8		Free	2		Free			Free
Actuated Green, G (s)	48.7	26.7	140.0	46.7	25.7	140.0	60.1	55.7	140.0	14.3	67.6	140.0
Effective Green, g (s)	48.7	26.7	140.0	46.7	25.7	140.0	60.1	55.7	140.0	14.3	67.6	140.0
Actuated g/C Ratio	0.35	0.19	1.00	0.33	0.18	1.00	0.43	0.40	1.00	0.10	0.48	1.00
Clearance Time (s)	3.0	7.8		3.0	7.8		3.0	6.5		5.0	6.5	
Vehicle Extension (s)	3.0	5.0		5.0	5.0		3.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	361	676	1566	500	651	1545	110	2006	1575	334	2459	1507
v/s Ratio Prot	c0.20	0.07		0.17	0.14		0.01	0.39		0.09	c0.49	
v/s Ratio Perm	c0.24		0.07	0.21		c0.91	0.18		0.10			0.05
v/c Ratio	1.25	0.38	0.07	1.14	0.77	0.91	0.46	0.99	0.10	0.90	1.01	0.05
Uniform Delay, d1	39.6	49.5	0.0	43.9	54.3	0.0	33.3	41.9	0.0	62.1	36.2	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	132.1	0.8	0.1	85.5	6.3	9.2	3.1	18.1	0.1	26.7	20.1	0.1
Delay (s)	171.7	50.2	0.1	129.4	60.6	9.2	36.4	60.0	0.1	88.8	56.3	0.1
Level of Service	F	D	A	F	E	A	D	E	A	F	E	A
Approach Delay (s)		110.4			47.4			55.0			58.2	
Approach LOS		F			D			D			E	

Intersection Summary

HCM Average Control Delay	59.3	HCM Level of Service	E
HCM Volume to Capacity ratio	1.00		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	3.0
Intersection Capacity Utilization	110.8%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Argentia Road & Winston Churchill Boulevard

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖↗	↑↑	↖	↖	↑↑	↖	↖↗	↑↑↑	↖	↖↗	↑↑↑	↖
Volume (vph)	376	313	215	119	101	284	188	900	297	497	564	152
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	5.0	7.0	7.0	3.0	7.0	7.0	5.0	7.0	4.0	5.0	7.0	7.0
Lane Util. Factor	0.97	0.95	1.00	1.00	0.95	1.00	0.97	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.97	1.00	1.00	0.97	1.00	1.00	0.99	1.00	1.00	0.97
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3298	3544	1415	1708	3318	1456	3298	5092	1560	3330	4995	1399
Flt Permitted	0.95	1.00	1.00	0.56	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3298	3544	1415	1010	3318	1456	3298	5092	1560	3330	4995	1399
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	376	313	215	119	101	284	188	900	297	497	564	152
RTOR Reduction (vph)	0	0	169	0	0	192	0	0	0	0	0	82
Lane Group Flow (vph)	376	313	46	119	101	92	188	900	297	497	564	70
Confl. Peds. (#/hr)	19		12	12		19	4		4	4		4
Heavy Vehicles (%)	5%	3%	10%	4%	10%	6%	5%	3%	1%	4%	5%	11%
Turn Type	Prot		Perm	pm+pt		Perm	Prot		Free	Prot		Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4	8		8			Free			6
Actuated Green, G (s)	20.6	29.8	29.8	29.0	18.1	18.1	13.2	43.4	140.0	33.9	64.1	64.1
Effective Green, g (s)	20.6	29.8	29.8	29.0	18.1	18.1	13.2	43.4	140.0	33.9	64.1	64.1
Actuated g/C Ratio	0.15	0.21	0.21	0.21	0.13	0.13	0.09	0.31	1.00	0.24	0.46	0.46
Clearance Time (s)	5.0	7.0	7.0	3.0	7.0	7.0	5.0	7.0		5.0	7.0	7.0
Vehicle Extension (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.0		5.0	5.0	5.0
Lane Grp Cap (vph)	485	754	301	264	429	188	311	1579	1560	806	2287	641
v/s Ratio Prot	c0.11	0.09		0.04	0.03		0.06	c0.18		c0.15	0.11	
v/s Ratio Perm			0.03	0.06		c0.06			0.19			0.05
v/c Ratio	0.78	0.42	0.15	0.45	0.24	0.49	0.60	0.57	0.19	0.62	0.25	0.11
Uniform Delay, d1	57.5	47.6	44.8	47.3	54.7	56.6	60.9	40.5	0.0	47.3	23.2	21.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	8.8	0.8	0.5	2.5	0.6	4.1	3.3	1.5	0.3	2.0	0.3	0.3
Delay (s)	66.3	48.4	45.3	49.8	55.3	60.7	64.2	42.0	0.3	49.3	23.5	22.0
Level of Service	E	D	D	D	E	E	E	D	A	D	C	C
Approach Delay (s)		55.1			57.1			36.1			33.9	
Approach LOS		E			E			D			C	

Intersection Summary

HCM Average Control Delay	42.3	HCM Level of Service	D
HCM Volume to Capacity ratio	0.61		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	24.0
Intersection Capacity Utilization	93.4%	ICU Level of Service	F
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Argentia Road & Winston Churchill Boulevard

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	512	410	228	246	493	430	329	910	202	298	974	295
Ideal Flow (vphpl)	2200	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	5.0	7.0	7.0	3.0	7.0	7.0	5.0	7.0	4.0	5.0	7.0	7.0
Lane Util. Factor	0.97	0.95	1.00	1.00	0.95	1.00	0.97	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.96	1.00	1.00	0.99	1.00	1.00	0.95
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3855	3444	1515	1699	3444	1449	3298	5043	1487	3267	5043	1448
Flt Permitted	0.95	1.00	1.00	0.49	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3855	3444	1515	881	3444	1449	3298	5043	1487	3267	5043	1448
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	512	410	228	246	493	430	329	910	202	298	974	295
RTOR Reduction (vph)	0	0	165	0	0	177	0	0	0	0	0	196
Lane Group Flow (vph)	512	410	63	246	493	253	329	910	202	298	974	99
Confl. Peds. (#/hr)	23		1	1		23	15		3	3		15
Heavy Vehicles (%)	4%	6%	4%	5%	6%	6%	5%	4%	6%	6%	4%	5%
Turn Type	Prot		Perm	pm+pt		Perm	Prot		Free	Prot		Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4	8		8			Free			6
Actuated Green, G (s)	18.8	38.5	38.5	48.1	32.9	32.9	17.4	47.3	140.0	17.0	46.9	46.9
Effective Green, g (s)	18.8	38.5	38.5	48.1	32.9	32.9	17.4	47.3	140.0	17.0	46.9	46.9
Actuated g/C Ratio	0.13	0.28	0.28	0.34	0.23	0.23	0.12	0.34	1.00	0.12	0.33	0.33
Clearance Time (s)	5.0	7.0	7.0	3.0	7.0	7.0	5.0	7.0		5.0	7.0	7.0
Vehicle Extension (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.0		5.0	5.0	5.0
Lane Grp Cap (vph)	518	947	417	391	809	341	410	1704	1487	397	1689	485
v/s Ratio Prot	c0.13	0.12		0.07	0.14		c0.10	0.18		0.09	c0.19	
v/s Ratio Perm			0.04	0.15		c0.17			c0.14			0.07
v/c Ratio	0.99	0.43	0.15	0.63	0.61	0.74	0.80	0.53	0.14	0.75	0.58	0.20
Uniform Delay, d1	60.5	41.8	38.4	35.3	47.8	49.6	59.6	37.4	0.0	59.5	38.4	33.2
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	36.4	0.7	0.4	4.4	1.9	10.1	10.8	1.2	0.2	9.2	1.4	0.9
Delay (s)	96.9	42.4	38.7	39.7	49.7	59.7	70.4	38.7	0.2	68.7	39.8	34.2
Level of Service	F	D	D	D	D	E	E	D	A	E	D	C
Approach Delay (s)		65.9			51.3			40.5			44.2	
Approach LOS		E			D			D			D	

Intersection Summary

HCM Average Control Delay	49.5	HCM Level of Service	D
HCM Volume to Capacity ratio	0.73		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	24.0
Intersection Capacity Utilization	95.2%	ICU Level of Service	F
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Argentia Road & Winston Churchill Boulevard

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖↗	↑↑	↖	↖	↑↑	↖	↖↗	↑↑↑	↖	↖↗	↑↑↑	↖
Volume (vph)	411	342	235	130	110	311	206	984	325	544	617	166
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	5.0	7.0	7.0	3.0	7.0	7.0	5.0	7.0	4.0	5.0	7.0	7.0
Lane Util. Factor	0.97	0.95	1.00	1.00	0.95	1.00	0.97	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.97	1.00	1.00	0.97	1.00	1.00	0.99	1.00	1.00	0.97
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3298	3544	1415	1708	3318	1456	3298	5092	1560	3330	4995	1399
Flt Permitted	0.95	1.00	1.00	0.55	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3298	3544	1415	982	3318	1456	3298	5092	1560	3330	4995	1399
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	411	342	235	130	110	311	206	984	325	544	617	166
RTOR Reduction (vph)	0	0	185	0	0	259	0	0	0	0	0	85
Lane Group Flow (vph)	411	342	50	130	110	52	206	984	325	544	617	81
Confl. Peds. (#/hr)	19		12	12		19	4		4	4		4
Heavy Vehicles (%)	5%	3%	10%	4%	10%	6%	5%	3%	1%	4%	5%	11%
Turn Type	Prot		Perm	pm+pt		Perm	Prot		Free	Prot		Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4	8		8			Free			6
Actuated Green, G (s)	18.0	29.8	29.8	23.8	16.8	16.8	13.2	51.7	140.0	29.5	68.0	68.0
Effective Green, g (s)	18.0	29.8	29.8	23.8	16.8	16.8	13.2	51.7	140.0	29.5	68.0	68.0
Actuated g/C Ratio	0.13	0.21	0.21	0.17	0.12	0.12	0.09	0.37	1.00	0.21	0.49	0.49
Clearance Time (s)	5.0	7.0	7.0	3.0	7.0	7.0	5.0	7.0		5.0	7.0	7.0
Vehicle Extension (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.0		5.0	5.0	5.0
Lane Grp Cap (vph)	424	754	301	203	398	175	311	1880	1560	702	2426	680
v/s Ratio Prot	c0.12	0.10		0.03	0.03		0.06	c0.19		c0.16	0.12	
v/s Ratio Perm			0.04	c0.08		0.04			0.21			0.06
v/c Ratio	0.97	0.45	0.17	0.64	0.28	0.30	0.66	0.52	0.21	0.77	0.25	0.12
Uniform Delay, d1	60.7	48.0	45.0	52.4	56.1	56.2	61.2	34.5	0.0	52.1	21.1	19.6
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	35.8	0.9	0.5	9.1	0.8	2.0	5.2	1.0	0.3	6.2	0.3	0.4
Delay (s)	96.6	48.9	45.5	61.5	56.9	58.2	66.5	35.6	0.3	58.4	21.4	20.0
Level of Service	F	D	D	E	E	E	E	D	A	E	C	C
Approach Delay (s)		67.9			58.7			32.2			36.4	
Approach LOS		E			E			C			D	

Intersection Summary

HCM Average Control Delay	44.9	HCM Level of Service	D
HCM Volume to Capacity ratio	0.65		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	20.0
Intersection Capacity Utilization	95.7%	ICU Level of Service	F
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Argentia Road & Winston Churchill Boulevard

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖↗	↑↑	↖	↖	↑↑	↖	↖↗	↑↑↑	↖	↖↗	↑↑↑	↖
Volume (vph)	560	448	249	269	539	470	360	995	221	326	1065	323
Ideal Flow (vphpl)	2200	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	5.0	7.0	7.0	3.0	7.0	7.0	5.0	7.0	4.0	5.0	7.0	7.0
Lane Util. Factor	0.97	0.95	1.00	1.00	0.95	1.00	0.97	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.96	1.00	1.00	0.99	1.00	1.00	0.95
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3855	3444	1515	1699	3444	1449	3298	5043	1487	3267	5043	1448
Flt Permitted	0.95	1.00	1.00	0.48	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3855	3444	1515	855	3444	1449	3298	5043	1487	3267	5043	1448
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	560	448	249	269	539	470	360	995	221	326	1065	323
RTOR Reduction (vph)	0	0	178	0	0	192	0	0	0	0	0	226
Lane Group Flow (vph)	560	448	71	269	539	278	360	995	221	326	1065	97
Confl. Peds. (#/hr)	23		1	1		23	15		3	3		15
Heavy Vehicles (%)	4%	6%	4%	5%	6%	6%	5%	4%	6%	6%	4%	5%
Turn Type	Prot		Perm	pm+pt		Perm	Prot		Free	Prot		Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4	8		8			Free			6
Actuated Green, G (s)	23.6	39.8	39.8	51.2	32.7	32.7	17.8	42.3	140.0	17.4	41.9	41.9
Effective Green, g (s)	23.6	39.8	39.8	51.2	32.7	32.7	17.8	42.3	140.0	17.4	41.9	41.9
Actuated g/C Ratio	0.17	0.28	0.28	0.37	0.23	0.23	0.13	0.30	1.00	0.12	0.30	0.30
Clearance Time (s)	5.0	7.0	7.0	3.0	7.0	7.0	5.0	7.0		5.0	7.0	7.0
Vehicle Extension (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.0		5.0	5.0	5.0
Lane Grp Cap (vph)	650	979	431	424	804	338	419	1524	1487	406	1509	433
v/s Ratio Prot	c0.15	0.13		0.08	0.16		c0.11	0.20		0.10	c0.21	
v/s Ratio Perm			0.05	0.15		c0.19			c0.15			0.07
v/c Ratio	0.86	0.46	0.16	0.63	0.67	0.82	0.86	0.65	0.15	0.80	0.71	0.22
Uniform Delay, d1	56.6	41.2	37.6	33.5	48.8	50.9	59.9	42.5	0.0	59.6	43.6	36.8
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	12.3	0.7	0.4	4.3	2.9	16.4	15.9	2.2	0.2	12.4	2.8	1.2
Delay (s)	68.9	41.9	38.0	37.7	51.6	67.2	75.8	44.7	0.2	72.0	46.4	38.0
Level of Service	E	D	D	D	D	E	E	D	A	E	D	D
Approach Delay (s)		53.2			54.4			45.5			49.7	
Approach LOS		D			D			D			D	

Intersection Summary

HCM Average Control Delay	50.4	HCM Level of Service	D
HCM Volume to Capacity ratio	0.79		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	24.0
Intersection Capacity Utilization	97.9%	ICU Level of Service	F
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Argentia Road & Winston Churchill Boulevard

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	454	378	260	144	122	343	227	1087	359	600	681	184
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	5.0	7.0	7.0	3.0	7.0	7.0	5.0	7.0	4.0	5.0	7.0	7.0
Lane Util. Factor	0.97	0.95	1.00	1.00	0.95	1.00	0.97	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.97	1.00	1.00	0.97	1.00	1.00	0.99	1.00	1.00	0.97
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3298	3544	1415	1709	3318	1456	3298	5092	1560	3330	4995	1399
Flt Permitted	0.95	1.00	1.00	0.53	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3298	3544	1415	949	3318	1456	3298	5092	1560	3330	4995	1399
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	454	378	260	144	122	343	227	1087	359	600	681	184
RTOR Reduction (vph)	0	0	206	0	0	254	0	0	0	0	0	98
Lane Group Flow (vph)	454	378	54	144	122	89	227	1087	359	600	681	86
Confl. Peds. (#/hr)	19		12	12		19	4		4	4		4
Heavy Vehicles (%)	5%	3%	10%	4%	10%	6%	5%	3%	1%	4%	5%	11%
Turn Type	Prot		Perm	pm+pt		Perm	Prot		Free	Prot		Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4	8		8			Free			6
Actuated Green, G (s)	18.0	29.3	29.3	27.3	18.3	18.3	14.1	45.6	140.0	34.1	65.6	65.6
Effective Green, g (s)	18.0	29.3	29.3	27.3	18.3	18.3	14.1	45.6	140.0	34.1	65.6	65.6
Actuated g/C Ratio	0.13	0.21	0.21	0.20	0.13	0.13	0.10	0.33	1.00	0.24	0.47	0.47
Clearance Time (s)	5.0	7.0	7.0	3.0	7.0	7.0	5.0	7.0		5.0	7.0	7.0
Vehicle Extension (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.0		5.0	5.0	5.0
Lane Grp Cap (vph)	424	742	296	234	434	190	332	1659	1560	811	2341	656
v/s Ratio Prot	c0.14	0.11		0.04	0.04		0.07	c0.21		c0.18	0.14	
v/s Ratio Perm			0.04	c0.08		0.06			0.23			0.06
v/c Ratio	1.07	0.51	0.18	0.62	0.28	0.47	0.68	0.66	0.23	0.74	0.29	0.13
Uniform Delay, d1	61.0	49.0	45.5	49.6	54.9	56.4	60.8	40.5	0.0	48.9	22.9	21.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	63.9	1.2	0.6	6.8	0.7	3.8	5.7	2.0	0.3	4.3	0.3	0.4
Delay (s)	124.9	50.1	46.1	56.4	55.7	60.1	66.5	42.5	0.3	53.2	23.2	21.5
Level of Service	F	D	D	E	E	E	E	D	A	D	C	C
Approach Delay (s)		80.3			58.4			36.7			35.3	
Approach LOS		F			E			D			D	

Intersection Summary

HCM Average Control Delay	48.8	HCM Level of Service	D
HCM Volume to Capacity ratio	0.71		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	20.0
Intersection Capacity Utilization	98.5%	ICU Level of Service	F
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Argentia Road & Winston Churchill Boulevard

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	619	495	275	297	596	519	397	1099	244	360	1177	356
Ideal Flow (vphpl)	2200	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	5.0	7.0	7.0	3.0	7.0	7.0	5.0	7.0	4.0	5.0	7.0	7.0
Lane Util. Factor	0.97	0.95	1.00	1.00	0.95	1.00	0.97	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.96	1.00	1.00	0.99	1.00	1.00	0.95
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3855	3444	1515	1700	3444	1449	3298	5043	1487	3267	5043	1448
Flt Permitted	0.95	1.00	1.00	0.42	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3855	3444	1515	747	3444	1449	3298	5043	1487	3267	5043	1448
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	619	495	275	297	596	519	397	1099	244	360	1177	356
RTOR Reduction (vph)	0	0	196	0	0	196	0	0	0	0	0	235
Lane Group Flow (vph)	619	495	79	297	596	323	397	1099	244	360	1177	121
Confl. Peds. (#/hr)	23		1	1		23	15		3	3		15
Heavy Vehicles (%)	4%	6%	4%	5%	6%	6%	5%	4%	6%	6%	4%	5%
Turn Type	Prot		Perm	pm+pt		Perm	Prot		Free	Prot		Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4	8		8			Free			6
Actuated Green, G (s)	24.0	40.3	40.3	54.3	34.3	34.3	18.1	39.7	140.0	18.0	39.6	39.6
Effective Green, g (s)	24.0	40.3	40.3	54.3	34.3	34.3	18.1	39.7	140.0	18.0	39.6	39.6
Actuated g/C Ratio	0.17	0.29	0.29	0.39	0.24	0.24	0.13	0.28	1.00	0.13	0.28	0.28
Clearance Time (s)	5.0	7.0	7.0	3.0	7.0	7.0	5.0	7.0		5.0	7.0	7.0
Vehicle Extension (s)	5.0	5.0	5.0	5.0	5.0	5.0	3.0	5.0		5.0	5.0	5.0
Lane Grp Cap (vph)	661	991	436	426	844	355	426	1430	1487	420	1426	410
v/s Ratio Prot	c0.16	0.14		0.10	0.17		c0.12	0.22		0.11	c0.23	
v/s Ratio Perm			0.05	0.17		c0.22			c0.16			0.08
v/c Ratio	0.94	0.50	0.18	0.70	0.71	0.91	0.93	0.77	0.16	0.86	0.83	0.30
Uniform Delay, d1	57.2	41.5	37.5	31.9	48.2	51.4	60.3	45.9	0.0	59.7	47.0	39.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	21.4	0.8	0.4	6.2	3.4	27.9	27.2	4.0	0.2	17.1	5.6	1.8
Delay (s)	78.6	42.3	37.9	38.1	51.6	79.2	87.5	50.0	0.2	76.8	52.5	41.1
Level of Service	E	D	D	D	D	E	F	D	A	E	D	D
Approach Delay (s)		57.6			58.9			51.6			55.0	
Approach LOS		E			E			D			E	

Intersection Summary






































HCM Average Control Delay	55.5	HCM Level of Service	E
HCM Volume to Capacity ratio	0.89		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	24.0
Intersection Capacity Utilization	101.1%	ICU Level of Service	G
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Mississauga Road

1/26/2014

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		  		  	  		 	  		  	  	
Volume (vph)	565	1469	130	341	502	58	431	1507	541	251	1167	253
Ideal Flow (vphpl)	2300	2000	1900	1900	1900	1900	1900	2000	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	3.0	8.6	4.0	5.0	8.6	4.0	5.0	7.8	4.0	5.0	7.8	4.0
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00	0.99	1.00	1.00	0.98	1.00	1.00	0.99
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	2117	5412	1506	3038	4902	1303	3395	5308	1512	3298	4995	1517
Flt Permitted	0.41	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	904	5412	1506	3038	4902	1303	3395	5308	1512	3298	4995	1517
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	565	1469	130	341	502	58	431	1507	541	251	1167	253
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	565	1469	130	341	502	58	431	1507	541	251	1167	253
Confl. Peds. (#/hr)	2		25	25		2	1		12	12		1
Heavy Vehicles (%)	2%	2%	4%	14%	7%	21%	2%	4%	4%	5%	5%	4%
Turn Type	pm+pt		Free	Prot		Free	Prot		Free	Prot		Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free			Free			Free			Free
Actuated Green, G (s)	59.8	39.0	140.0	17.4	37.6	140.0	18.8	39.8	140.0	17.4	38.4	140.0
Effective Green, g (s)	59.8	39.0	140.0	17.4	37.6	140.0	18.8	39.8	140.0	17.4	38.4	140.0
Actuated g/C Ratio	0.43	0.28	1.00	0.12	0.27	1.00	0.13	0.28	1.00	0.12	0.27	1.00
Clearance Time (s)	3.0	8.6		5.0	8.6		5.0	7.8		5.0	7.8	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		3.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	566	1508	1506	378	1317	1303	456	1509	1512	410	1370	1517
v/s Ratio Prot	c0.15	0.27		0.11	0.10		c0.13	c0.28		0.08	0.23	
v/s Ratio Perm	c0.28		0.09			0.04			c0.36			0.17
v/c Ratio	1.00	0.97	0.09	0.90	0.38	0.04	0.95	1.00	0.36	0.61	0.85	0.17
Uniform Delay, d1	34.8	50.0	0.0	60.5	41.7	0.0	60.1	50.1	0.0	58.1	48.1	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	37.2	17.4	0.1	25.1	0.4	0.1	28.5	22.9	0.7	3.9	6.8	0.2
Delay (s)	72.0	67.4	0.1	85.6	42.1	0.1	88.6	72.9	0.7	62.0	55.0	0.2
Level of Service	E	E	A	F	D	A	F	E	A	E	D	A
Approach Delay (s)		64.6			55.9			59.9			47.7	
Approach LOS		E			E			E			D	
Intersection Summary												
HCM Average Control Delay			58.0				HCM Level of Service			E		
HCM Volume to Capacity ratio			0.91									
Actuated Cycle Length (s)			140.0				Sum of lost time (s)		11.0			
Intersection Capacity Utilization			96.6%				ICU Level of Service		F			
Analysis Period (min)			15									

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Mississauga Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘↗	↑↑↑	↗	↘↗	↑↑↑	↗
Volume (vph)	493	759	234	520	1275	142	276	1774	277	144	1348	235
Ideal Flow (vphpl)	2300	1900	1900	1900	1900	1900	1900	2200	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	3.0	8.6	4.0	5.0	8.6	4.0	5.0	7.8	4.0	5.0	7.8	4.0
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	2139	5142	1562	3298	5193	1421	3362	5896	1452	3267	5092	1536
Flt Permitted	0.12	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	274	5142	1562	3298	5193	1421	3362	5896	1452	3267	5092	1536
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	493	759	234	520	1275	142	276	1774	277	144	1348	235
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	493	759	234	520	1275	142	276	1774	277	144	1348	235
Confl. Peds. (#/hr)	1		1	1		1						
Heavy Vehicles (%)	1%	2%	1%	5%	1%	11%	3%	3%	10%	6%	3%	4%
Turn Type	pm+pt		Free	Prot		Free	Prot		Free	Prot		Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free			Free			Free			Free
Actuated Green, G (s)	50.9	32.9	140.0	24.3	41.2	140.0	16.2	42.6	140.0	13.8	40.2	140.0
Effective Green, g (s)	50.9	32.9	140.0	24.3	41.2	140.0	16.2	42.6	140.0	13.8	40.2	140.0
Actuated g/C Ratio	0.36	0.23	1.00	0.17	0.29	1.00	0.12	0.30	1.00	0.10	0.29	1.00
Clearance Time (s)	3.0	8.6		5.0	8.6		5.0	7.8		5.0	7.8	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		3.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	339	1208	1562	572	1528	1421	389	1794	1452	322	1462	1536
v/s Ratio Prot	c0.19	0.15		c0.16	0.25		c0.08	c0.30		0.04	0.26	
v/s Ratio Perm	c0.34		0.15			0.10			c0.19			0.15
v/c Ratio	1.45	0.63	0.15	0.91	0.83	0.10	0.71	0.99	0.19	0.45	0.92	0.15
Uniform Delay, d1	40.9	48.1	0.0	56.8	46.2	0.0	59.6	48.5	0.0	59.5	48.4	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	220.2	1.5	0.2	19.3	4.6	0.1	5.8	18.8	0.3	2.1	11.1	0.2
Delay (s)	261.1	49.5	0.2	76.0	50.8	0.1	65.5	67.2	0.3	61.6	59.4	0.2
Level of Service	F	D	A	E	D	A	E	E	A	E	E	A
Approach Delay (s)		112.0			53.8			59.1			51.6	
Approach LOS		F			D			E			D	

Intersection Summary

HCM Average Control Delay	66.5	HCM Level of Service	E
HCM Volume to Capacity ratio	1.03		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	11.0
Intersection Capacity Utilization	108.0%	ICU Level of Service	G
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Mississauga Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘↗	↑↑↑	↗	↘↗	↑↑↑	↗
Volume (vph)	624	1623	144	377	555	64	476	1665	598	277	1289	279
Ideal Flow (vphpl)	2300	2000	1900	1900	1900	1900	1900	2000	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	3.0	8.6	4.0	5.0	8.6	4.0	5.0	7.8	4.0	5.0	7.8	4.0
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00	0.99	1.00	1.00	0.98	1.00	1.00	0.99
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	2118	5412	1506	3038	4902	1303	3395	5308	1512	3298	4995	1517
Flt Permitted	0.34	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	750	5412	1506	3038	4902	1303	3395	5308	1512	3298	4995	1517
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	624	1623	144	377	555	64	476	1665	598	277	1289	279
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	624	1623	144	377	555	64	476	1665	598	277	1289	279
Confl. Peds. (#/hr)	2		25	25		2	1		12	12		1
Heavy Vehicles (%)	2%	2%	4%	14%	7%	21%	2%	4%	4%	5%	5%	4%
Turn Type	pm+pt		Free	Prot		Free	Prot		Free	Prot		Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free			Free			Free			Free
Actuated Green, G (s)	62.9	40.9	140.0	17.0	32.9	140.0	19.0	43.7	140.0	12.0	36.7	140.0
Effective Green, g (s)	62.9	40.9	140.0	17.0	32.9	140.0	19.0	43.7	140.0	12.0	36.7	140.0
Actuated g/C Ratio	0.45	0.29	1.00	0.12	0.23	1.00	0.14	0.31	1.00	0.09	0.26	1.00
Clearance Time (s)	3.0	8.6		5.0	8.6		5.0	7.8		5.0	7.8	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		3.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	601	1581	1506	369	1152	1303	461	1657	1512	283	1309	1517
v/s Ratio Prot	c0.20	0.30		0.12	0.11		c0.14	c0.31		0.08	0.26	
v/s Ratio Perm	c0.27		0.10			0.05			0.40			0.18
v/c Ratio	1.04	1.03	0.10	1.02	0.48	0.05	1.03	1.00	0.40	0.98	0.98	0.18
Uniform Delay, d1	32.0	49.5	0.0	61.5	46.2	0.0	60.5	48.1	0.0	63.9	51.4	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	47.0	29.7	0.1	52.5	0.7	0.1	50.5	23.3	0.8	47.6	21.5	0.3
Delay (s)	79.0	79.2	0.1	114.0	46.9	0.1	111.0	71.4	0.8	111.4	72.9	0.3
Level of Service	E	E	A	F	D	A	F	E	A	F	E	A
Approach Delay (s)		74.4			69.3			62.9			67.7	
Approach LOS		E			E			E			E	

Intersection Summary

HCM Average Control Delay	68.2	HCM Level of Service	E
HCM Volume to Capacity ratio	0.94		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	103.1%	ICU Level of Service	G
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Mississauga Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘↗	↑↑↑	↗	↘↗	↑↑↑	↗
Volume (vph)	545	838	258	574	1408	157	305	1960	306	159	1489	260
Ideal Flow (vphpl)	2300	1900	1900	1900	1900	1900	1900	2200	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	3.0	8.6	4.0	5.0	8.6	4.0	5.0	7.8	4.0	5.0	7.8	4.0
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	2139	5142	1562	3298	5193	1421	3362	5896	1452	3267	5092	1536
Flt Permitted	0.13	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	287	5142	1562	3298	5193	1421	3362	5896	1452	3267	5092	1536
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	545	838	258	574	1408	157	305	1960	306	159	1489	260
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	545	838	258	574	1408	157	305	1960	306	159	1489	260
Confl. Peds. (#/hr)	1		1	1		1						
Heavy Vehicles (%)	1%	2%	1%	5%	1%	11%	3%	3%	10%	6%	3%	4%
Turn Type	pm+pt		Free	Prot		Free	Prot		Free	Prot		Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free			Free			Free			Free
Actuated Green, G (s)	55.4	31.4	140.0	25.0	34.4	140.0	13.0	45.2	140.0	12.0	44.2	140.0
Effective Green, g (s)	55.4	31.4	140.0	25.0	34.4	140.0	13.0	45.2	140.0	12.0	44.2	140.0
Actuated g/C Ratio	0.40	0.22	1.00	0.18	0.25	1.00	0.09	0.32	1.00	0.09	0.32	1.00
Clearance Time (s)	3.0	8.6		5.0	8.6		5.0	7.8		5.0	7.8	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		3.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	431	1153	1562	589	1276	1421	312	1904	1452	280	1608	1536
v/s Ratio Prot	c0.22	0.16		0.17	0.27		c0.09	c0.33		0.05	0.29	
v/s Ratio Perm	c0.28		0.17			0.11			c0.21			0.17
v/c Ratio	1.26	0.73	0.17	0.97	1.10	0.11	0.98	1.03	0.21	0.57	0.93	0.17
Uniform Delay, d1	43.3	50.3	0.0	57.2	52.8	0.0	63.4	47.4	0.0	61.5	46.3	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	136.4	2.8	0.2	30.8	58.5	0.2	44.4	28.6	0.3	4.3	10.6	0.2
Delay (s)	179.7	53.2	0.2	88.0	111.3	0.2	107.7	76.0	0.3	65.8	56.9	0.2
Level of Service	F	D	A	F	F	A	F	E	A	E	E	A
Approach Delay (s)		86.9			96.9			70.7			50.0	
Approach LOS		F			F			E			D	

Intersection Summary

HCM Average Control Delay	75.9	HCM Level of Service	E
HCM Volume to Capacity ratio	1.01		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	11.0
Intersection Capacity Utilization	116.0%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Mississauga Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘↗	↑↑↑	↗	↘↗	↑↑↑	↗
Volume (vph)	689	1792	159	416	613	71	526	1839	660	306	1424	309
Ideal Flow (vphpl)	2300	2000	1900	1900	1900	1900	1900	2000	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	3.0	8.6	4.0	5.0	8.6	4.0	5.0	7.8	4.0	5.0	7.8	4.0
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00	0.99	1.00	1.00	0.98	1.00	1.00	0.99
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	2118	5412	1506	3038	4902	1303	3395	5308	1512	3298	4995	1517
Flt Permitted	0.30	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	671	5412	1506	3038	4902	1303	3395	5308	1512	3298	4995	1517
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	689	1792	159	416	613	71	526	1839	660	306	1424	309
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	689	1792	159	416	613	71	526	1839	660	306	1424	309
Confl. Peds. (#/hr)	2		25	25		2	1		12	12		1
Heavy Vehicles (%)	2%	2%	4%	14%	7%	21%	2%	4%	4%	5%	5%	4%
Turn Type	pm+pt		Free	Prot		Free	Prot		Free	Prot		Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free			Free			Free			Free
Actuated Green, G (s)	62.4	41.4	140.0	16.0	33.4	140.0	18.0	44.2	140.0	12.0	38.2	140.0
Effective Green, g (s)	62.4	41.4	140.0	16.0	33.4	140.0	18.0	44.2	140.0	12.0	38.2	140.0
Actuated g/C Ratio	0.45	0.30	1.00	0.11	0.24	1.00	0.13	0.32	1.00	0.09	0.27	1.00
Clearance Time (s)	3.0	8.6		5.0	8.6		5.0	7.8		5.0	7.8	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		3.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	568	1600	1506	347	1169	1303	437	1676	1512	283	1363	1517
v/s Ratio Prot	c0.23	0.33		0.14	0.13		c0.15	c0.35		0.09	0.29	
v/s Ratio Perm	c0.32		0.11			0.05			0.44			0.20
v/c Ratio	1.21	1.12	0.11	1.20	0.52	0.05	1.20	1.10	0.44	1.08	1.04	0.20
Uniform Delay, d1	31.7	49.3	0.0	62.0	46.4	0.0	61.0	47.9	0.0	64.0	50.9	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	111.4	63.0	0.1	114.0	0.8	0.1	111.6	53.6	0.9	76.8	36.9	0.3
Delay (s)	143.1	112.3	0.1	176.0	47.2	0.1	172.6	101.5	0.9	140.8	87.8	0.3
Level of Service	F	F	A	F	D	A	F	F	A	F	F	A
Approach Delay (s)		113.6			92.9			92.0			82.5	
Approach LOS		F			F			F			F	

Intersection Summary

HCM Average Control Delay	96.4	HCM Level of Service	F
HCM Volume to Capacity ratio	1.07		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	110.5%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Mississauga Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘↗	↑↑↑	↗	↘↗	↑↑↑	↗
Volume (vph)	602	926	286	634	1556	173	337	2165	338	176	1645	287
Ideal Flow (vphpl)	2300	1900	1900	1900	1900	1900	1900	2200	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	3.0	8.6	4.0	5.0	8.6	4.0	5.0	7.8	4.0	5.0	7.8	4.0
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	2139	5142	1562	3298	5193	1421	3362	5896	1452	3267	5092	1536
Flt Permitted	0.12	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	270	5142	1562	3298	5193	1421	3362	5896	1452	3267	5092	1536
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	602	926	286	634	1556	173	337	2165	338	176	1645	287
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	602	926	286	634	1556	173	337	2165	338	176	1645	287
Confl. Peds. (#/hr)	1		1	1		1						
Heavy Vehicles (%)	1%	2%	1%	5%	1%	11%	3%	3%	10%	6%	3%	4%
Turn Type	pm+pt		Free	Prot		Free	Prot		Free	Prot		Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free			Free			Free			Free
Actuated Green, G (s)	55.4	33.4	140.0	23.0	36.4	140.0	13.0	44.4	140.0	12.8	44.2	140.0
Effective Green, g (s)	55.4	33.4	140.0	23.0	36.4	140.0	13.0	44.4	140.0	12.8	44.2	140.0
Actuated g/C Ratio	0.40	0.24	1.00	0.16	0.26	1.00	0.09	0.32	1.00	0.09	0.32	1.00
Clearance Time (s)	3.0	8.6		5.0	8.6		5.0	7.8		5.0	7.8	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		3.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	401	1227	1562	542	1350	1421	312	1870	1452	299	1608	1536
v/s Ratio Prot	c0.24	0.18		0.19	0.30		c0.10	c0.37		0.05	0.32	
v/s Ratio Perm	c0.36		0.18			0.12			c0.23			0.19
v/c Ratio	1.50	0.75	0.18	1.17	1.15	0.12	1.08	1.16	0.23	0.59	1.02	0.19
Uniform Delay, d1	43.4	49.5	0.0	58.5	51.8	0.0	63.5	47.8	0.0	61.1	47.9	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	238.3	3.2	0.3	94.8	77.6	0.2	74.0	77.5	0.4	4.5	28.5	0.3
Delay (s)	281.6	52.7	0.3	153.3	129.4	0.2	137.5	125.3	0.4	65.6	76.4	0.3
Level of Service	F	D	A	F	F	A	F	F	A	E	E	A
Approach Delay (s)		120.4			126.3			111.8			65.1	
Approach LOS		F			F			F			E	

Intersection Summary

HCM Average Control Delay	106.5	HCM Level of Service	F
HCM Volume to Capacity ratio	1.16		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	11.0
Intersection Capacity Utilization	124.9%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Winston Churchill Boulevard

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖↗	↑↑↑	↖	↖	↑↑↑	↖	↖	↑↑↑	↖	↖↗	↑↑↑	↖
Volume (vph)	223	1610	345	170	275	144	146	734	308	154	670	75
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	5.0	6.9	4.0	3.0	6.9	4.0	6.9	6.9	4.0	5.0	6.9	4.0
Lane Util. Factor	0.97	0.91	1.00	1.00	0.91	1.00	1.00	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	0.99
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3429	5142	1530	1653	4768	1458	1645	5092	1559	3298	4902	1558
Flt Permitted	0.95	1.00	1.00	0.08	1.00	1.00	0.39	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3429	5142	1530	148	4768	1458	668	5092	1559	3298	4902	1558
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	223	1610	345	170	275	144	146	734	308	154	670	75
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	223	1610	345	170	275	144	146	734	308	154	670	75
Confl. Peds. (#/hr)	6		5	5		6	8		7	7		8
Confl. Bikes (#/hr)												2
Heavy Vehicles (%)	1%	2%	3%	8%	10%	8%	8%	3%	1%	5%	7%	1%
Turn Type	Prot		Free	pm+pt		Free	Perm		Free	Prot		Free
Protected Phases	7	4		3	8			2		1	6	
Permitted Phases			Free	8		Free	2		Free			Free
Actuated Green, G (s)	16.0	54.1	140.0	58.1	47.1	140.0	39.3	39.3	140.0	13.8	58.1	140.0
Effective Green, g (s)	16.0	54.1	140.0	58.1	47.1	140.0	39.3	39.3	140.0	13.8	58.1	140.0
Actuated g/C Ratio	0.11	0.39	1.00	0.42	0.34	1.00	0.28	0.28	1.00	0.10	0.42	1.00
Clearance Time (s)	5.0	6.9		3.0	6.9		6.9	6.9		5.0	6.9	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	392	1987	1530	180	1604	1458	188	1429	1559	325	2034	1558
v/s Ratio Prot	0.07	c0.31		c0.07	0.06			0.14		c0.05	0.14	
v/s Ratio Perm			0.23	c0.32		0.10	c0.22		0.20			0.05
v/c Ratio	0.57	0.81	0.23	0.94	0.17	0.10	0.78	0.51	0.20	0.47	0.33	0.05
Uniform Delay, d1	58.7	38.4	0.0	35.9	32.7	0.0	46.3	42.3	0.0	59.7	27.7	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	3.1	3.0	0.3	51.9	0.1	0.1	26.4	1.3	0.3	2.3	0.4	0.1
Delay (s)	61.8	41.3	0.3	87.8	32.8	0.1	72.7	43.6	0.3	61.9	28.2	0.1
Level of Service	E	D	A	F	C	A	E	D	A	E	C	A
Approach Delay (s)		36.9			40.7			36.0			31.6	
Approach LOS		D			D			D			C	

Intersection Summary

HCM Average Control Delay	36.2	HCM Level of Service	D
HCM Volume to Capacity ratio	0.83		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	24.8
Intersection Capacity Utilization	92.5%	ICU Level of Service	F
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Mississauga Road

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↑↑↑	↗	↘↗	↑↑↑	↗	↘↗	↑↑↑	↗	↘↗	↑↑↑	↗
Volume (vph)	493	759	234	520	1275	142	276	1774	277	144	1348	235
Ideal Flow (vphpl)	2300	1900	1900	1900	1900	1900	1900	2200	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	3.0	8.6	4.0	5.0	8.6	4.0	5.0	7.8	4.0	5.0	7.8	4.0
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	2139	5142	1562	3298	5193	1421	3362	5896	1452	3267	5092	1536
Flt Permitted	0.12	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	274	5142	1562	3298	5193	1421	3362	5896	1452	3267	5092	1536
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	493	759	234	520	1275	142	276	1774	277	144	1348	235
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	493	759	234	520	1275	142	276	1774	277	144	1348	235
Confl. Peds. (#/hr)	1		1	1		1						
Heavy Vehicles (%)	1%	2%	1%	5%	1%	11%	3%	3%	10%	6%	3%	4%
Turn Type	pm+pt		Free	Prot		Free	Prot		Free	Prot		Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free			Free			Free			Free
Actuated Green, G (s)	50.9	32.9	140.0	24.3	41.2	140.0	16.2	42.6	140.0	13.8	40.2	140.0
Effective Green, g (s)	50.9	32.9	140.0	24.3	41.2	140.0	16.2	42.6	140.0	13.8	40.2	140.0
Actuated g/C Ratio	0.36	0.23	1.00	0.17	0.29	1.00	0.12	0.30	1.00	0.10	0.29	1.00
Clearance Time (s)	3.0	8.6		5.0	8.6		5.0	7.8		5.0	7.8	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		3.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	339	1208	1562	572	1528	1421	389	1794	1452	322	1462	1536
v/s Ratio Prot	c0.19	0.15		c0.16	0.25		c0.08	c0.30		0.04	0.26	
v/s Ratio Perm	c0.34		0.15			0.10			c0.19			0.15
v/c Ratio	1.45	0.63	0.15	0.91	0.83	0.10	0.71	0.99	0.19	0.45	0.92	0.15
Uniform Delay, d1	40.9	48.1	0.0	56.8	46.2	0.0	59.6	48.5	0.0	59.5	48.4	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	220.2	1.5	0.2	19.3	4.6	0.1	5.8	18.8	0.3	2.1	11.1	0.2
Delay (s)	261.1	49.5	0.2	76.0	50.8	0.1	65.5	67.2	0.3	61.6	59.4	0.2
Level of Service	F	D	A	E	D	A	E	E	A	E	E	A
Approach Delay (s)		112.0			53.8			59.1			51.6	
Approach LOS		F			D			E			D	

Intersection Summary

HCM Average Control Delay	66.5	HCM Level of Service	E
HCM Volume to Capacity ratio	1.03		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	11.0
Intersection Capacity Utilization	108.0%	ICU Level of Service	G
Analysis Period (min)	15		

c Critical Lane Group

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Winston Churchill Boulevard

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖↗	↑↑↑	↖	↖	↑↑↑	↖	↖	↑↑↑	↖	↖↗	↑↑↑	↖
Volume (vph)	246	1778	381	188	304	159	161	811	340	170	740	83
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	5.0	6.9	4.0	3.0	6.9	4.0	6.9	6.9	4.0	5.0	6.9	4.0
Lane Util. Factor	0.97	0.91	1.00	1.00	0.91	1.00	1.00	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	0.99
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3429	5142	1530	1653	4768	1458	1646	5092	1559	3298	4902	1558
Flt Permitted	0.95	1.00	1.00	0.09	1.00	1.00	0.36	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3429	5142	1530	156	4768	1458	622	5092	1559	3298	4902	1558
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	246	1778	381	188	304	159	161	811	340	170	740	83
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	246	1778	381	188	304	159	161	811	340	170	740	83
Confl. Peds. (#/hr)	6		5	5		6	8		7	7		8
Confl. Bikes (#/hr)												2
Heavy Vehicles (%)	1%	2%	3%	8%	10%	8%	8%	3%	1%	5%	7%	1%
Turn Type	Prot		Free	pm+pt		Free	Perm		Free	Prot		Free
Protected Phases	7	4		3	8			2		1	6	
Permitted Phases			Free	8		Free	2		Free			Free
Actuated Green, G (s)	14.6	49.1	140.0	56.5	44.5	140.0	50.1	50.1	140.0	7.0	62.1	140.0
Effective Green, g (s)	14.6	49.1	140.0	56.5	44.5	140.0	50.1	50.1	140.0	7.0	62.1	140.0
Actuated g/C Ratio	0.10	0.35	1.00	0.40	0.32	1.00	0.36	0.36	1.00	0.05	0.44	1.00
Clearance Time (s)	5.0	6.9		3.0	6.9		6.9	6.9		5.0	6.9	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	358	1803	1530	191	1516	1458	223	1822	1559	165	2174	1558
v/s Ratio Prot	0.07	c0.35		c0.08	0.06			0.16		c0.05	0.15	
v/s Ratio Perm			0.25	0.31		0.11	c0.26		0.22			0.05
v/c Ratio	0.69	0.99	0.25	0.98	0.20	0.11	0.72	0.45	0.22	1.03	0.34	0.05
Uniform Delay, d1	60.5	45.1	0.0	39.6	34.8	0.0	38.9	34.3	0.0	66.5	25.5	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	6.9	17.9	0.4	60.5	0.1	0.2	18.3	0.8	0.3	78.3	0.4	0.1
Delay (s)	67.4	63.1	0.4	100.2	34.9	0.2	57.2	35.1	0.3	144.8	26.0	0.1
Level of Service	E	E	A	F	C	A	E	D	A	F	C	A
Approach Delay (s)		53.6			45.3			28.8			44.1	
Approach LOS		D			D			C			D	

Intersection Summary

HCM Average Control Delay	44.8	HCM Level of Service	D
HCM Volume to Capacity ratio	0.88		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	21.8
Intersection Capacity Utilization	97.6%	ICU Level of Service	F
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Winston Churchill Boulevard

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔↔	↑↑↑	↗	↖	↑↑↑	↗	↖	↑↑↑	↗	↖↖	↑↑↑	↗
Volume (vph)	146	535	246	433	1667	180	366	898	189	251	1275	360
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	2200	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	5.0	6.9	4.0	3.0	6.9	4.0	3.0	6.9	4.0	5.0	6.9	4.0
Lane Util. Factor	0.97	0.91	1.00	1.00	0.91	1.00	1.00	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	0.99
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3395	5193	1546	1785	5193	1559	2026	5092	1516	3463	5193	1559
Flt Permitted	0.95	1.00	1.00	0.33	1.00	1.00	0.09	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3395	5193	1546	612	5193	1559	193	5092	1516	3463	5193	1559
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	146	535	246	433	1667	180	366	898	189	251	1275	360
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	146	535	246	433	1667	180	366	898	189	251	1275	360
Confl. Peds. (#/hr)	6		1	1		6	5		3	3		5
Confl. Bikes (#/hr)			1									2
Heavy Vehicles (%)	2%	1%	2%	0%	1%	1%	2%	3%	4%	0%	1%	1%
Turn Type	Prot		Free	pm+pt		Free	pm+pt		Free	Prot		Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			Free	8		Free	2		Free			Free
Actuated Green, G (s)	7.0	32.1	140.0	60.1	48.1	140.0	66.1	45.7	140.0	15.4	41.3	140.0
Effective Green, g (s)	7.0	32.1	140.0	60.1	48.1	140.0	66.1	45.7	140.0	15.4	41.3	140.0
Actuated g/C Ratio	0.05	0.23	1.00	0.43	0.34	1.00	0.47	0.33	1.00	0.11	0.29	1.00
Clearance Time (s)	5.0	6.9		3.0	6.9		3.0	6.9		5.0	6.9	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		3.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	170	1191	1546	472	1784	1559	377	1662	1516	381	1532	1559
v/s Ratio Prot	0.04	0.10		c0.16	c0.32		c0.15	0.18		0.07	0.25	
v/s Ratio Perm			0.16	0.23		0.12	c0.31		0.12			0.23
v/c Ratio	0.86	0.45	0.16	0.92	0.93	0.12	0.97	0.54	0.12	0.66	0.83	0.23
Uniform Delay, d1	66.0	46.4	0.0	31.0	44.4	0.0	43.5	38.6	0.0	59.8	46.1	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	34.9	0.6	0.2	23.6	10.0	0.2	38.5	1.3	0.2	5.5	5.4	0.3
Delay (s)	100.9	46.9	0.2	54.6	54.4	0.2	82.0	39.8	0.2	65.2	51.6	0.3
Level of Service	F	D	A	D	D	A	F	D	A	E	D	A
Approach Delay (s)		43.0			50.2			45.3			43.6	
Approach LOS		D			D			D			D	

Intersection Summary

HCM Average Control Delay	46.2	HCM Level of Service	D
HCM Volume to Capacity ratio	0.89		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	6.0
Intersection Capacity Utilization	98.4%	ICU Level of Service	F
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Winston Churchill Boulevard

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔↔	↑↑↑	↔	↔	↑↑↑	↔	↔	↑↑↑	↔	↔↔	↑↑↑	↔
Volume (vph)	272	1965	421	207	336	176	178	896	376	188	818	92
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	5.0	6.9	4.0	3.0	6.9	4.0	6.9	6.9	4.0	5.0	6.9	4.0
Lane Util. Factor	0.97	0.91	1.00	1.00	0.91	1.00	1.00	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	0.99
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3429	5142	1530	1653	4768	1458	1646	5092	1559	3298	4902	1558
Flt Permitted	0.95	1.00	1.00	0.09	1.00	1.00	0.33	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3429	5142	1530	156	4768	1458	574	5092	1559	3298	4902	1558
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	272	1965	421	207	336	176	178	896	376	188	818	92
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	272	1965	421	207	336	176	178	896	376	188	818	92
Confl. Peds. (#/hr)	6		5	5		6	8		7	7		8
Confl. Bikes (#/hr)												2
Heavy Vehicles (%)	1%	2%	3%	8%	10%	8%	8%	3%	1%	5%	7%	1%
Turn Type	Prot		Free	pm+pt		Free	Perm		Free	Prot		Free
Protected Phases	7	4		3	8			2		1	6	
Permitted Phases			Free	8		Free	2		Free			Free
Actuated Green, G (s)	15.6	51.1	140.0	55.5	44.5	140.0	48.1	48.1	140.0	8.0	61.1	140.0
Effective Green, g (s)	15.6	51.1	140.0	55.5	44.5	140.0	48.1	48.1	140.0	8.0	61.1	140.0
Actuated g/C Ratio	0.11	0.37	1.00	0.40	0.32	1.00	0.34	0.34	1.00	0.06	0.44	1.00
Clearance Time (s)	5.0	6.9		3.0	6.9		6.9	6.9		5.0	6.9	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	382	1877	1530	179	1516	1458	197	1749	1559	188	2139	1558
v/s Ratio Prot	0.08	c0.38		c0.09	0.07			0.18		c0.06	0.17	
v/s Ratio Perm			0.28	c0.37		0.12	c0.31		0.24			0.06
v/c Ratio	0.71	1.05	0.28	1.16	0.22	0.12	0.90	0.51	0.24	1.00	0.38	0.06
Uniform Delay, d1	60.0	44.5	0.0	38.9	35.0	0.0	43.7	36.6	0.0	66.0	26.7	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	7.6	34.3	0.4	115.6	0.2	0.2	43.0	1.1	0.4	65.6	0.5	0.1
Delay (s)	67.6	78.7	0.4	154.6	35.2	0.2	86.7	37.7	0.4	131.6	27.2	0.1
Level of Service	E	E	A	F	D	A	F	D	A	F	C	A
Approach Delay (s)		65.2			61.0			34.0			42.8	
Approach LOS		E			E			C			D	

Intersection Summary

HCM Average Control Delay	52.9	HCM Level of Service	D
HCM Volume to Capacity ratio	1.04		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	24.8
Intersection Capacity Utilization	103.2%	ICU Level of Service	G
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

3: Derry Road & Winston Churchill Boulevard

1/26/2014



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖↗	↑↑↑	↖	↖	↑↑↑	↖	↖	↑↑↑	↖	↖↗	↑↑↑	↖
Volume (vph)	161	591	272	478	1841	199	404	992	209	277	1408	398
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	2200	1900	1900	1900	1900	1900
Lane Width	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5	3.5	3.7	3.5
Total Lost time (s)	5.0	6.9	4.0	3.0	6.9	4.0	3.0	6.9	4.0	5.0	6.9	4.0
Lane Util. Factor	0.97	0.91	1.00	1.00	0.91	1.00	1.00	0.91	1.00	0.97	0.91	1.00
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	0.99	1.00	1.00	0.99
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	3395	5193	1546	1785	5193	1559	2026	5092	1516	3463	5193	1559
Flt Permitted	0.95	1.00	1.00	0.29	1.00	1.00	0.09	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	3395	5193	1546	543	5193	1559	198	5092	1516	3463	5193	1559
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	161	591	272	478	1841	199	404	992	209	277	1408	398
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	161	591	272	478	1841	199	404	992	209	277	1408	398
Confl. Peds. (#/hr)	6		1	1		6	5		3	3		5
Confl. Bikes (#/hr)			1									2
Heavy Vehicles (%)	2%	1%	2%	0%	1%	1%	2%	3%	4%	0%	1%	1%
Turn Type	Prot		Free	pm+pt		Free	pm+pt		Free	Prot		Free
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			Free	8		Free	2		Free			Free
Actuated Green, G (s)	7.0	32.1	140.0	61.1	49.1	140.0	65.1	43.8	140.0	16.3	40.1	140.0
Effective Green, g (s)	7.0	32.1	140.0	61.1	49.1	140.0	65.1	43.8	140.0	16.3	40.1	140.0
Actuated g/C Ratio	0.05	0.23	1.00	0.44	0.35	1.00	0.46	0.31	1.00	0.12	0.29	1.00
Clearance Time (s)	5.0	6.9		3.0	6.9		3.0	6.9		5.0	6.9	
Vehicle Extension (s)	5.0	5.0		5.0	5.0		3.0	5.0		5.0	5.0	
Lane Grp Cap (vph)	170	1191	1546	468	1821	1559	379	1593	1516	403	1487	1559
v/s Ratio Prot	0.05	0.11		c0.19	0.35		c0.17	0.19		0.08	0.27	
v/s Ratio Perm			0.18	c0.26		0.13	c0.33		0.14			0.26
v/c Ratio	0.95	0.50	0.18	1.02	1.01	0.13	1.07	0.62	0.14	0.69	0.95	0.26
Uniform Delay, d1	66.3	46.9	0.0	32.4	45.5	0.0	44.6	41.0	0.0	59.4	48.9	0.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	54.3	0.7	0.2	47.1	23.8	0.2	64.8	1.8	0.2	6.2	13.7	0.4
Delay (s)	120.6	47.6	0.2	79.5	69.3	0.2	109.5	42.9	0.2	65.6	62.6	0.4
Level of Service	F	D	A	E	E	A	F	D	A	E	E	A
Approach Delay (s)		46.5			65.8			54.1			51.1	
Approach LOS		D			E			D			D	

Intersection Summary

HCM Average Control Delay	56.2	HCM Level of Service	E
HCM Volume to Capacity ratio	0.98		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	6.0
Intersection Capacity Utilization	106.1%	ICU Level of Service	G
Analysis Period (min)	15		
c Critical Lane Group			

Appendix F

Northbound Right-Turn Assessment

Memo – Northbound Right-Turn Lane

Project: Derry / Argentia EA
Subject: **Northbound Right-Turn Lane**
To: Sally Rook – Region of Peel
From: Tony Reitmeier – HDR
Andrew O’Connor – HDR
Date: July 28, 2014

The baseline recommended configuration for the intersection of Derry Road and Argentia Road includes a dedicated northbound right-turn lane on Argentia Road, in addition to 2 through lanes and a dedicated left-turn lane. This configuration is shown in **Figure 1**.

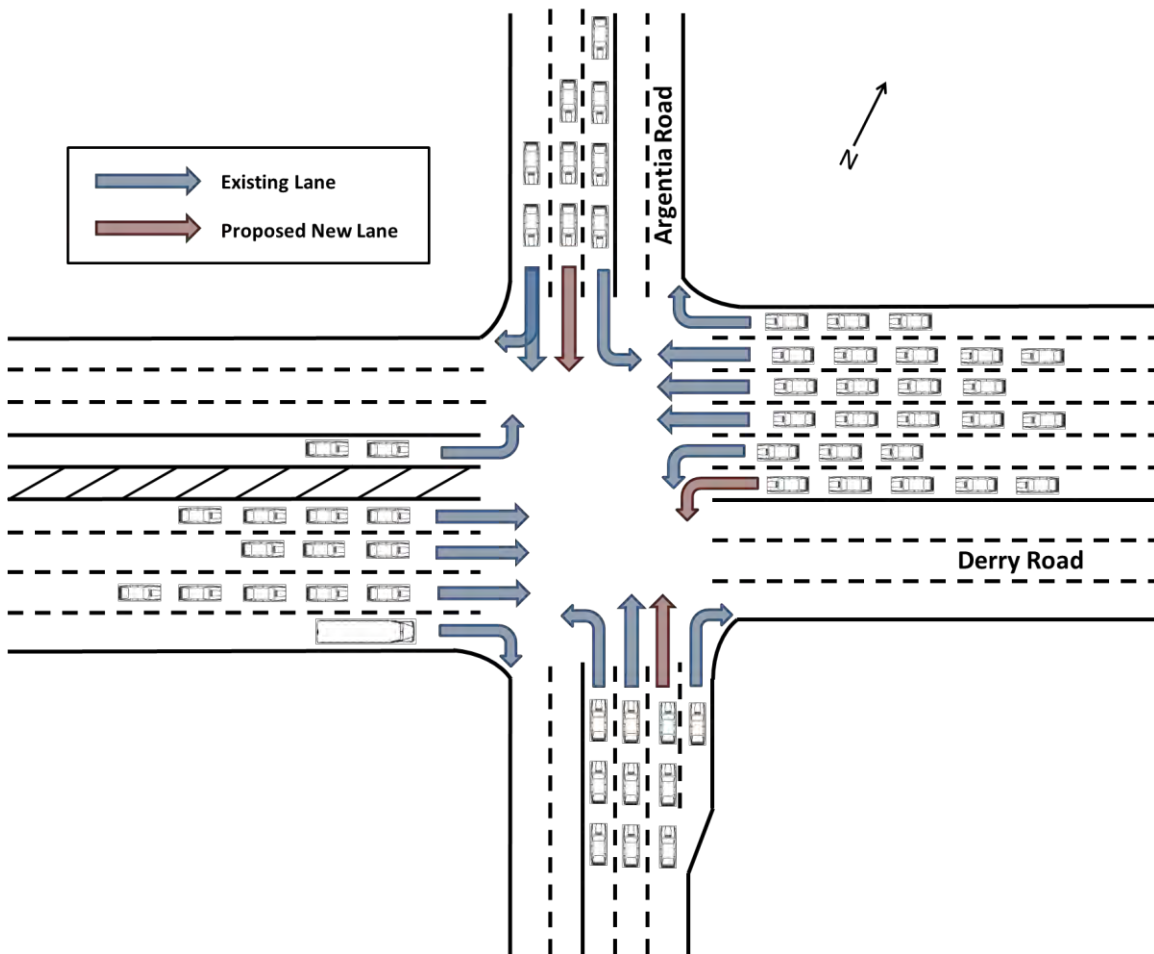


Figure 1: Preferred Configuration for Intersection of Derry Road and Argentia Road

The right-turn lane was sized based on the queuing results from Synchro. During the consultation process, however, it was noted that the northbound right-turn lane would result in impacts at the southeast quadrant of the intersection:

- Removal of mature trees required
- Impacts to utilities

- Potential impact to existing brick fence for Sheraton Four Points, which includes lighting and electrical connections

To mitigate these impacts, the team is considering modifications to the proposed configuration at this part of the intersection. The purpose of this memo is to evaluate two scenarios regarding the northbound right-turn lane proposed on Argentia Road:

- Scenario 1: Eliminate dedicated northbound right-turn lane
- Scenario 2: Provide shorter northbound right-turn lane

Intersection operations for these two scenarios are compared to the “Do Nothing” approach and the “Baseline Recommended” approach during the AM and PM peak period for the following horizon years:

- Existing conditions (see **Table 1**)
- 2021 conditions (see **Table 2**)
- 2031 conditions (see **Table 3**)

Queuing for each option is summarized in **Table 4**.

Findings

- Inclusion of a northbound right-turn lane **does result** in an improvement to intersection operations (particularly during the PM Peak period) during all horizon years compared to “Scenario 1” which does not include a right-turn lane.
- “Baseline Recommended” option includes a right-turn lane with a storage length (105 m) that is close to the 2031 PM Peak period 95th percentile queue (110 m). However, this is the option that results in the impacts to the Sheraton property described above
- “Scenario 2” includes the shorter right-turn lane. Level-of-service and v/c ratios **are equivalent** to the “Baseline Recommended” option; however, its storage length is not long enough to contain 95th percentile queues in any horizon year. This indicates that in the most congested periods of the PM peak hour, queues will extend beyond the right-turn lane and into the through lane
- Preferred option based on traffic operations in “Scenario 2” with the shorter right-turn lane. This option balances traffic operations and queuing with the cost of the project and extent of impacts to the Sheraton Four Points property.

Design Feasibility

A design review has indicated that a right-turn lane can fit at the intersection as follows:

- 26 m storage
- 44 m taper

A conceptual design sketch is shown in

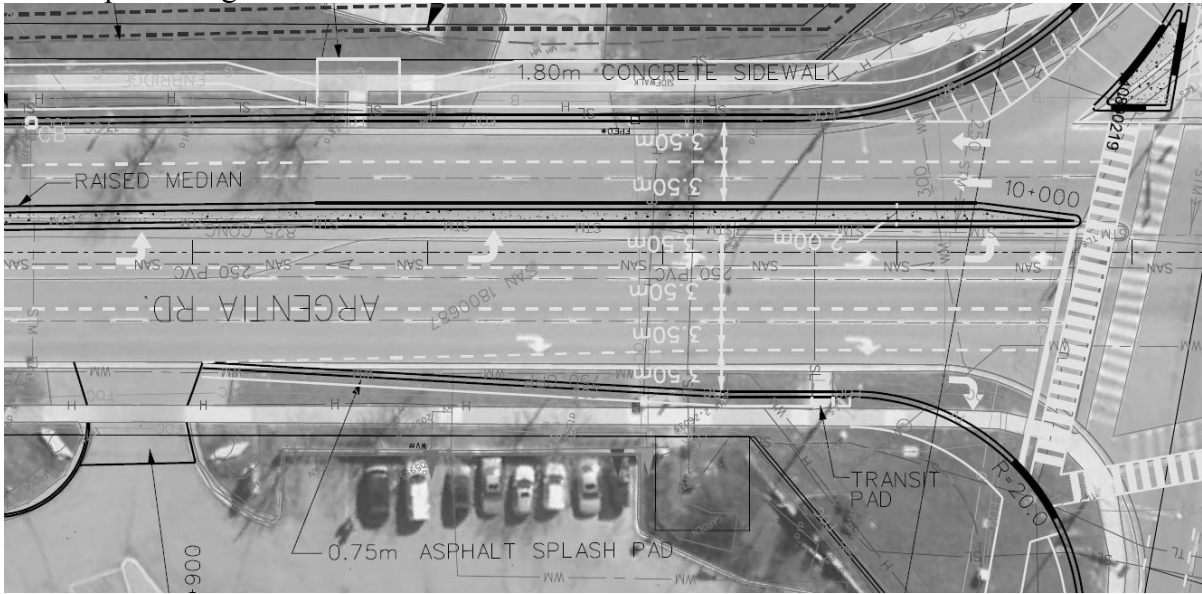


Figure 2.

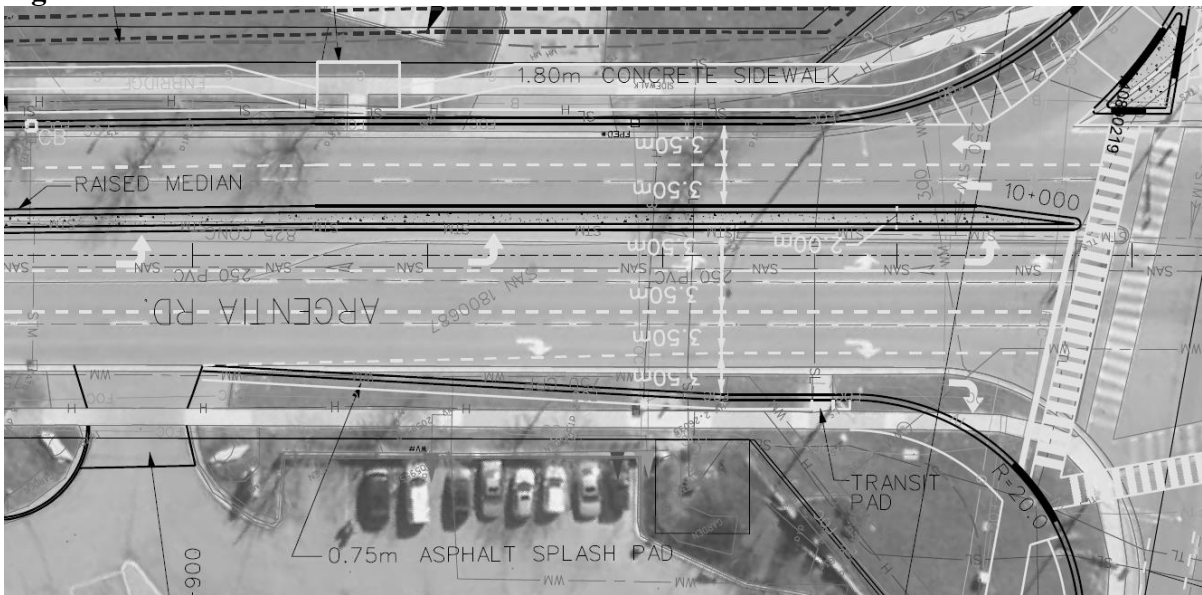


Figure 2: Conceptual Sketch of Shortened Northbound Right-Turn Lane

A lane of this length would result in fewer impacts to the Sheraton hotel site. It is **recommended for incorporation** into the preliminary design that will be documented within the EA. It is noted that this provides less storage than would be ideal; however, it is a compromise to reduce the cost and social and environmental impacts in this location.

Table 1: Existing Traffic Operations

Movement	Existing				Baseline Recommended*				Scenario 1: No NB RT Lane				Scenario 2: Shorter NB RT Lane			
	AM		PM		AM		PM		AM		PM		AM		PM	
	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS
Intersection Overall	0.94	D	0.91	D	0.83	C	0.79	C	0.86	D	0.89	D	0.83	C	0.79	C
Eastbound Left	0.20	C	0.85	F	0.19	C	0.77	F	0.19	C	0.77	F	0.19	C	0.77	F
Eastbound Through	0.98	E	0.43	C	0.91	D	0.43	C	0.91	D	0.49	C	0.91	D	0.43	C
Eastbound Right	0.31	A	0.11	A	0.31	A	0.11	A	0.31	A	0.11	A	0.31	A	0.11	A
Westbound Left	0.86	E	0.20	C	0.56	D	0.25	E	0.56	D	0.32	E	0.56	D	0.25	E
Westbound Through	0.24	B	0.91	D	0.23	B	0.84	C	0.23	B	0.96	D	0.23	B	0.84	C
Westbound Right	0.15	A	0.28	A	0.15	A	0.28	A	0.15	A	0.28	A	0.15	A	0.28	A
Northbound Left	0.60	D	0.79	D	0.41	D	0.76	D	0.42	D	0.64	C	0.41	D	0.76	D
Northbound Through	0.30	D	0.91	E	0.21	D	0.63	D	0.26	D	0.76	D	0.21	D	0.63	D
Northbound Right	0.08	D	0.43	D	0.08	D	0.54	D	n/a	n/a	n/a	n/a	0.08	D	0.54	D
Southbound Left	0.81	E	1.00	F	0.94	E	0.73	D	1.04	F	0.86	D	0.94	E	0.73	D
Southbound Through/Right	0.98	F	0.75	E	0.63	D	0.42	D	0.64	D	0.32	D	0.63	D	0.42	D

*Cycle length and cycle splits optimized

Table 2: 2021 Traffic Operations

Movement	Do Nothing				Baseline Recommended*				Scenario 1: No NB RT Lane				Scenario 2: Shorter NB RT Lane			
	AM		PM		AM		PM		AM		PM		AM		PM	
	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS
Intersection Overall	1.06	E	1.01	E	0.91	D	0.90	D	0.99	D	0.96	D	0.91	D	0.90	D
Eastbound Left	0.23	C	0.96	F	0.22	C	0.94	F	0.24	C	0.98	F	0.22	C	0.94	F
Eastbound Through	1.01	E	0.48	C	0.97	D	0.44	C	1.04	E	0.50	C	0.97	D	0.44	C
Eastbound Right	0.35	A	0.13	A	0.35	A	0.13	A	0.35	A	0.13	A	0.35	A	0.13	A
Westbound Left	1.16	F	0.31	C	0.70	E	0.51	E	0.62	E	0.62	F	0.70	E	0.51	E
Westbound Through	0.27	B	1.05	E	0.25	B	0.92	D	0.25	B	1.05	E	0.25	B	0.92	D
Westbound Right	0.17	A	0.31	A	0.17	A	0.31	A	0.17	A	0.31	A	0.17	A	0.31	A
Northbound Left	0.69	E	0.90	D	0.44	D	0.90	E	0.47	D	0.71	D	0.44	D	0.90	E
Northbound Through	0.36	D	0.98	F	0.22	D	0.64	D	0.27	D	0.88	E	0.22	D	0.64	D
Northbound Right	0.09	D	0.59	D	0.09	D	0.72	E	n/a	n/a	n/a	n/a	0.09	D	0.72	E
Southbound Left	0.92	E	1.06	F	0.98	F	0.88	E	1.15	F	0.93	F	0.98	F	0.88	E
Southbound Through/Right	1.11	F	0.82	E	0.74	E	0.46	D	0.71	D	0.37	D	0.74	E	0.48	D

Table 3: 2031 Traffic Operations

Movement	Do Nothing				Baseline Recommended*				Scenario 1: No NB RT Lane				Scenario 2: Shorter NB RT Lane			
	AM		PM		AM		PM		AM		PM		AM		PM	
	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS	v/c	LOS
Intersection Overall	1.22	F	1.13	F	1.02	E	1.01	D	1.09	E	1.06	E	1.02	E	1.01	D
Eastbound Left	0.27	C	1.04	F	0.26	C	1.04	F	0.26	C	1.12	F	0.26	C	1.04	F
Eastbound Through	1.12	F	0.52	C	1.07	E	0.50	C	1.07	E	0.54	C	1.07	E	0.50	C
Eastbound Right	0.39	A	0.14	A	0.39	A	0.14	A	0.39	A	0.14	A	0.39	A	0.14	A
Westbound Left	1.39	F	0.37	C	0.85	E	0.60	E	0.85	E	0.62	F	0.85	E	0.59	E
Westbound Through	0.30	B	1.13	F	0.28	B	1.06	E	0.28	B	1.14	E	0.28	B	1.06	E
Westbound Right	0.18	A	0.34	A	0.18	A	0.34	A	0.18	A	0.34	A	0.18	A	0.34	A
Northbound Left	0.76	E	1.06	F	0.56	D	0.97	E	0.56	D	1.86	F	0.56	D	0.97	E
Northbound Through	0.40	D	1.09	F	0.22	D	0.71	D	0.28	D	1.01	F	0.22	D	0.71	D
Northbound Right	0.10	D	0.69	D	0.10	D	0.81	E	n/a	n/a	n/a	n/a	0.10	D	0.81	E
Southbound Left	0.99	F	1.24	F	1.10	F	0.91	E	1.32	F	1.04	F	1.10	F	0.91	E
Southbound Through/Right	1.17	F	0.98	F	0.76	E	0.47	D	0.76	E	0.42	D	0.76	E	0.47	D

Table 4: Northbound Movements - Proposed Storage and 95th Percentile Queues

Movement	Existing Storage	Do Nothing			Baseline Recommended*			Scenario 1: No NB RT Lane			Scenario 2: Shorter NB RT Lane		
		Proposed Storage (m)	AM Queue (m)	PM Queue (m)	Proposed Storage (m)	AM Queue (m)	PM Queue (m)	Proposed Storage (m)	AM Queue (m)	PM Queue (m)	Proposed Storage (m)	AM Queue (m)	PM Queue (m)
Existing													
Northbound Through	75*	75*	45	180#	115**	20	65	115**	20	95	115**	20	65
Northbound Right	75	75	15	60	105 m storage (60 m taper)	12	55	No dedicated lane – 0 m	N/A	N/A	26 m storage (44 m taper)	12	55
2021													
Northbound Through	75*	75*	50	215#	115**	25	80	115**	25	140	115**	25	80
Northbound Right	75	75	15	90	105 m storage (60 m taper)	15	90	No dedicated lane – 0 m	N/A	N/A	26 m storage (44 m taper)	15	90
2031													
Northbound Through	75*	75*	55	250#	115**	30	95	115**	30	185#	115**	30	95
Northbound Right	75	75	15	105	105 m storage (60 m taper)	15	110#	No dedicated lane – 0 m	N/A	N/A	26 m storage (44 m taper)	15	110#

*Through movement storage measured as distance to first driveway where it will block left-turn movements (existing Sheraton driveway – 75 m)

**Through lane storage measured as distance to re-located Sheraton driveway, 115 m south of the intersection

#Synchro output for queues is based upon a probability distribution; there is a small probability that queues will slightly exceed this length in some instance

Appendix G

Northbound Left-Turn Lane and Accesses

Memo – Northbound Left-Turn Lane

Project: Derry / Argentia EA
Subject: **Northbound Left-Turn Lane**
To: Sally Rook – Region of Peel
From: Tony Reitmeier – HDR
 Andrew O’Connor – HDR
Date: Thursday, December 4, 2014

The purpose of this memo is to outline design concepts considered for accesses to Argentia Road south of Derry Road.

During the development of the preliminary design, HDR proposed some modifications to accesses to Argentia Road south of Derry Road:

- Convert existing Sheraton Hotel access to “Right-in/Right-out”
- Provide a second full-moves access to the Sheraton Hotel at the south edge of their site
- Convert proposed First Gulf access to “Right-in/Right-out/Left-in”

The re-location of the accesses was proposed for the following reasons:

- Enable improved traffic operations at the intersection of Derry Road and Argentia Road
- Improve safety at the driveway to the Sheraton hotel and the proposed First Gulf development
- Access control

Traffic Operations on Argentia Road

Modifying the accesses would allow for a longer left-turn lane providing additional storage for vehicles making a northbound left-turn. The Synchro results for the northbound movements are shown below in **Table 1**.

Table 1: Queue Lengths

Movement	Existing Distance Available for Queuing (m)	2011 – 95 th Percentile Queue (m)		2021 – 95 th Percentile Queue (m)		2031 – 95 th Percentile Queue (m)	
		AM	PM	AM	PM	AM	PM
NBL	60	20	75	25	80*	25	115*
NBT	75	20	75	25	80	25	95
NBR	75	15	65	15	90	15	110*

*Synchro output for queues is based upon a probability distribution; there is a small probability that queues will slightly exceed this length in some instances

The Sheraton driveway limits the existing northbound left turn lane to a storage length of 60 m. Adding a “full moves” driveway to the south can allow for an extended northbound left-turn lane that provides a storage length of 100 m. The Synchro modelling results indicate that storage lengths beyond the existing 60 m length are required during the PM Peak Hour under existing conditions, 2021 conditions and 2031 conditions.

Shifting the “full-moves” access to the south also reduces the extent of queues in the through lane or the right-turn lane blocking the access to the site.

Safety and Operations at the Sheraton Driveway

Adding a “full moves” access to the south will allow for improved operations at the Sheraton driveway. Vehicles will be able to turn into and out of the driveway more easily, because queues on Argentia Road will be less likely to block the driveway.

Shifting the access is also expected to improve safety performance at the driveway. Six out of the 82 collisions (7.3%) reported at the intersection of Derry Road and Argentia Road over the 5 year assessment period (2008 to 2012) took place specifically at the Sheraton driveway (refer to **Figure 1**). There were 5 left-turn collisions and 1 rear end collision. It is likely that the left-turn collisions were the result of the congestion and queuing on Argentia Road. Restricting the existing driveway to “Right-in/Right-out” and adding a “full moves” driveway further to the south is expected to improve the safety performance at the driveway.

A conceptual design of the proposed second access at the Sheraton hotel site is provided as **Figure 2**.

Access Control

The original First Gulf Corporation development application for 2476 Argentia Road (the site at the southwest corner of Derry Road and Argentia Road) included a full moves access directly across from the existing Sheraton Hotel access. Vehicles making left turns out of this site would have difficulty making the turn during the PM Peak when there are vehicle queues on Argentia Road. By restricting the access to “Right-in/Right-out/Left-in”, all vehicles would make a right-turn to exit the site. This would reduce the number of conflicting turning movement and queues near to the intersection of Derry Road and Argentia Road.

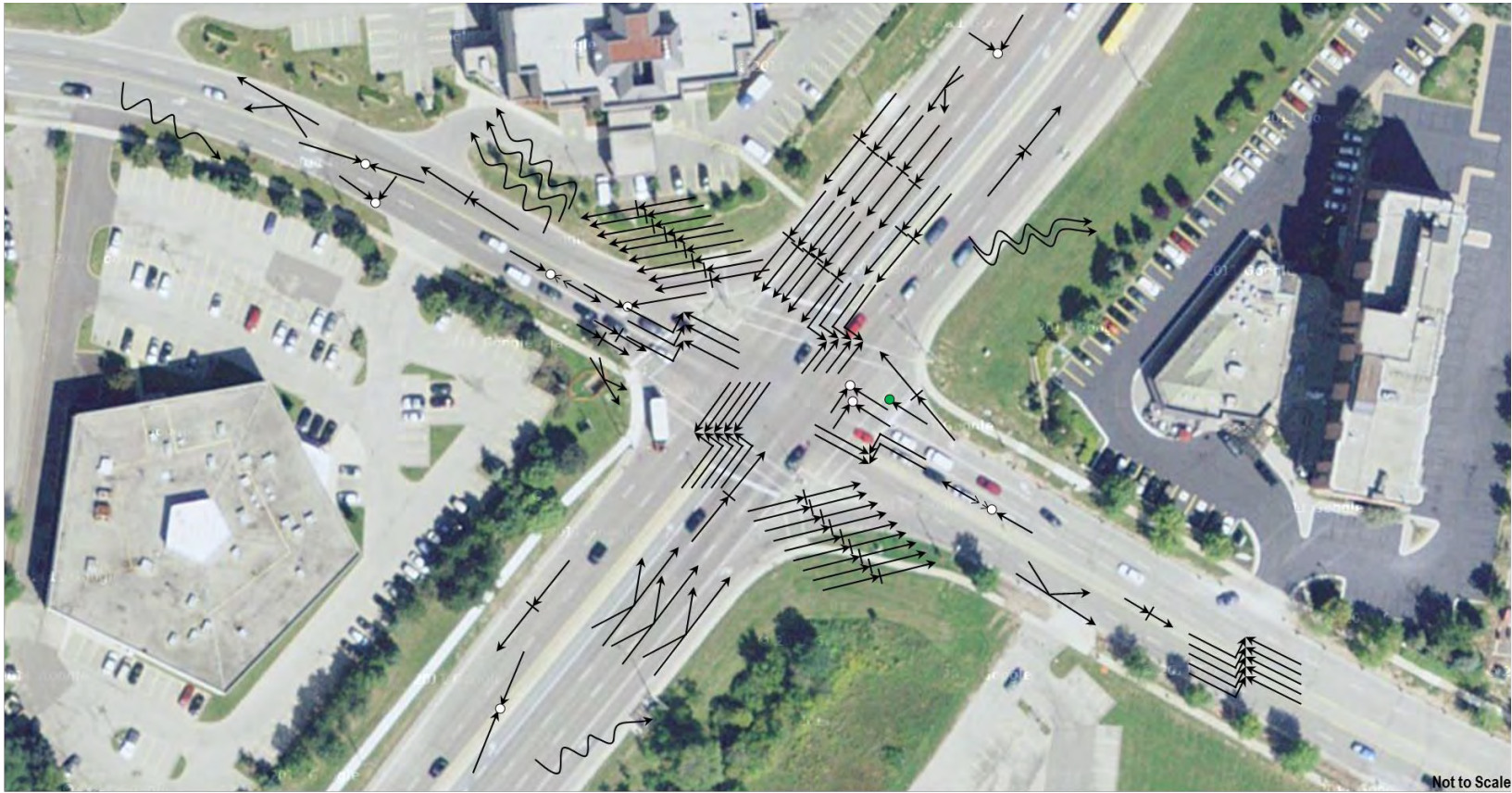
The Traffic Impact Study completed for this development forecast that 86% (or 150 out of 175 vehicles) would turn right out of the driveway instead of turning left. The study notes that “site traffic was assigned to the network acknowledging the prevailing capacity constraints at the Derry Road / Argentia Road intersection”, indicating that vehicles turn right instead of left as a result of the queues on Argentia Road. Therefore, restricting the “left-out” move would only impact a small number of vehicles.

Final Design

Through consultation with the Sheraton Hotel, they indicated support for the proposed design, which would include the construction of a “full moves” access at the south side of the property and the conversion of their existing access to “Right-in/Right-out”. This recommendation is reflected in the preliminary design.

Through consultation with First Gulf, they indicated that they did not support the restriction of their access to “Right-in/Right-out/Left-in”. Working with the City of Mississauga, the site plan for First Gulf was amended to reflect a re-location of their driveway to the southern-most limit of their property, to maximize the unimpeded storage of the northbound lanes on

Argentia Road. The driveway will be designed to permit full moves, but signage is recommended to restrict “left-out” movements during the PM Peak period when conflicts with queued vehicles are expected. This change is reflected in the preliminary design.



Key to Symbols	
Vehicle Movement	Collision Type
↶ Left	→→ Rear-end
→ Straight	↘ Turning Movement
↷ Right	● Pedestrian
↔ Reversing	⚡ Run Off Road
	↘ Angle
	↘ Sideswipe (same direction)
	↘ Sideswipe (opposite direction)
Notes: Collision locations are approximate	

Figure 1: Collision Locations within Intersection of Derry Road and Argentia Road

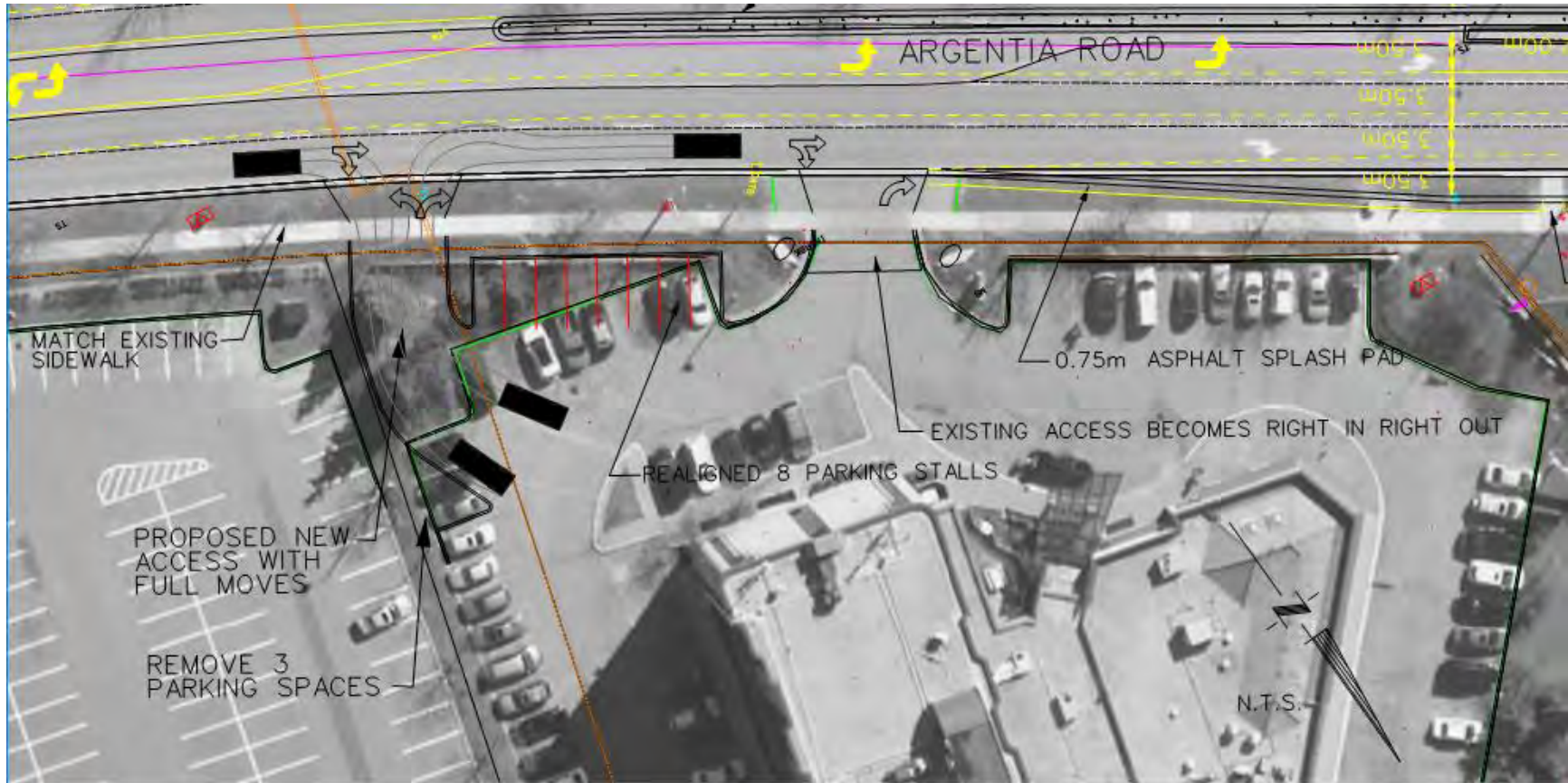


Figure 2: Conceptual Design of Sheraton Second Access

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